

**FIELD EVALUATION SHEET**

PRELIMINARY EVALUATION DATE 6-5-24, FIELD EVALUATION DATE 7-13-24  
PROPERTY OWNER: Todd & Kellie Akins PHONE \_\_\_\_\_  
ADDRESS: 19984 472 nd street CITY, STATE, ZIP: McGregor, MN 55760  
LEGAL DESCRIPTION: Part of lot 2  
PIN# 29-0-057304 SEC 28 T 49 R 23 TWP NAME Shamrock  
FIRE# \_\_\_\_\_ LAKE/RIVER Minnewawa LAKE CLASS \_\_\_\_\_ OHWL \_\_\_\_\_ FT.

**DESCRIPTION OF SOIL TREATMENT AREAS**

	AREA #1	AREA #2	REFERENCE BM ELEV. _____ FT.
DISTURBED AREAS	YES ___ NO <u>X</u>	YES ___ NO ___	REFERENCE BM DESCRIPTION _____
COMPACTED AREAS	YES ___ NO <u>X</u>	YES ___ NO ___	_____
FLOODING	YES ___ NO <u>X</u>	YES ___ NO ___	_____
RUN ON POTENTIAL	YES ___ NO <u>X</u>	YES ___ NO ___	_____
SLOPE %	_____	_____	_____
DIRECTION OF SLOPE	_____	_____	_____
LANDSCAPE POSITION	<u>Summit</u>	_____	_____
VEGETATION TYPES	<u>Wooded</u>	_____	_____

DEPTH TO STANDING WATER OR MOTTLED SOIL: BORING# 1 68", 1A \_\_\_\_\_, 2 68", 2A \_\_\_\_\_

BOTTOM ELEVATION--FIRST TRENCH OR BOTTOM OF ROCK BED: #1 \_\_\_\_\_ FT., #2 \_\_\_\_\_ FT.

SOIL SIZING FACTOR: SITE #1 1.27, SITE #2 \_\_\_\_\_

CONSTRUCTION RELATED ISSUES: \_\_\_\_\_

LIC# 910 SITE EVALUATOR SIGNATURE: Ernie Darlow Jr

SITE EVALUATOR NAME: Ernie Darlow Jr. TELEPHONE# 218-426-4320

LUG REVIEW \_\_\_\_\_ DATE 7-13-24

Comments: Sewage will be pumped up from the walkout into a gravity line. 1650 gallon combo tank - Gravity to a settling pump tank.

Pumped up to a pressure bed sized for 4 bedrooms.

SOIL BORING LOGS ON REVERSE SIDE

7-13-24  
29-0-057304

# SOILS CHARTS FOR BOTH PROPOSED AND ALTERNATE SITES

1 (PROPOSED) SOILS DATA

DEPTH (INCHES)	TEXTURE	MUNSELL COLOR
0-4"	Top Soil	10yr 3/2
4" to 48"	Sand	10yr 4/4
48" to 68"	Sand	10yr 4/6
No mottling down to 68" or redox		

2 (PROPOSED) SOILS DATA

DEPTH (INCHES)	TEXTURE	MUNSELL COLOR
0-4"	Top Soil	10yr 3/2
4" to 48"	Sand	10yr 4/4
48" to 68"	Sand	10yr 4/6
No mottling down to 68" or redox		

1 (ALTERNATE) SOILS DATA

DEPTH (INCHES)	TEXTURE	MUNSELL COLOR

2 (ALTERNATE) SOILS DATA

DEPTH (INCHES)	TEXTURE	MUNSELL COLOR

ADDITIONAL SOIL BORINGS MAY BE REQUIRED



# TRENCH AND BED WORKSHEET

## 1. AVERAGE DESIGN FLOW

- A. Estimated 600 gpd (see figure A-1)  
 or measured     x 1.5 (safety factor) =     gpd
- B. Septic tank capacity     gal (see figure C-1)

## 2. SOILS (Site evaluation data)

- C. Depth to restricting layer = 5.7 ft (32")
- D. Max depth of system Item 2C - 3 ft = 5.7 ft - 3 ft = 2.7 ft
- E. Texture Sand Percolation rate     MPI
- F. Soil Sizing Factor (SSF) 1.27 sqft/gpd (see figure D-15)
- G. % Land Slope     %

## 3. TRENCH or BED BOTTOM AREA

- H. For trenches with 6 inches of rock below the pipe:  
 $A \times F = 600 \text{ gpd} \times 1.27 \text{ sqft/gpd} = 762 \text{ sqft}$
- I. For trenches with 12 inches of rock below the pipe:  
 $A \times F \times 0.8 = \text{   } \text{ gpd} \times \text{   } \text{ sqft/gpd} \times 0.8 = \text{   } \text{ sqft}$
- J. For trenches with 18 inches of rock below the pipe:  
 $A \times F \times 0.66 = \text{   } \text{ gpd} \times \text{   } \text{ sqft/gpd} \times 0.66 = \text{   } \text{ sqft}$
- K. For trenches with 24 inches of rock below the pipe:  
 $A \times F \times 0.6 = \text{   } \text{ gpd} \times \text{   } \text{ sqft/gpd} \times 0.6 = \text{   } \text{ sqft}$
- L. For gravity beds with 6 or 12 inches of rock below the pipe:  
 $1.5 \times A \times F = 1.5 \times \text{   } \text{ gpd} \times \text{   } \text{ sqft/gpd} = \text{   } \text{ sqft}$   
 For pressure beds with 6 or 12 inches of rock below the pipe:  
 $A \times F = \text{   } \text{ gpd} \times \text{   } \text{ sqft/gpd} = \text{   } \text{ sqft}$

