

FIELD EVALUATION SHEET

PRELIMINARY EVALUATION DATE 11/2023, FIELD EVALUATION DATE 2/14/2024
PROPERTY OWNER: John & Nicole Swanson PHONE _____
ADDRESS: 49832 405th Pl CITY, STATE, ZIP: Palisade, MN 56469
LEGAL DESCRIPTION: Lot 3 BIK 1 Northwoods Shores
PIN# 52-1-044300 SEC 13 T 49 R 27 TWP NAME _____
FIRE# 49832 LAKE/RIVER Esguagamah LAKE CLASS RD OHWLN/A FT. _____

DESCRIPTION OF SOIL TREATMENT AREAS

	AREA #1	AREA #2	REFERENCE BM ELEV. <u>N/A</u> FT.
DISTURBED AREAS	YES ___ NO <u>X</u>	YES ___ NO ___	REFERENCE BM DESCRIPTION _____
COMPACTED AREAS	YES ___ NO <u>X</u>	YES ___ NO ___	_____
FLOODING	YES ___ NO <u>X</u>	YES ___ NO ___	_____
RUN ON POTENTIAL	YES <u>X</u> NO ___	YES ___ NO ___	_____
SLOPE %	<u>7</u>	_____	_____
DIRECTION OF SLOPE	<u>NW</u>	_____	_____
LANDSCAPE POSITION	<u>side slope</u>	_____	_____
VEGETATION TYPES	<u>lawn</u>	_____	_____

DEPTH TO STANDING WATER OR MOTTLED SOIL: BORING# 1 14, 1A 16, 2 _____, 2A _____

BOTTOM ELEVATION--FIRST TRENCH OR BOTTOM OF ROCK BED: #1 N/A FT., #2 _____ FT.

SOIL SIZING FACTOR: SITE #1 1.27, SITE #2 _____

CONSTRUCTION RELATED ISSUES: system replacement

LIC# 2205 SITE EVALUATOR SIGNATURE: Dan Switzer

SITE EVALUATOR NAME: Dan Switzer TELEPHONE# 218-838-8369

LUG REVIEW _____ DATE _____

Comments: _____

SOIL BORING LOGS ON REVERSE SIDE

SOILS CHARTS FOR BOTH PROPOSED AND ALTERNATE SITES

1 (PROPOSED) SOILS DATA

DEPTH (INCHES)	TEXTURE	MUNSELL COLOR
0-2	ORGANIC	3/2 10YR
4-16	SANDY LOAM	5/3 10YR
+16	CLAY LOAM	6/4 10YR
	Mottle	6/2 10YR

2 (PROPOSED) SOILS DATA

DEPTH (INCHES)	TEXTURE	MUNSELL COLOR
0-4	ORGANIC	3/2 10YR
4-18	SANDY CLAY LOAM	4/4 10YR
+18	CLAY LOAM	6/4 10YR
	mottle	6/2 10YR

