

**Becklin & Whitney**  
*Consulting Engineers, Inc.*  
**139 1<sup>ST</sup> AVE. E, SUITE 100**  
**P. O. Box 471**  
**CAMBRIDGE, MN 55008**  
**PHONE (763) 689-5631 FAX (763) 552-5631**

October 19, 2023

Todd Bohnen  
37707 State Hwy. 18  
Aitkin, MN

RE: Review of Proposed Addition adjacent to  
Septic System Box Mound

To Whom It May Concern:

**Introduction**

The owner is planning to build onto the south side of the existing cabin. A Septic System Box Mound is also being installed on west side of property. The Septic Tank is existing. We were asked to comment about any effects on septic system that would be caused by cabin addition.

**Details**

The existing cabin and proposed addition have a 2 foot deep foundation. The existing Septic Tank is 10 feet from building and bottom of Tank is 6 feet below grade. The new Septic Pump Tank will be over 10 feet from building. The cabin is 25 to 28 feet from west property line. The Septic System Box Mound edge will be at least 15 feet from cabin.

**Discussion**

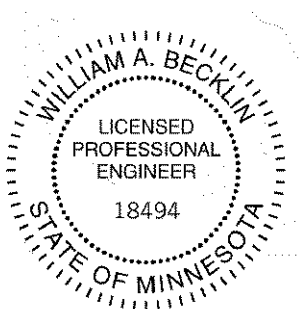
The existing Septic Tank is outside the influence zone of the cabin foundation and will not be affected by the addition. The new Pump Tank will be further away from the cabin and is approved. The Septic System Box Mound will not be affected by the addition to the cabin.

**Conclusions**

**The existing Septic Tank and proposed Pump Tank are outside the influence zone of the cabin foundation and are approved. The Septic System Box Mound is at least 15 feet away from cabin addition and will not be affected by the cabin foundation and is approved. Positive drainage must be maintained between cabin and Septic System.**

**Attachments:**

**Survey, Sketch, Septic Design**



I HEREBY CERTIFY THAT THIS PLAN, SPECIFICATION, OR REPORT WAS PREPARED BY ME OR UNDER MY DIRECT SUPERVISION AND THAT I AM A DULY REGISTERED PROFESSIONAL ENGINEER UNDER THE LAWS OF THE STATE OF MINNESOTA.

*William A Becklin*

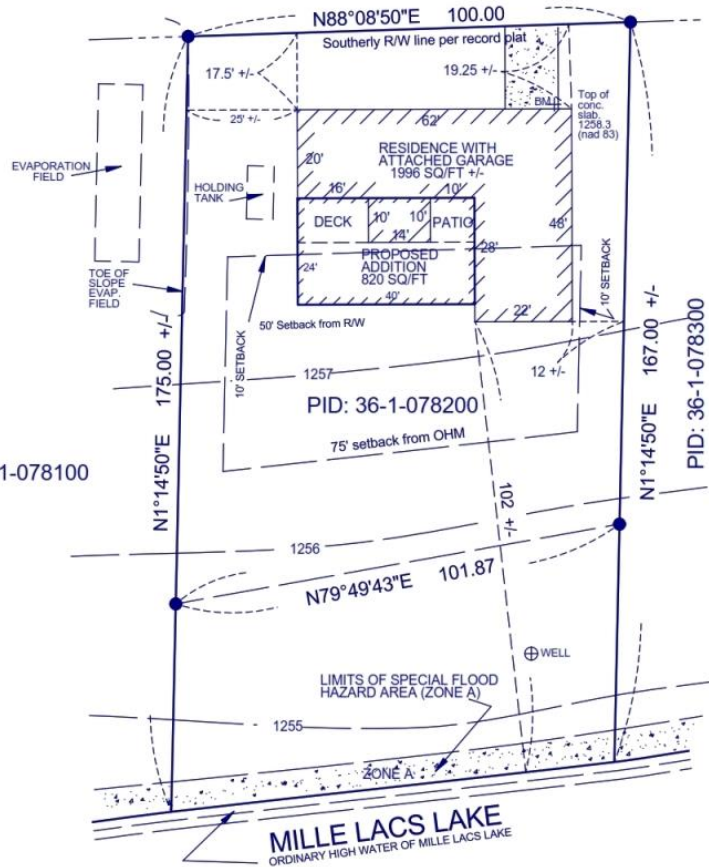
WILLIAM A. BECKLIN, P.E.