## 4. DISTRIBUTION (Check all that apply)

- Bed (< 6% slope)     Drop boxes (any slope)  Rock
- Trenches     Distribution box (< 3%)     Chamber
- Pressure     Gravity     Gravelless

## 5. SYSTEM WIDTH, LENGTH and VOLUME

- M. Select trench width = 18 ft
- N. If using rock, divide bottom area by width:  $(H, I, J, K \text{ or } L) \div M =$   
 $762 \text{ sqft} \div 18 \text{ ft} = 42.5 \text{ lineal feet}$   
 Rock depth below distribution pipe plus 0.5 foot times bottom area:  
 Rock depth in feet + 0.5 feet x Area (H, I, J, K, or L)  
 $(0.5 \text{ ft} + 0.5 \text{ ft}) \times 762 \text{ sqft} = 762 \text{ cuft}$   
 Volume in cubic yards = cuft  $\div 27$   
 $762 \text{ cuft} \div 27 = 28.3 \text{ cu yds}$   
 Weight of rock in tons = cubic yds x 1.4  
 $28.3 \text{ cu yds} \times 1.4 = 40 \text{ tons}$
- O. If using 10" Gravelless Pipe, Flow (A) x Gravelless SSF (see figure D-9)  
    gpd x     lineal feet/gpd =     lineal feet
- P. If using Chambers, H, I, J, or K (based on height of chamber slats)  $\div$   
 width of chamber in feet (M)  
    sqft  $\div$      ft =     lineal ft

## 6. LAWN AREA

- Q. Select trench spacing, center to center = 18 feet
- R. Multiply trench spacing by lineal feet R x Q = sqft of lawn area  
18 ft x 42.5 ft = 762 sqft

7. Include a drawing with scale (one inch = 40 ft). Show pertinent boundaries, right of way, easements, location of house, garage, driveway, all other improvements, existing or proposed soil treatment system, well and dimensions of all elevations, setbacks and separation distances.

number of bedrooms	Class I	Class II	Class III	Class IV
2	300	225	180	60%
3	450	300	218	of the
<u>4</u>	<u>600</u>	375	256	values
5	750	450	294	in the
6	900	525	332	Class I,
7	1050	600	370	II, or III
8	1200	675	408	columns.

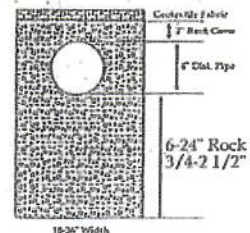
Number of Bedrooms	Minimum Liquid Capacity	Liquid capacity with garbage disposal	Liquid capacity with disposal & lift inside
2 or less	750	1125	1500
3 or 4	1000	1500	2000
5 or 6	1500	2250	3000
7, 8 or 9	2000	3000	4000

Percolation Rate (minutes per inch (mpi))	Soil Texture	Soil Sizing Factor (square feet/gallon per day (sqft/gpd))
faster than 0.1*	Coarse sand	0.83
0.1 to 5'	Medium sand	0.83
	Loamy sand	
0.1 to 5"	Fine sand	1.67
6 to 15	Sandy loam	1.27
16 to 30	Loam	1.67
31 to 45	Silt loam	2.00
	Silt	
46 to 60	Clay loam	2.20
	Sandy clay	
	Silty clay	
over 61 to 120***	Clay	4.20
	Sandy clay	
	Silty clay	
slower than 120****		

\*Use systems for rapidly permeable soils: pressure distribution or serial distribution with no trench > 25% of the total system.  
 \*\*Soil having 50% or more fine sand plus very fine sand  
 \*\*\*A mound must be used.  
 \*\*\*\*An other or performance system must be used

percolation rate (minutes/inch)	soil texture	lineal feet/gallon/day
Faster than 0.1*	Coarse Sand	---
0.1 to 5	Medium Sand	0.28
	Loamy Sand	
0.1 to 5	Fine Sand**	0.6
6 to 15	Sandy Loam	0.42
16 to 30	Loam	0.56
31 to 45	Silt Loam	0.67
	Silt	
46 to 60	Clay Loam (CL)	0.74
	Sandy CL	
	Silty CL	
slower than 60***	Clay	---
	Sandy Clay	
	Silty Clay	

\*Soil too coarse for sewage treatment.  
 Use systems for rapidly permeable soils.  
 \*\*Soil having 50% or more fine sand + very fine sand.  
 \*\*\*Soil with too high a percentage of clay for installation of a standard inground system.