~~1 (ALTERNATE) SOILS DATA~~
^{PROPOSED}

DEPTH (INCHES)	TEXTURE	MUNSELL COLOR
0-4	ORGANIC	3/2 10YR
4-18	SANDY LOAM	4/4 10YR
+18	CLAY LOAM	6/4 10YR
		6/2 10YR

2 (ALTERNATE) SOILS DATA

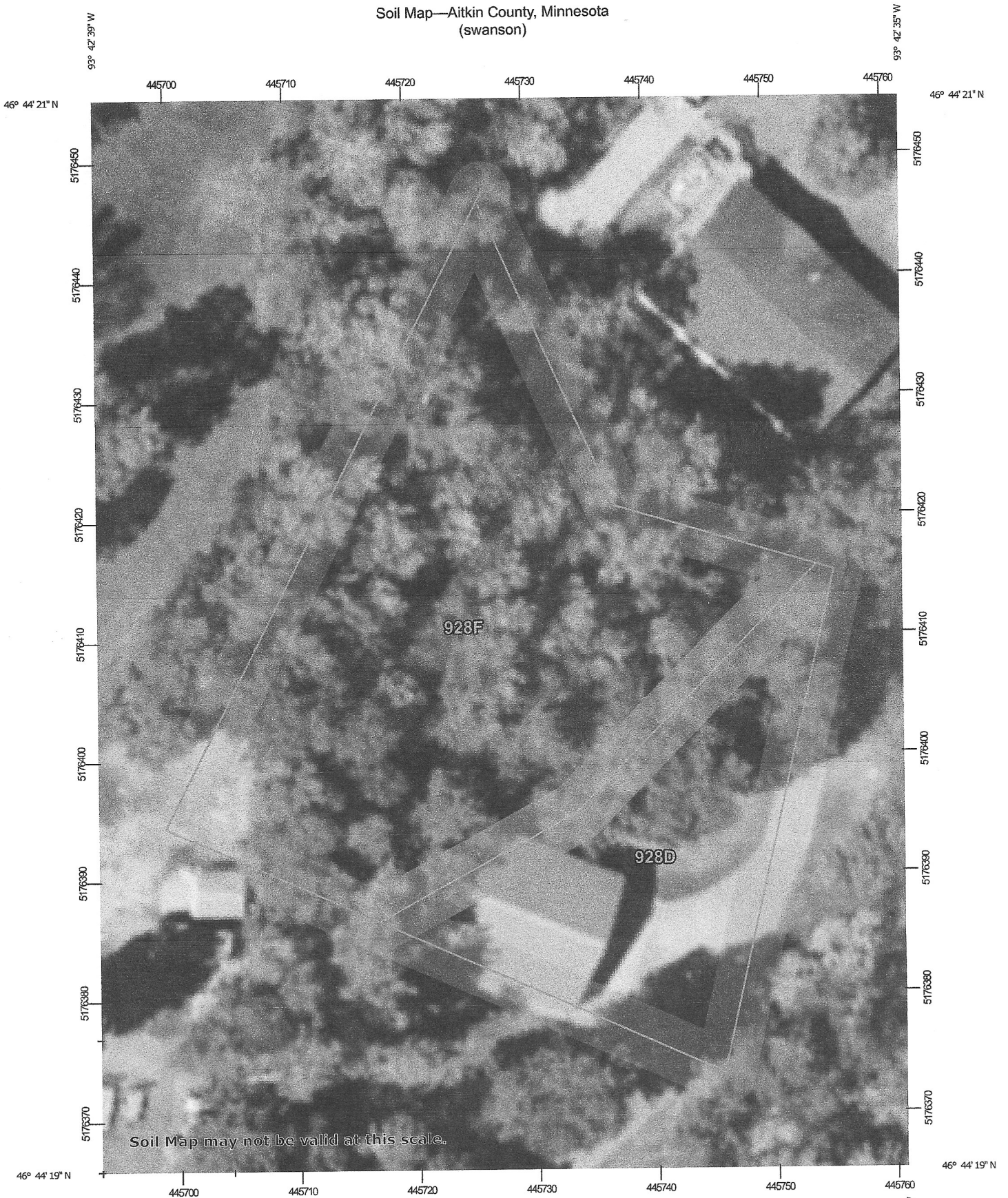
DEPTH (INCHES)	TEXTURE	MUNSELL COLOR

ADDITIONAL SOIL BORINGS MAY BE REQUIRED

Map Unit Legend

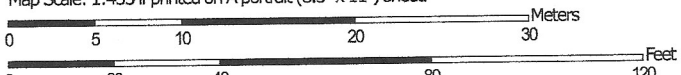
Map Unit Symbol	Map Unit Name	Acres in AOI	Percent of AOI
928D	Cushing-Mahtomedi complex, 10 to 25 percent slopes	0.2	29.9%
928F	Cushing-Mahtomedi complex, 25 to 40 percent slopes	0.4	70.1%
Totals for Area of Interest		0.5	100.0%

Soil Map—Aitkin County, Minnesota
(swanson)



Soil Map may not be valid at this scale.

Map Scale: 1:435 if printed on A portrait (8.5" x 11") sheet.



Map projection: Web Mercator Corner coordinates: WGS84 Edge tics: UTM Zone 15N WGS84



Natural Resources
Conservation Service

Web Soil Survey
National Cooperative Soil Survey

1/22/2024
Page 1 of 3

MOUND DESIGN WORK SHEET (For Flows up to 1200 gpd)

A. Average Design FLOW

Estimated 450 gpd (see figure A-1)
 or measured ✓ x 1.5 (safety factor) = ✓ gpd

A-1: Estimated Sewage Flows in Gallons per Day

number of bedrooms	Class I	Class II	Class III	Class IV
2	300	225	180	60%
3	450	300	218	of the
4	600	375	256	values
5	750	450	294	in the
6	900	525	332	Class I,
7	1050	600	370	II, or III
8	1200	675	408	columns.