DATE: OCTOBER 19, 2023

LIC. No. 18494

# Certificate of Survey Of Lot 10, Ely's Lakeview Lots, Aitkin County, Minnesota.

- 1.) OHW line of MILLE LACS LAKE: 1252.8 NGVD 29 (ft) 1253.3 (NAD 83)
- 2.) As per Contours shown, NO BLUFF as defined by ordinance.
- 3.) Existing Flood Insurance Rate Map indicates that subject structures & proposed Septic Systems are located in ZONE C (X). 2706280400C. ( i.e. NOT IN A SFHA).
- 4.) Square footage of existing structures are shown.
- 5.) Dimensions of existing structures from boundaries are shown.
- 6.) Wetlands if any, will be delineated by Brinks Wetland Services, LLC
- 7.) Topographic contours are shown.
- 8.) Total area: 17,100 sq/ft +/-
- 9.) Existing impervious coverage: 2506 sq/ft +/- (15%)
- 10.) SITE BENCHMARK: Top of SE CORNER OF CONC. SLAB 1258.3 NAD 83 (FT)
- 11.) Existing structure coverage: 2256 sq/ft +/- (12%)
- 12.) TAXPAYER: NANCY THOMPSON & TODD BOHNEN Document No. A468391
- 13.) Proposed Impervious coverage with improvements: 3045 sq/ft +/- (18%).
- 14.) MILLE LACS Lake is classified as a GENERALI Development Lake [GD]
- 15.) PARCEL NO: 36-1-078200
- 16.) PROPERTY ADDRESS: 37707 STATE HIGHWAY 18 AITKIN MN 56431



PID: 36-1-078100

Terry J. Betley  
Land Surveyor  
Aitkin County Abstract Company Building  
112 Third Street Northwest  
Aitkin, Minnesota 56431

Graphic Scale: 1 inch = 30 feet.



Bearing Datum is Local Assumed.

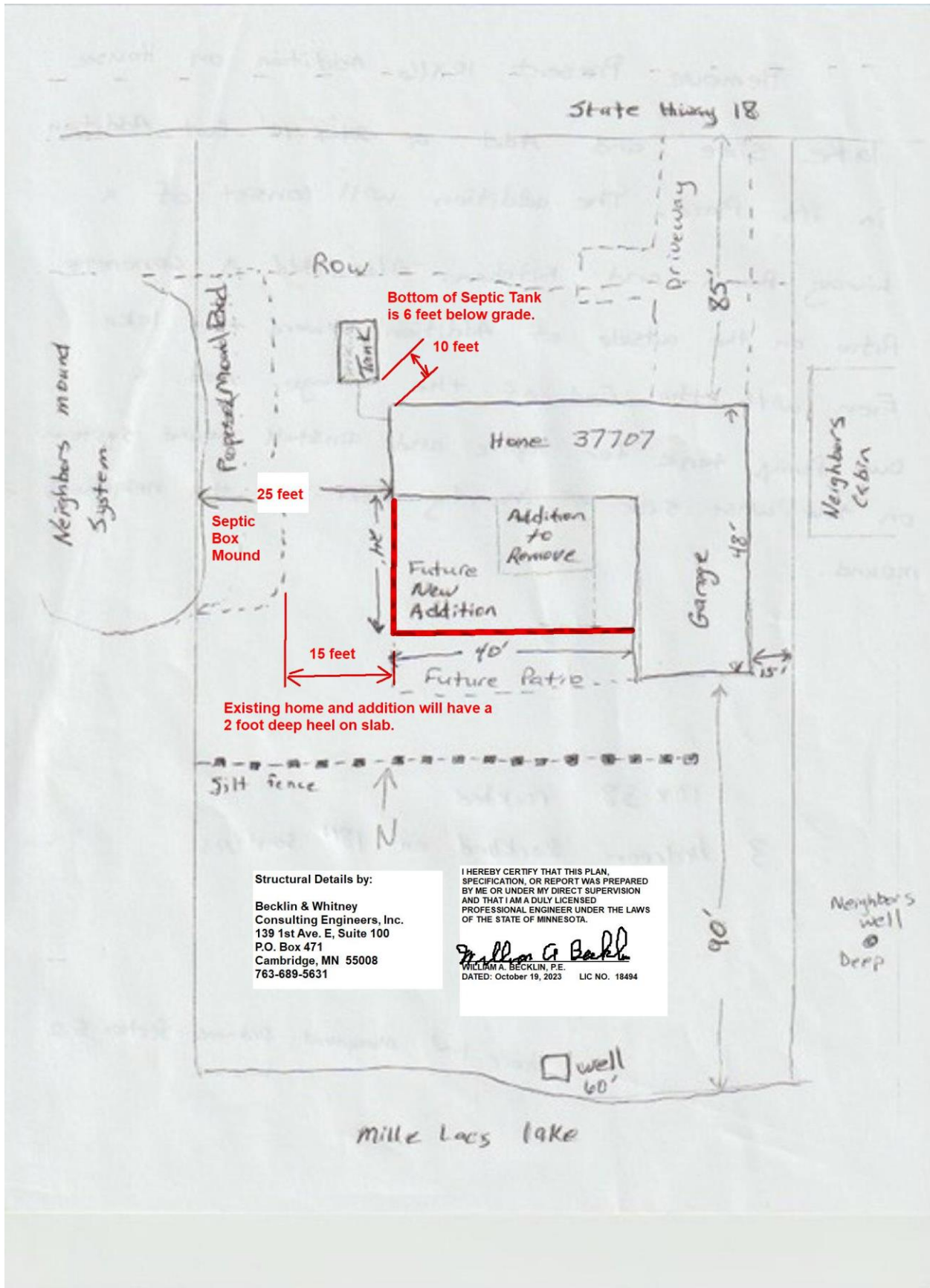
- Denotes iron monument from prior surveys.
- Denotes set iron monument.