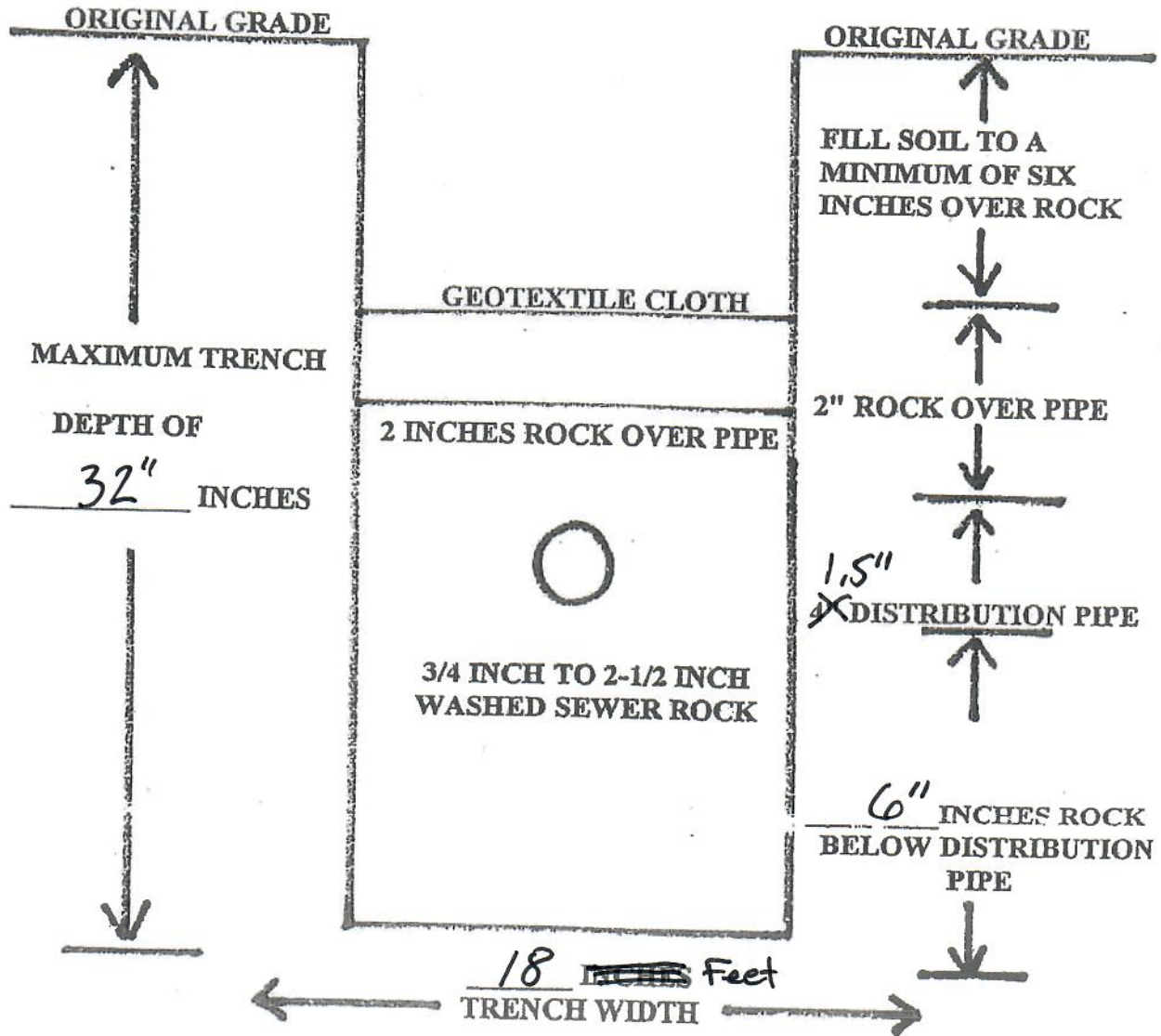
I hereby certify that I have completed this work in accordance with applicable ordinances, rules and laws.

Emie Darlo Jr. (signature) 910 (license #) 7-13-24 (date)

TRENCH CROSS-SECTION

FINISHED GRADE

22.5 INCHES OF BACKFILL OVER ROCK





# PRESSURE DISTRIBUTION SYSTEM

- Select number of perforated laterals 5
- Select perforation spacing = 3 ft
- Since perforations should not be placed closer than 1 foot to the edge of the rock layer (see diagram), subtract 2 feet from the rock layer length.

$$\frac{42.5}{\text{Rock layer length}} - 2 \text{ ft} = \underline{40.5} \text{ ft}$$

- Determine the number of spaces between perforations. Divide the length (3) by perforation spacing (2) and round down to nearest whole number.

$$\text{Perforation spacing} = \underline{40.5} \text{ ft} \div \underline{3} \text{ ft} = \underline{13.5} \text{ spaces}$$

- Number of perforations is equal to one plus the number of perforation spaces (4). Check figure E-4 to assure the number of perforations per lateral guarantees <10% discharge variation.

$$\underline{13.5} \text{ spaces} + 1 = \underline{15} \text{ perforations/lateral}$$

- A. Total number of perforations = perforations per lateral (5) times number of laterals (1)

$$\underline{15} \text{ perms/lat} \times \underline{5} \text{ lat} = \underline{75} \text{ perforations}$$

- B. Calculate the square footage per perforation.

Should be 6-10 sqft/perf. Does not apply to at-grades.

Rock bed area = rock width (ft) x rock length (ft)

$$\underline{18} \text{ ft} \times \underline{42.5} \text{ ft} = \underline{765} \text{ sqft}$$

Square foot per perforation = Rock bed area ÷ number of perms (6)

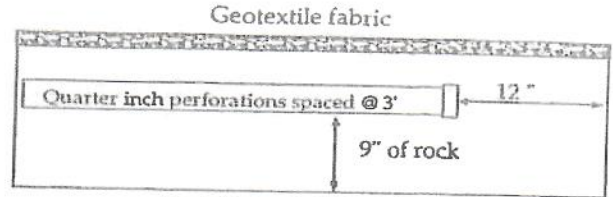
$$\underline{765} \text{ sqft} \div \underline{75} \text{ perms} = \underline{10.2} \text{ sqft/perf}$$

- Determine required flow rate by multiplying the total number of perforations (6A) by flow per perforation (see figure E-6)

$$\underline{75} \text{ perms} \times \underline{.56} \text{ gpm/perfs} = \underline{42} \text{ gpm}$$

- If laterals are connected to header pipe as shown on upper example, to select minimum required lateral diameter; enter figure E-4 with perforation spacing (2) and number of perforations per lateral (5) Select minimum diameter for perforated lateral = 1.5 inches.

- If perforated lateral system is attached to manifold pipe near the center, lower diagram, perforated lateral length (3) and number of perforations per lateral (5) will be approximately one half of that in step 8. Using these values, select minimum diameter for perforated lateral = 1.5 inches.