B. SEPTIC TANK Capacity

1000 1500 Combination
 gallons (see figure C-1)

C-1: Septic Tank Capacities (in gallons)

Number of Bedrooms	Minimum Liquid Capacity	Liquid capacity with garbage disposal	Liquid capacity with disposal & lift inside
2 or less	750	1125	1500
3 or 4	1000	1500	2000
5 or 6	1500	2250	3000
7, 8 or 9	2000	3000	4000

C. SOILS (refer to site evaluation)

- Depth to restricting layer = 1.34 feet 16"
- Depth of percolation tests = ✓ feet
- Texture SANDY LOAM
 Percolation rate ✓ mpi
- Soil loading rate 1.27 gpd/sqft (see figure D-33)
- Percent land slope 7 %

D. ROCK LAYER DIMENSIONS

- Multiply average design flow (A) by 0.83 to obtain required rock layer area.
450 gpd x 0.83 sqft/gpd = 373.5 sqft
- Determine rock layer width = 0.83 sqft/gpd x linear Loading Rate (LLR)
 0.83 sqft/gpd x 1.2 gpd/sqft = 1.2 ft
- Length of rock layer = area ÷ width =
373.5 sqft (D1) ÷ 10 ft (D2) = 38 ft

Mound LLR

< 120 MPI	≤ 12
≥ 120 MPI	≤ 6

E. ROCK VOLUME

- Multiply rock area (D1) by rock depth of 1 ft to get cubic feet of rock
380 sqft x 1 ft = 380 cuft
- Divide cuft by 27 cuft/cuyd to get cubic yards
380 cuft ÷ 27 cuft/cuyd = 14 cuyd
- Multiply cubic yards by 1.4 to get weight of rock in tons
14 cuyd x 1.4 ton/cuyd = 19.7 tons

F. SEWAGE ABSORPTION WIDTH

Absorption width equals absorption ratio (See Figure D-33)
 times rock layer width (D2)

10.380 x 1.5 ft = 15 ft

D-33: Absorption Width Sizing Table

Percolation Rate in Minutes per Inch (MPI)	Soil Texture	Loading Rate Gallons per day per square foot	Absorption Ratio
Faster than 5	Coarse Sand Medium Sand Loamy Sand Fine Sand	1.20	1.00
6 to 15	Sandy Loam	0.79	1.50
16 to 30	Loam	0.60	2.00
31 to 45	Silt Loam	0.50	2.40
46 to 60	Silt Sandy Clay Loam Silty Clay Loam Clay Loam	0.45	2.67
61 to 120	Silty Clay Sandy Clay Clay	0.24	5.00
Slower than 120*			

*System designed for these soils must be other or performance

G. MOUND SLOPE WIDTH & LENGTH

(landslope greater than 1%)

1. Downslope absorption width = absorption width (F) minus rock layer width (D2)

$10 \text{ ft} - 15 \text{ ft} = 5 \text{ ft}$

2. Calculate mound size
UPSLOPE

a. Depth of clean sand fill at upslope edge of rock layer = 3 ft minus the distance to restricting layer (C1)

$3 \text{ ft} - 1.34 \text{ ft} = 1.66 \text{ ft}$

b. Mound height at the upslope edge of rock layer = depth of clean sand for separation (G2a) at upslope edge plus depth of rock layer (1 ft) plus depth of cover (1 ft)

$1.66 \text{ ft} + 1 \text{ ft} + 1 \text{ ft} = 3.66 \text{ ft}$

c. Upslope berm multiplier based on land slope

2.48 (see figure D-34)

d. Upslope width = berm multiplier (G2c) x upslope mound height (G2b):

$3.66 \times 2.48 = 9 \text{ ft}$

DOWNSLOPE

e. Drop in elevation = rock layer width (D2) x percent landslope (C5) ÷ 100

$10 \text{ ft} \times 7\% \div 100 = 0.7 \text{ ft}$

f. Downslope mound height = depth of clean sand for slope difference (G2e) at downslope rock edge plus the mound height at the upslope edge of rock layer (G2b)