I hereby certify that this plan, survey or report was prepared by me or under my direct supervision and that I am a duly licensed land surveyor & abstractor under the laws of the State of Minnesota.

*Terry J. Betley*  
... electronically generated ...

Terry J. Betley, Minnesota Registration No. 15811 & 19  
Date: AUGUST 14, 2023





Bottom of Septic Tank is 6 feet below grade.

10 feet

25 feet

Septic Box Mound

15 feet

Existing home and addition will have a 2 foot deep heel on slab.

Structural Details by:  
 Becklin & Whitney  
 Consulting Engineers, Inc.  
 139 1st Ave. E, Suite 100  
 P.O. Box 471  
 Cambridge, MN 55008  
 763-689-5631

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 William A. Becklin  
 WILLIAM A. BECKLIN, P.E.  
 DATED: October 19, 2023 LIC NO. 18494

Neighbors well Deep

well 60'

mille Laes lake

FIELD EVALUATION SHEET

PRELIMINARY EVALUATION DATE 7-29-23 FIELD EVALUATION DATE 7-29-23  
PROPERTY OWNER: Todd Bohner PHONE 651-747-7457  
ADDRESS: 37707 St. Hwy. 18 CITY, STATE, ZIP: Aitkin 56431  
LEGAL DESCRIPTION: Lot 10 Elys Lake View Lots  
PIN# 36-1-078200 SEC 21 T 45 R 26 TWP NAME Weathered twp  
FIRE# LAKE/RIVER Mill Lake LAKE CLASS 60 OHWL FT.

DESCRIPTION OF SOIL TREATMENT AREAS

AREA #1 AREA #2 REFERENCE BM ELEV. FT  
DISTURBED AREAS YES NOX YES NO REFERENCE BM DESCRIPTION  
COMPACTED AREAS YES NOX YES NO  
FLOODING YES NOX YES NO  
RUN ON POTENTIAL YES NOX YES NO  
SLOPE % 0  
DIRECTION OF SLOPE 0  
LANDSCAPE POSITION Level  
VEGETATION TYPES grass yard

DEPTH TO STANDING WATER OR MOTTLED SOIL: BORING# 1 20", 1A, 2 21", 2A

BOTTOM ELEVATION-FIRST TRENCH OR BOTTOM OF ROCK BED: #1 FT., #2 FT.