Perf Sizing 3/16" - 1/4"  
Perf Spacing 1.5' - 5'

E-4: Maximum allowable number of 1/4-inch perforations per lateral to guarantee <10% discharge variation

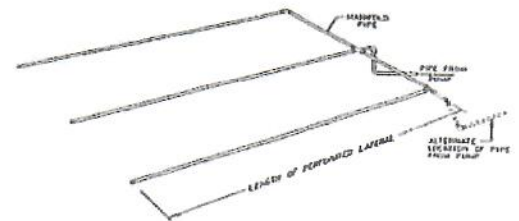
perforation spacing (feet)	1 inch	1.25 inch	1.5 inch	2.0 inch
2.5	8	14	18	28
<u>3.0</u>	8	13	<u>17</u>	26
3.3	7	12	16	25
4.0	7	11	15	23
5.0	6	10	14	22

E-6: Perforation Discharge in gpm

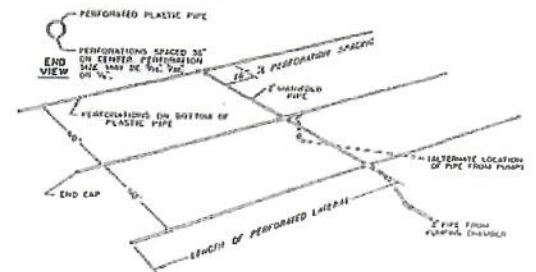
head (feet)	perforation diameter (inches)			
	1/8	3/16	<u>7/32</u>	1/4
1.0 <sup>a</sup>	0.18	0.42	<u>0.56</u>	0.74
2.0 <sup>b</sup>	0.26	0.59	0.80	1.04
5.0	0.41	0.94	1.26	1.65

<sup>a</sup> Use 1.0 foot for single-family homes.  
<sup>b</sup> Use 2.0 feet for anything else.

MANIFOLD LOCATED AT END OF PRESSURE DISTRIBUTION SYSTEM



LAYOUT OF PERFORATED PIPE LATERALS FOR PRESSURE DISTRIBUTION IN MOUND



I hereby certify that I have completed this work in accordance with applicable ordinances, rules and laws.

Eric Dark (signature)

(signature)

910 (license #)

(license #)

7-13-24 (date)

(date)



# PUMP SELECTION PROCEDURE

## 1. Determine pump capacity:

### A. Gravity distribution

1. Minimum required discharge is 10 gpm
2. Maximum suggested discharge is 45 gpm. For other establishments at least 10% greater than the water supply rate, but no faster than the rate at which effluent will flow out of the distribution device.

### B. Pressure distribution

See pressure distribution work sheet

From A or B Selected pump capacity: 42 gpm

## 2. Determine pump head requirements:

A. Elevation difference between pump and point of discharge?

10 feet

B. Special head requirement? (See Figure at right - Special Head Requirements)

5 feet

C. Calculate Friction loss

1. Select pipe diameter 1.5 in

2. Enter Figure E-9 with gpm (1A or B) and pipe diameter (C1).

Read friction loss in feet per 100 feet from Figure E-9

Friction Loss = 11.07 ft/100ft of pipe

3. Determine total pipe length from pump discharge to soil treatment discharge point. Estimate by adding 25 percent to pipe length for fitting loss. Total pipe length times 1.25 = equivalent pipe length

45 feet x 1.25 = 56.25 feet

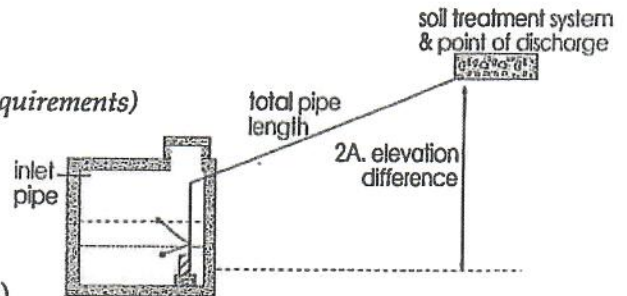
4. Calculate total friction loss by multiplying friction loss (C2) in ft/100 ft by the equivalent pipe length (C3) and divide by 100.

= 11.07 ft/100ft x 56 ÷ 100 = 6.2 ft

D. Total head required is the sum of elevation difference (A), special head requirements (B), and total friction loss (C4)

5 ft + 10 ft + 6.2 ft =

Total head: 21.2 feet



Special Head Requirements	
Gravity Distribution	0 ft
Pressure Distribution	<u>5</u> ft

flow rate gpm	E-9: Friction Loss in Plastic Pipe Per 100 feet		
	nominal pipe diameter		
	<u>1.5"</u>	2"	3"
20	2.47	0.73	0.11
25	3.73	1.11	0.16
30	5.23	1.55	0.23
35	6.96	2.06	0.30
40	8.91	2.64	0.39
<u>45</u>	<u>11.07</u>	3.28	0.48
50	13.46	3.99	0.58
55		4.76	0.70
60		5.60	0.82
65		6.48	0.95
70		7.44	1.09

## 3. Pump selection

A pump must be selected to deliver at least 42 gpm (1A or B) with at least 21.2 feet of total head (2D)

I hereby certify that I have completed this work in accordance with applicable ordinances, rules and laws.