$7 \text{ ft} + 1.66 \text{ ft} = 2.36 \text{ ft}$

g. Downslope berm multiplier based on percent land slc

3.80 (see figure D-34)

h. Downslope width = downslope multiplier (G2g) times downslope mound height (G2f)

$3.80 \times 2.36 \text{ ft} = 9 \text{ ft}$

i. Select the greater of G1 and G2h as the downslope width:

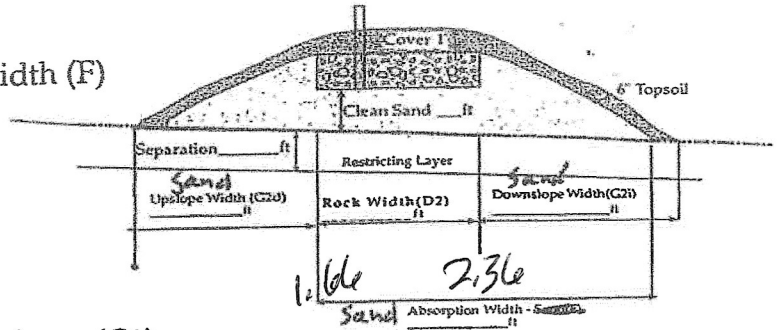
9 ft

j. Total mound width is the sum of upslope width (G2d) width plus rock layer width (D2) plus downslope width (G2i)

$9 \text{ ft} + 9 \text{ ft} + 10 \text{ ft} = 28 \text{ ft}$

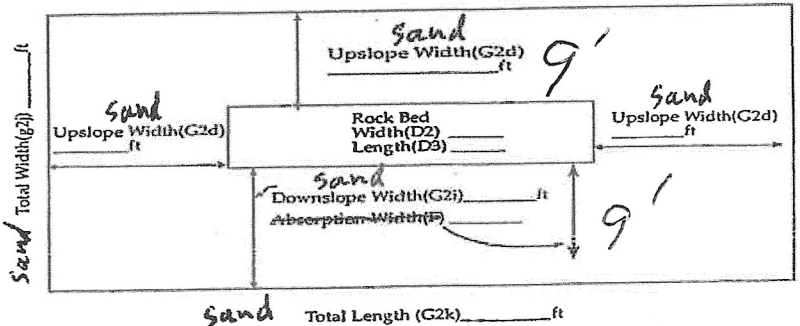
k. Total mound length is the sum of upslope width (G2d) plus rock layer length (D3) plus upslope width (G2d)

$9 \text{ ft} + 9 \text{ ft} + 38 \text{ ft} = 56 \text{ feet}$



D-34: SLOPE MULTIPLIER TABLE

Land Slope in %	UPSLOPE multipliers for various slope ratios						DOWNSLOPE multipliers for various slope ratios				
	3:1	4:1	5:1	6:1	7:1	8:1	3:1	4:1	5:1	6:1	7:1
0	3.0	4.0	5.0	6.0	7.0	8.0	3.0	4.0	5.0	6.0	7.0
1	2.91	3.85	4.76	5.66	6.54	7.41	3.09	4.17	5.26	6.38	7.53
2	2.83	3.70	4.54	5.36	6.14	6.90	3.19	4.35	5.56	6.82	8.14
3	2.75	3.57	4.35	5.08	5.79	6.45	3.30	4.54	5.88	7.32	8.86
4	2.68	3.45	4.17	4.84	5.46	6.06	3.41	4.76	6.25	7.89	9.72
5	2.61	3.33	4.00	4.62	5.19	5.71	3.53	5.00	6.67	8.57	10.77
6	2.54	3.23	3.85	4.41	4.93	5.41	3.66	5.26	7.14	9.38	12.07
7	2.48	3.12	3.70	4.23	4.70	5.13	3.80	5.56	7.69	10.34	13.73
8	2.42	3.03	3.57	4.05	4.49	4.88	3.95	5.88	8.33	11.54	15.91
9	2.36	2.94	3.45	3.90	4.30	4.65	4.11	6.25	9.09	13.04	18.92
10	2.31	2.86	3.33	3.75	4.12	4.44	4.29	6.67	10.00	15.00	23.33
11	2.26	2.78	3.23	3.61	3.95	4.26	4.48	7.14	11.11	17.65	30.43
12	2.21	2.70	3.12	3.49	3.80	4.08	4.69	7.69	12.50	21.43	43.75



Final Dimensions;
28 x 56

I hereby certify that I have completed this work in accordance with applicable ordinances, rules and laws.