SOIL SIZING FACTOR: SITE #1 1.27, SITE #2

CONSTRUCTION RELATED ISSUES: add 500 Pump tank plus 10'x38' Rock Bed on 18" Sand Base and Box in 14'x42' Field

LIC# 2088 SITE EVALUATOR SIGNATURE: Bob Bartel

SITE EVALUATOR NAME: Bob Bartel TELEPHONE# 218-831-2431

LUG REVIEW DATE

Comments:

SOIL BORING LOGS ON REVERSE SIDE

36-1-078207  
SOILS CHARTS FOR BOTH PROPOSED AND ALTERNATE SITES

1 (PROPOSED) SOILS DATA

| DEPTH<br>(INCHES) | TEXTURE | MUNSELL<br>COLOR |
|-------------------|---------|------------------|
| 4"                | Topsoil | 10R 3/3          |
| 1<br>20"          | Sub     | 10R 4/4          |
| 0                 | mottles | 10R 4/2          |

2 (PROPOSED) SOILS DATA

| DEPTH<br>(INCHES) | TEXTURE | MUNSELL<br>COLOR |
|-------------------|---------|------------------|
| 4"                | Topsoil | 10R 3/3          |
| 1<br>21"          | Sub     | 10R 4/4          |
| 2                 | mottles | 10R 4/2          |

1 (ALTERNATE) SOILS DATA

| DEPTH<br>(INCHES) | TEXTURE | MUNSELL<br>COLOR |
|-------------------|---------|------------------|
|                   |         |                  |

2 (ALTERNATE) SOILS DATA

| DEPTH<br>(INCHES) | TEXTURE | MUNSELL<br>COLOR |
|-------------------|---------|------------------|
|                   |         |                  |

ADDITIONAL SOIL BORINGS MAY BE REQUIRED

36-1-07 8200

MOUND DESIGN WORK SHEET (For Flows up to 1200 gpd)

A. Average Design FLOW

Estimated 450 gpd (see figure A-1)  
 or measured \_\_\_\_\_ x 1.5 (safety factor) = \_\_\_\_\_ gpd

A-1: Estimated Sewage Flows in Gallons per Day

| number of bedrooms | Class I | Class II | Class III | Class IV   |
|--------------------|---------|----------|-----------|------------|
| 2                  | 300     | 225      | 180       | 60%        |
| 3                  | 450     | 300      | 218       | of the     |
| 4                  | 600     | 375      | 256       | values     |
| 5                  | 750     | 450      | 294       | in the     |
| 6                  | 900     | 525      | 332       | Class I,   |
| 7                  | 1050    | 600      | 370       | II, or III |
| 8                  | 1200    | 675      | 408       | columns.   |

B. SEPTIC TANK Capacity

1000 Exist.  
500 Left gallons (see figure C-1)  
 Now

C-1: Septic Tank Capacities (in gallons)

| Number of Bedrooms | Minimum Liquid Capacity | Liquid capacity with garbage disposal | Liquid capacity with disposal & lift inside |
|--------------------|-------------------------|---------------------------------------|---|
| 2 or less          | 750                     | 1125                                  | 1500  |
| 3 or 4             | 1000                    | 1500                                  | 2000  |
| 5 or 6             | 1500                    | 2250                                  | 3000  |
| 7, 8 or 9          | 2000                    | 3000                                  | 4000  |

C. SOILS (refer to site evaluation)

- Depth to restricting layer = 18" feet
- Depth of percolation tests = \_\_\_\_\_ feet
- Texture \_\_\_\_\_  
 Percolation rate \_\_\_\_\_ mpi
- Soil loading rate 1.27 gpd/sqft (see figure D-33)
- Percent land slope 0 %

D. ROCK LAYER DIMENSIONS

- Multiply average design flow (A) by 0.83 to obtain required rock layer area.  
450 gpd x 0.83 sqft/gpd = 380 sqft
- Determine rock layer width = 0.83 sqft/gpd x linear Loading Rate (LLR)  
 0.83 sqft/gpd x \_\_\_\_\_ gpd/sqft = \_\_\_\_\_ ft
- Length of rock layer = area ÷ width =  
380 sqft (D1) ÷ 10 ft (D2) = 38 ft

|           |      |
|-----------|------|
| Mound LLR |      |
| < 120 MPI | ≤ 12 |
| ≥ 120 MPI | ≤ 6  |

E. ROCK VOLUME

- Multiply rock area (D1) by rock depth of 1 ft to get cubic feet of rock  
380 sqft x 1 ft = 380 cuft
- Divide cuft by 27 cuft/cuyd to get cubic yards  
380 cuft ÷ 27 cuyd/cuft = 15 cuyd
- Multiply cubic yards by 1.4 to get weight of rock in tons  
15 cuyd x 1.4 ton/cuyd = 20 tons

F. SEWAGE ABSORPTION WIDTH

Absorption width equals absorption ratio (See Figure D-33) times rock layer width (D2)