Ernie Danks (signature)

(signature)

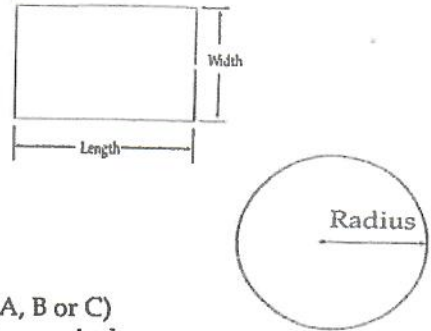
910 (license #)

(license #)

7-13-24 (date)

(date)

# 1650 Combo DOSING CHAMBER SIZING



1. Determine area
  - A. Rectangle area =  $L \times W$   
 $\underline{\hspace{2cm}} \times \underline{\hspace{2cm}} = \underline{\hspace{2cm}}$  square feet
  - B. Circle area =  $\pi (3.14) \times \text{radius in feet} \times \text{radius in feet}$   
 $3.14 \times \underline{\hspace{2cm}} \text{ ft} \times \underline{\hspace{2cm}} \text{ ft} = \underline{\hspace{2cm}}$  sqft
  - C. Get area from manufacturer  $\underline{\hspace{2cm}}$  sqft
2. Calculate gallons per inch  
 There are 7.5 gallons per cubic foot of volume, therefore multiply the area (1A, B or C) times the conversion factor and divide by 12 inches per foot to calculate gallon per inch.  
 $\text{Area} \times 7.5 \div 12 = \underline{\hspace{2cm}}$  sqft  $\times 7.5 \div 12 \text{ in/ft} = \underline{12.69}$  gallon per inch

**Legal Tank:**  
**500 gallons or**  
**100% the Daily flow**  
**or**  
**Alternating Pumps**

3. Calculate total tank volume
  - A. Depth from bottom of inlet pipe to tank bottom 48.5 in
  - B. Total tank volume = depth from bottom of inlet pipe to tank bottom (3A)  $\times$  gal/in (2)  
 $= \underline{48.5} \text{ in} \times \underline{12.69} \text{ gal/in} = \underline{615}$  gal

4. Calculate gallons to cover pump (with 2-3 inches of water covering pump)  
 (Pump and block height (inch) + 2 inch)  $\times$  gallon/inch  
 ( $\underline{\hspace{2cm}}$  in + 2 in)  $\times$   $\underline{\hspace{2cm}}$  gal/in =  $\underline{\hspace{2cm}}$  gallon

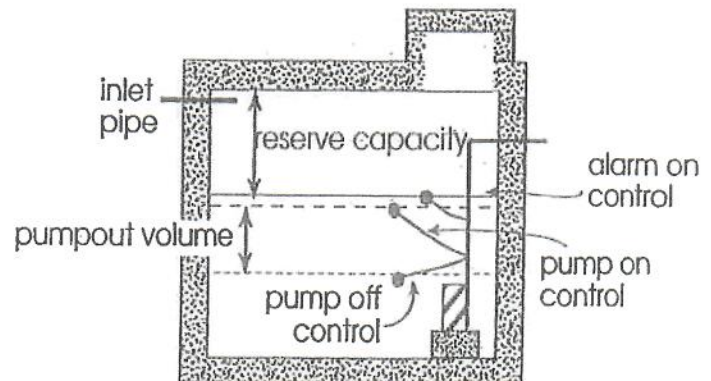
5. Calculate total pumpout volume
  - A. Select pump size for 4-5 does per day. Gallon per dose = gpd (see figure A-1) / doses per day =  $\underline{\hspace{2cm}}$  gpd  $\div$   $\underline{\hspace{2cm}}$  doses/day =  $\underline{\hspace{2cm}}$  gallons
  - B. Calculate drainback
    1. Determine total pipe length,  $\underline{\hspace{2cm}}$  feet
    2. Determine liquid volume of pipe,  $\underline{\hspace{2cm}}$  gal per ft (see figure E-20)
    3. Drainback quantity =  $\underline{\hspace{2cm}}$  ft (5B1)  $\times$   $\underline{\hspace{2cm}}$  gal per ft (5B2) =  $\underline{\hspace{2cm}}$  gal
  - C. Total pump out volume = dose volume (5A) + drainback (5B3)  
 $\underline{\hspace{2cm}}$  gal +  $\underline{\hspace{2cm}}$  gal =  $\underline{\hspace{2cm}}$  Total gallon

number of bedrooms	Class I	Class II	Class III	Class IV
2	300	225	180	60%
3	450	300	218	of the
4	600	375	256	values
5	750	450	294	in the
6	900	525	332	Class I,
7	1050	600	370	II, or III
8	1200	675	408	columns.

Pipe Diameter inches	Gallons per foot
1	0.045
1.25	0.078
1.5	0.11
2	0.17
2.5	0.25
3	0.38
4	0.66

6. Float separation distance (using total pumpout volume)  
 Total pumpout volume (5C)  $\div$  gal/inch (2)  
 $\underline{\hspace{2cm}}$  gal  $\div$   $\underline{\hspace{2cm}}$  gal/in =  $\underline{\hspace{2cm}}$  inch
7. Calculate volume for alarm (typically 2 to 3 inches)  
 Alarm depth (inch)  $\times$  gallon/inch (2) =  $\underline{\hspace{2cm}}$  in  $\times$   $\underline{\hspace{2cm}}$  gal/in =  $\underline{\hspace{2cm}}$  gal
8. Calculate total gallon = gallons over pump (4) + gallons pumpout (5C) + gallons alarm (7)  
 $\underline{\hspace{2cm}}$  gal +  $\underline{\hspace{2cm}}$  gal +  $\underline{\hspace{2cm}}$  gal =  $\underline{\hspace{2cm}}$  gallons
9. Total Tank Depth = total gallon (8)  $\div$  gallon/inch (2)  
 $\underline{\hspace{2cm}}$  gal  $\div$   $\underline{\hspace{2cm}}$  gal/in =  $\underline{\hspace{2cm}}$  in

**Recommended:**  
 Calculate reserve capacity (75% the daily flow)  
 Daily flow  $\times .75 = \underline{\hspace{2cm}}$   $\times .75 = \underline{\hspace{2cm}}$  gallons



I hereby certify that I have completed this work in accordance with applicable ordinances, rules and laws.  
Eric Daulton (signature) 910 (license #) 7-13-24 (date)