[Signature]

(signature)

2205

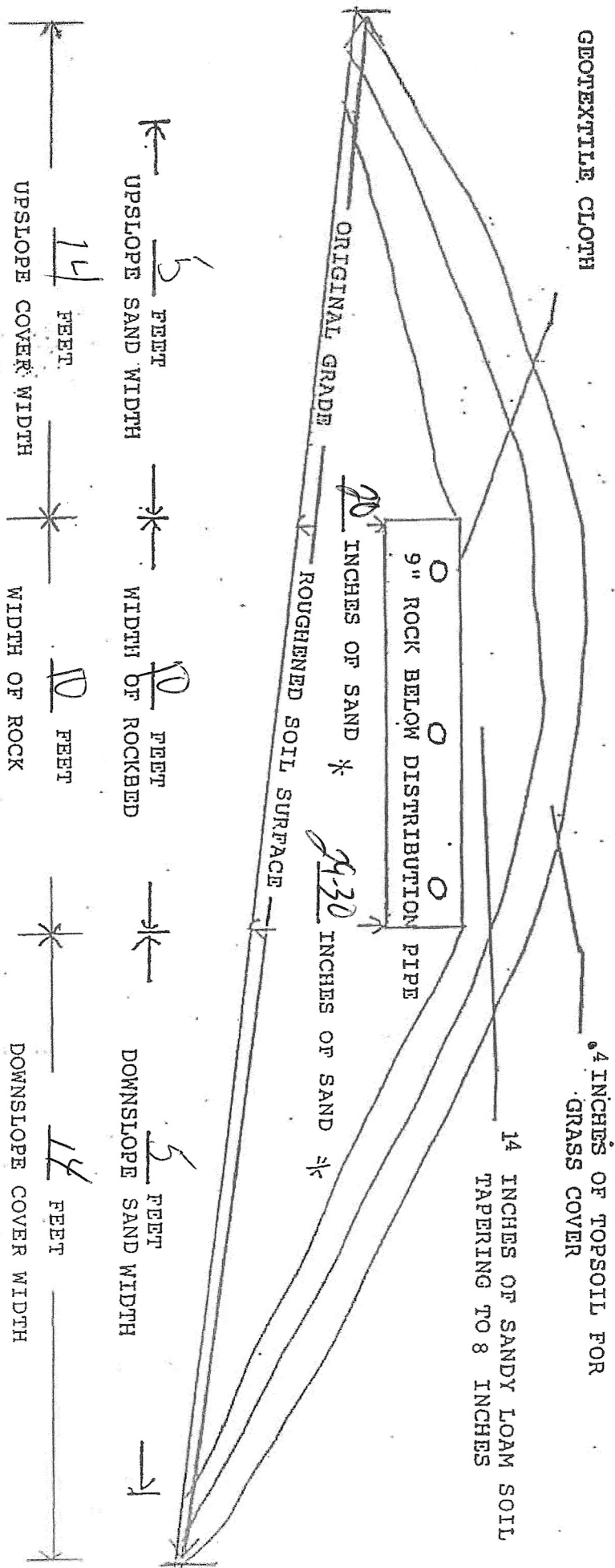
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3/10/24

(date)

GROUND CROSS-SECTION

7 PERCENT SLOPE OF ORIGINAL SOIL 10 FT. x 38 FT. SIZE OF ROCKBED 15 FT. x 48 FT. SIZE OF SANDBASE



4 INCHES OF TOPSOIL FOR GRASS COVER

14 INCHES OF SANDY LOAM SOIL TAPERING TO 8 INCHES

20 INCHES OF SAND
 24-30 INCHES OF SAND

5 FEET UPSLOPE SAND WIDTH
 14 FEET UPSLOPE COVER WIDTH
 10 FEET WIDTH OF ROCKBED
 10 FEET WIDTH OF ROCK
 5 FEET DOWNSLOPE SAND WIDTH
 14 FEET DOWNSLOPE COVER WIDTH

56

PUMP SELECTION PROCEDURE

1. Determine pump capacity:

A. Gravity distribution

1. Minimum required discharge is 10 gpm
2. Maximum suggested discharge is 45 gpm. For other establishments at least 10% greater than the water supply rate, but no faster than the rate at which effluent will flow out of the distribution device.