# 760 Lift pump tank DOSING CHAMBER SIZING

1. Determine area

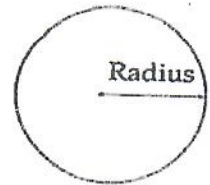
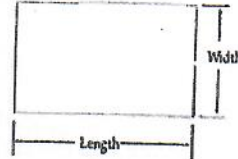
A. Rectangle area =  $L \times W$

\_\_\_\_\_ x \_\_\_\_\_ = \_\_\_\_\_ square feet

B. Circle area =  $\pi (3.14) \times \text{radius in feet} \times \text{radius in feet}$

$3.14 \times \text{_____ ft} \times \text{_____ ft} = \text{_____ sqft}$

C. Get area from manufacturer \_\_\_\_\_ sqft



2. Calculate gallons per inch

There are 7.5 gallons per cubic foot of volume, therefore multiply the area (1A, B or C) times the conversion factor and divide by 12 inches per foot to calculate gallon per inch.  
 $\text{Area} \times 7.5 \div 12 = \text{_____ sqft} \times 7.5 \div 12 \text{ in/ft} = \underline{24.92} \text{ gallon per inch}$

3. Calculate total tank volume

A. Depth from bottom of inlet pipe to tank bottom 33.5 in

B. Total tank volume = depth from bottom of inlet pipe to tank bottom (3A) x gal/in (2)  
 $= \underline{33.5} \text{ in} \times \underline{24.92} \text{ gal/in} = \underline{834} \text{ gal}$

**Legal Tank:**  
**500 gallons or**  
**100% the Daily flow**  
**or**  
**Alternating Pumps**

4. Calculate gallons to cover pump (with 2-3 inches of water covering pump)

(Pump and block height (inch) + 2 inch) x gallon/inch

$(\underline{4} \text{ in} + 2 \text{ in}) \times \underline{24.92} \text{ gal/in} = \underline{399} \text{ gallon}$

number of bedrooms	Class I	Class II	Class III	Class IV
2	300	225	180	60%
3	450	300	218	of the
4	600	375	256	values
5	750	450	294	in the
6	900	525	332	Class I,
7	1050	600	370	II, or III
8	1200	675	408	columns.

5. Calculate total pumpout volume

A. Select pump size for 4-5 does per day. Gallon per dose = gpd (see figure A-1) / doses per day = 600 gpd ÷ 5 doses/day = 120 gallons

B. Calculate drainback

1. Determine total pipe length, 45 feet

2. Determine liquid volume of pipe, .11 gal per ft (see figure E-20)

3. Drainback quantity = 45 ft (5B1) x .11 gal per ft (5B2) = 5 gal

C. Total pump out volume = dose volume (5A) + drainback (5B3)

$\underline{120} \text{ gal} + \underline{5} \text{ gal} = \underline{125} \text{ Total gallon}$

Pipe Diameter inches	Gallons per foot
1	0.045
1.25	0.078
<u>1.5</u>	<u>0.11</u>
2	0.17
2.5	0.25
3	0.38
4	0.66

6. Float separation distance (using total pumpout volume)

Total pumpout volume (5C) ÷ gal/inch (2)

$\underline{125} \text{ gal} \div \underline{24.92} \text{ gal/in} = \underline{5} \text{ inch}$

7. Calculate volume for alarm (typically 2 to 3 inches)

Alarm depth (inch) x gallon/inch (2) = 2 in x 24.92 gal/in = 49.9 gal

8. Calculate total gallon = gallons over pump (4) + gallons pumpout (5C) + gallons alarm (7)

$\underline{399} \text{ gal} + \underline{125} \text{ gal} + \underline{49.9} \text{ gal} = \underline{574} \text{ gallons}$

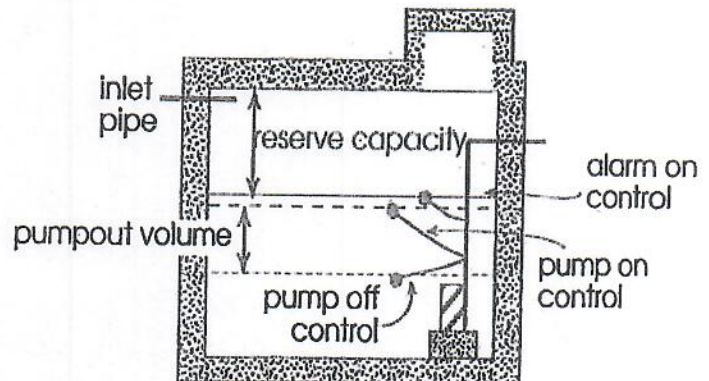
9. Total Tank Depth = total gallon (8) ÷ gallon/inch (2)

$\underline{574} \text{ gal} \div \underline{24.92} \text{ gal/in} = \underline{23} \text{ in}$

**Recommended:**

Calculate reserve capacity (75% the daily flow)

Daily flow x .75 = \_\_\_\_\_ x .75 = \_\_\_\_\_ gallons



I hereby certify that I have completed this work in accordance with applicable ordinances, rules and laws.

Ernie Park (signature)

(signature)

910 (license #)

(license #)

7.13-24 (date)

(date)



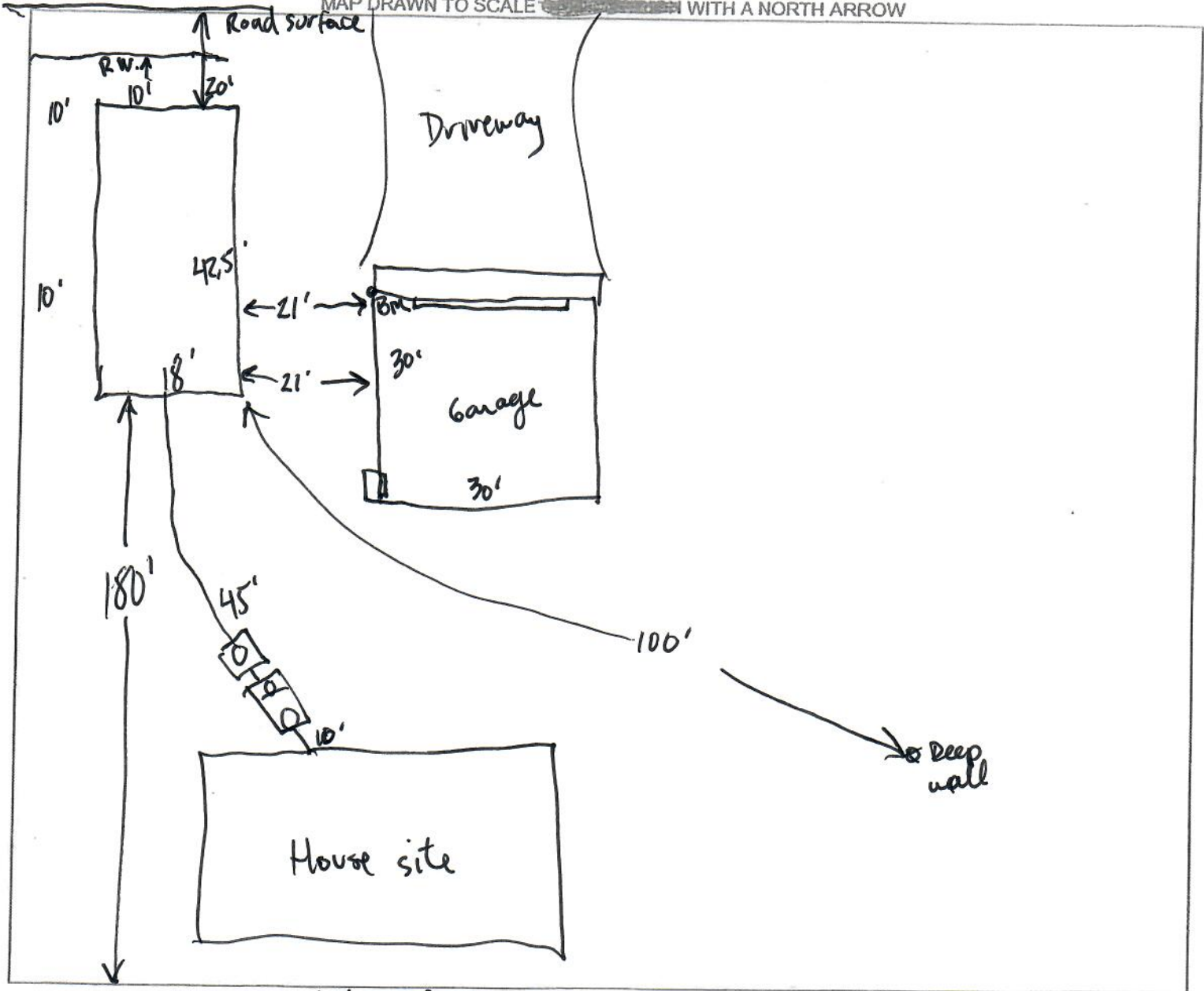
SKETCH SHEET

CLIENT: 29-0-057304

DATE: 7-13-24

472nd street

MAP DRAWN TO SCALE WITH A NORTH ARROW



Lake Minnetonka

CHECK OFF LIST--HAVE ALL OF THE FOLLOWING BEEN DRAWN ON THE MAP??

SHOW EXISTING OR PROPOSED

- WATER WELLS WITHIN 100 FT OF TREATMENT AREAS
- PRESSURE WATER LINES WITHIN 10 FT OF TREATMENT AREAS
- STRUCTURES
- ALL SOIL TREATMENT AREAS
- HORIZONTAL AND VERTICAL REFERENCE
- POINT OF SOIL BORINGS
- LOT EASEMENTS
- DISTURBED/ COMPACTED AREAS
- SITE PROTECTION--LATHE AND RIBBON EVERY 15 FT
- ACCESS ROUTE FOR TANK MAINTENANCE
- REQUIRED SETBACKS
- STRUCTURES
- OHWL
- LOT IMPROVEMENTS
- ALL ISTS COMPONENTS
- DIRECTION OF SLOPE
- ALL LOT DIMENSIONS
- PROPERTY LINES

INDICATE ELEVATIONS

- 100.0 BENCHMARK Corner of Garage
- 105.0 ELEVATION OF SEWER LINE @ HOUSE
- 105.5 ELEVATION @ TANK INLET
- 102.5 ELEVATION @ BOTTOM OF ROCK LAYER
- 105.5 ELEVATION @ BOTTOM OF BORING OR RESTRICTIVE LAYER
- 110.0 ELEVATION OF PUMP
- 102.0 ELEVATION OF DISTRIBUTION DEVICE

COMMENTS:

DESIGNER SIGNATURE Eric Dark  
 LICENSE# 910

DATE 7-13-24

# CHIPMUNK CORRAL

**REVISIONS**

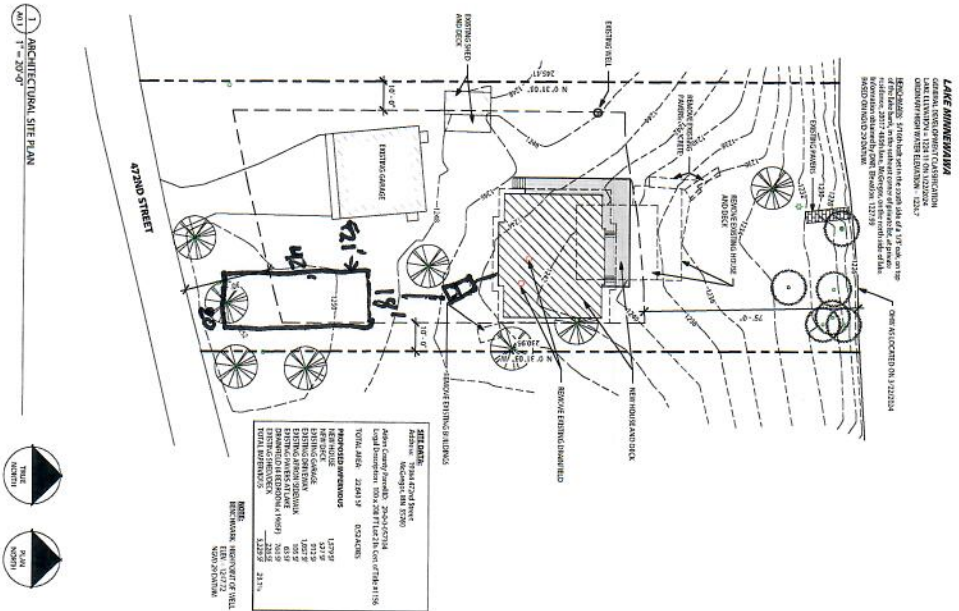

**PRELIMINARY**

4/08/2024

JOB NO. 4092

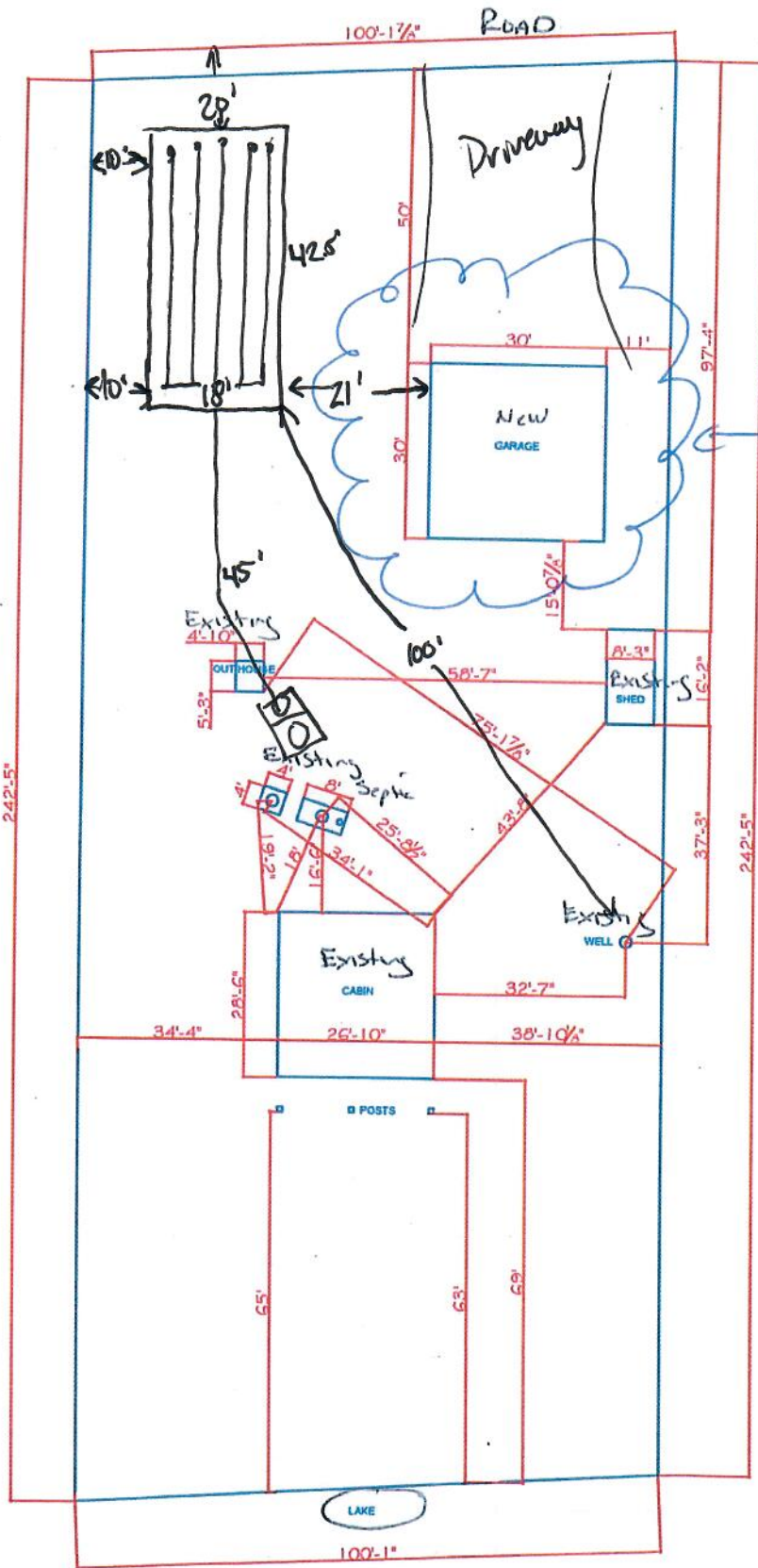
ARCHITECTURAL  
SITE PLAN

SHEET  
**A0.1**



1 ARCHITECTURAL SITE PLAN  
SCALE: 1" = 20'-0"





GARAGE IS BEING BUILT ON EXISTING DRIVEWAY  
 5-10 TREES MAY NEED TO BE REMOVED  
 4-8" DIAMETER TYPICAL SIZE OF TRUNK