B. Pressure distribution

See pressure distribution work sheet

From A or B Selected pump capacity: _____ gpm

2. Determine pump head requirements:

A. Elevation difference between pump and point of discharge?

8 feet

B. Special head requirement? (See Figure at right - Special Head Requirements)

5 feet

C. Calculate Friction loss

1. Select pipe diameter 2 in

2. Enter Figure E-9 with gpm (1A or B) and pipe diameter (C1).

Read friction loss in feet per 100 feet from Figure E-9

Friction Loss = 1.55 ft/100ft of pipe

3. Determine total pipe length from pump discharge to soil treatment discharge point. Estimate by adding 25 percent to pipe length for fitting loss. Total pipe length times 1.25 = equivalent pipe length

60 feet x 1.25 = 75 feet

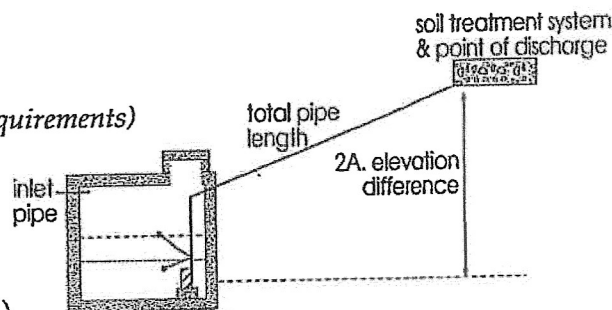
4. Calculate total friction loss by multiplying friction loss (C2) in ft/100 ft by the equivalent pipe length (C3) and divide by 100.

= 1.55 ft/100ft x 75 ÷ 100 = 1.16 ft

D. Total head required is the sum of elevation difference (A), special head requirements (B), and total friction loss (C4)

13 ft + 1.8 ft + _____ ft =

Total head: 14.8 feet



Special Head Requirements	
Gravity Distribution	0 ft
Pressure Distribution	5 ft

flow rate gpm	nominal pipe diameter		
	1.5"	2"	3"
20	2.47	0.73	0.11
25	3.73	1.11	0.16
30	5.23	1.55	0.23
35	6.96	2.06	0.30
40	8.91	2.64	0.39
45	11.07	3.28	0.48
50	13.46	3.99	0.58
55		4.76	0.70
60		5.60	0.82
65		6.48	0.95
70		7.44	1.09

3. Pump selection

A pump must be selected to deliver at least 28.86 gpm
(1A or B) with at least 14.8 feet of total head (2D)

I hereby certify that I have completed this work in accordance with applicable ordinances, rules and laws.

Daniel Lopez

(signature)

2205

(license #)

3/11/24

(date)

PRESSURE DISTRIBUTION SYSTEM

- Select number of perforated laterals 3
- Select perforation spacing = 3 ft
- Since perforations should not be placed closer than 1 foot to the edge of the rock layer (see diagram), subtract 2 feet from the rock layer length.

$$\frac{38}{\text{Rock layer length}} - 2 \text{ ft} = \underline{36} \text{ ft}$$

- Determine the number of spaces between perforations. Divide the length (3) by perforation spacing (2) and round down to nearest whole number.

$$\text{Perforation spacing} = \underline{36} \text{ ft} \div \underline{3} \text{ ft} = \underline{12} \text{ spaces}$$

- Number of perforations is equal to one plus the number of perforation spaces (4). Check figure E-4 to assure the number of perforations per lateral guarantees <10% discharge variation.

$$\underline{12} \text{ spaces} + 1 = \underline{13} \text{ perforations/lateral}$$

- A. Total number of perforations = perforations per lateral (5) times number of laterals (1)

$$\underline{13} \text{ perfs/lat} \times \underline{3} \text{ lat} = \underline{39} \text{ perforations}$$

- B. Calculate the square footage per perforation. Should be 6-10 sqft/perf. Does not apply to at-grades.

$$\text{Rock bed area} = \text{rock width (ft)} \times \text{rock length (ft)}$$

$$\underline{10} \text{ ft} \times \underline{38} \text{ ft} = \underline{380} \text{ sqft}$$

$$\text{Square foot per perforation} = \text{Rock bed area} \div \text{number of perfs (6)}$$

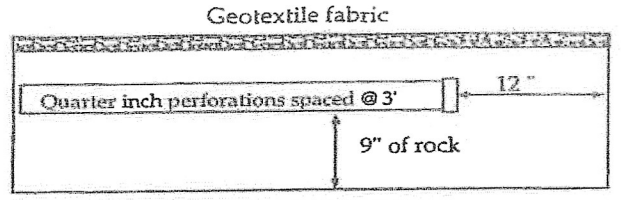
$$\underline{380} \text{ sqft} \div \underline{39} \text{ perfs} = \underline{9} \text{ sqft/perf}$$

- Determine required flow rate by multiplying the total number of perforations (6A) by flow per perforation (see figure E-6)

$$\underline{39} \text{ perfs} \times \underline{74} \text{ gpm/perfs} = \underline{2886} \text{ gpm}$$

- If laterals are connected to header pipe as shown on upper example, to select minimum required lateral diameter; enter figure E-4 with perforation spacing (2) and number of perforations per lateral (5) Select minimum diameter for perforated lateral = 1.25 inches.

- If perforated lateral system is attached to manifold pipe near the center, lower diagram, perforated lateral length (3) and number of perforations per lateral (5) will be approximately one half of that in step 8. Using these values, select minimum diameter for perforated lateral = 1 inches.



Perf Sizing 3/16" - 1/4"
Perf Spacing 1.5' - 5'

E-4: Maximum allowable number of 1/4-inch perforations per lateral to guarantee <10% discharge variation

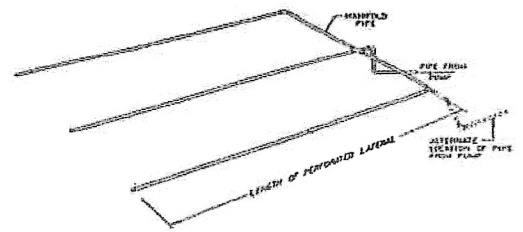
perforation spacing (feet)	1 inch	1.25 inch	1.5 inch	2.0 inch
2.5	8	14	18	28
3.0	8	<u>13</u>	17	26
3.3	7	12	16	25
4.0	7	11	15	23
5.0	6	10	14	22

E-6: Perforation Discharge in gpm

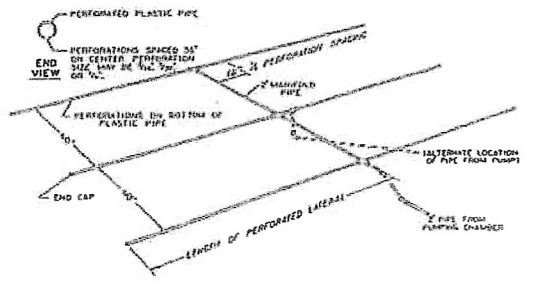
head (feet)	perforation diameter (inches)			
	1/8	3/16	7/32	1/4
1.0 ^a	0.18	0.42	0.56	<u>0.74</u>
2.0 ^b	0.26	0.59	0.80	1.04
5.0	0.41	0.94	1.26	1.65

^a Use 1.0 foot for single-family homes.
^b Use 2.0 feet for anything else.

MANIFOLD LOCATED AT END OF PRESSURE DISTRIBUTION SYSTEM



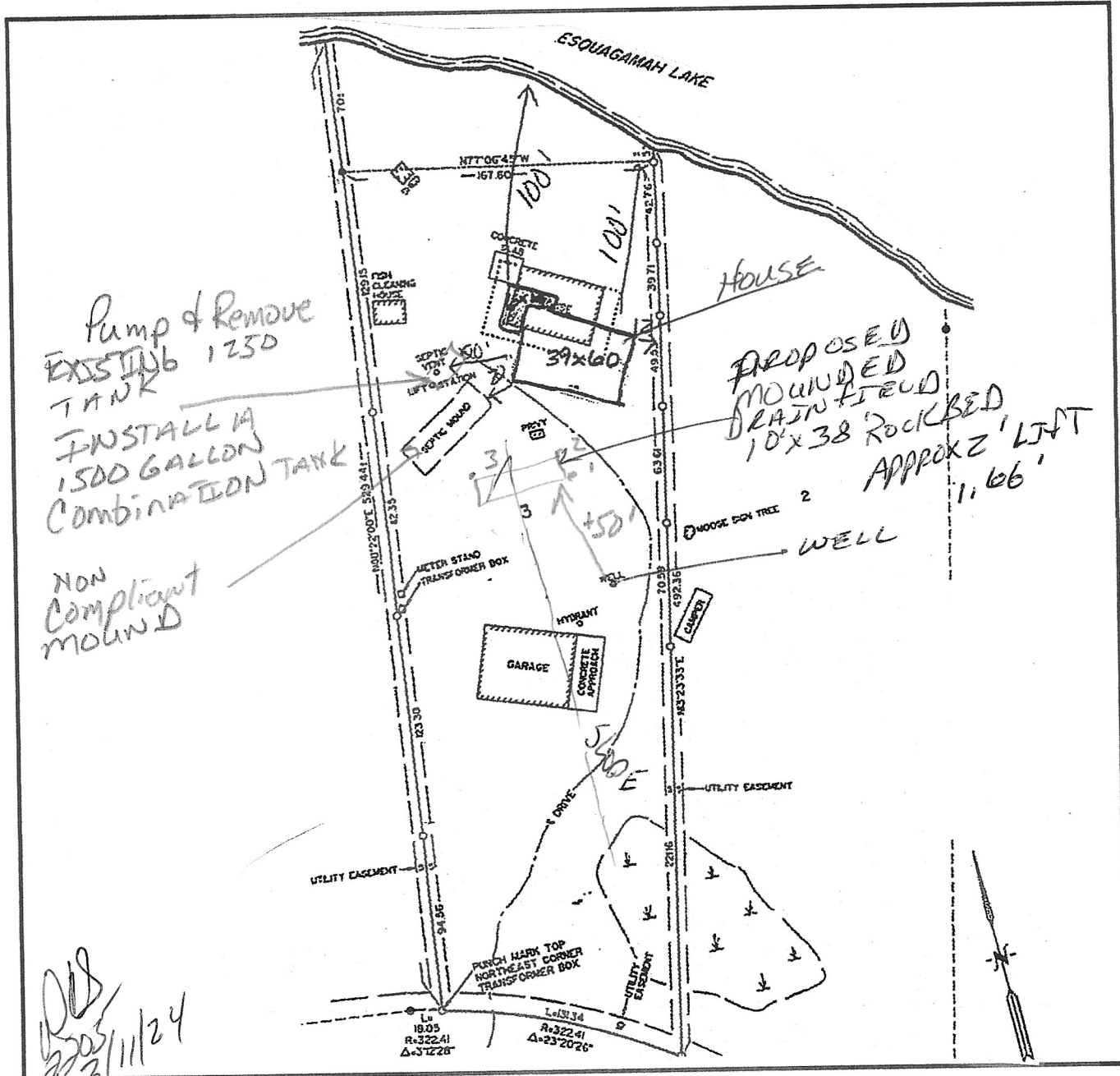
LAYOUT OF PERFORATED PIPE LATERALS FOR PRESSURE DISTRIBUTION IN MOUND



I hereby certify that I have completed this work in accordance with applicable ordinances, rules and laws.

Daniel E. [Signature] (signature) 2205 (license #) 3/11/24 (date)

Property Owner/Client:



Map scale:

Indicated north

Show slope/contours

Elevations in feet

Benchmark: ft

System Corners:

Soil Observation:

NW:	<input type="text"/>	ft
NE:	<input type="text"/>	ft
SW:	<input type="text"/>	ft
SE:	<input type="text"/>	ft

#1:	<input type="text"/>	ft
#2:	<input type="text"/>	ft
#3:	<input type="text"/>	ft
#4:	<input type="text"/>	ft

Tank Outlet:	<input type="text"/>	ft
Other:	<input type="text"/>	ft
	<input type="text"/>	ft

Date Completed:

Handwritten signature and date:
 [Signature]
 11/1/24