1.50 x 10 ft = 15 ft

D-33: Absorption Width Sizing Table

| Percolation Rate in Minutes per Inch (MPI) | Soil Texture  | Loading Rate Gallons per day per square foot | Absorption Ratio |
|--|---|--|------------------|
| Faster than 5                              | Coarse Sand<br>Medium Sand<br>Loamy Sand<br>Fine Sand   | 1.20   | 1.00             |
| 6 to 15                                    | Sandy Loam  | 0.72   | 1.30             |
| 16 to 30                                   | Loam  | 0.60   | 2.00             |
| 31 to 45                                   | Silt Loam   | 0.50   | 2.40             |
| 46 to 60                                   | Silt<br>Sandy Clay Loam<br>Silty Clay Loam<br>Clay Loam | 0.45   | 2.67             |
| 61 to 120                                  | Silty Clay<br>Sandy Clay<br>Clay                        | 0.24   | 5.00             |
| Slower than 120*                           |   |  |                  |

\*System designed for these soils must be other or performance

36-1-678200  
 <=1% land slope

G. Mound Slope Width and Length  
 (landslope less than or equal to 1%)

1. Absorption width (F) 15 ft

2. Calculate mound size

a. Determine depth of clean sand fill

at upslope edge of rock layer = 3 ft

minus the distance to restricting layer (C1)

3 ft - 18" ft = 18" ft

b. Mound height at the upslope edge of rock layer = depth of clean sand for separation (G2a)

at upslope edge plus depth of rock layer (1 ft) plus depth of cover (1 ft)

1.6 ft + 1ft + 1ft = 3.6 ft

c. Berm width = upslope mound height (G2b) times 4 (4 is recommended, but could be 3-12)

3.6 x 4 = 14.4 ft *Box around*

d. The total landscape width is the sum of berm (G2c) width plus rock layer width (D2) plus berm width (G2c): 2 ft + 10 ft + 2 ft = 14 ft 14' x 42'

e. Additional width necessary for absorption = absorption width (F) minus the landscape width (G2d)

15 ft - 14 ft = 1 ft, if number is negative (<0) skip to g

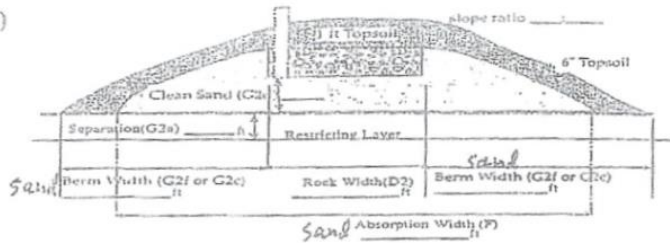
f. Final berm width = additional width (G2e) plus the berm width (G2c)

1 ft + 2 ft = 3 ft

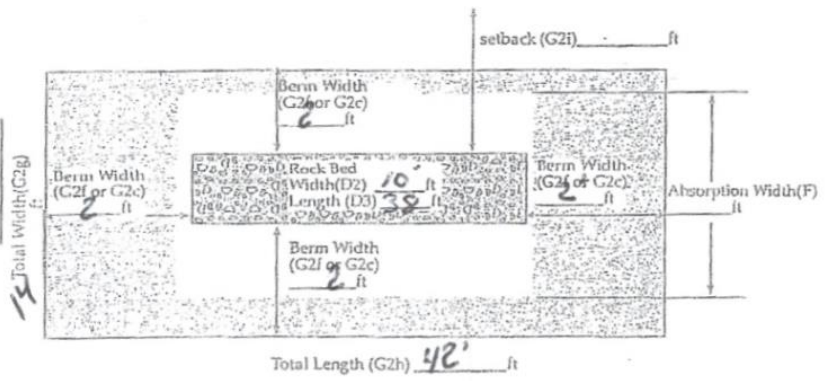
g. Total mound width is the sum of berm width (G2f or G2c) plus rock layer width (D2) plus berm width (G2f or G2c): 3 ft + 10 ft + 3 ft = 16 ft

h. Total mound length is the sum of berm (G2f or G2c) plus rock layer length (D3) plus berm (G2f or G2c): 2 ft + 38 ft + 2 ft = 42 ft

i. Setbacks from the rockbed are calculated as follows: the absorption width (F) minus the rock bed width (D2) divided by 2: (15 ft - 10 ft) ÷ 2 = 2.5 ft



**Final Dimensions:**  
14 x 42



I hereby certify that I have completed this work in accordance with applicable ordinances, rules and laws.

Bob Bull (signature) 2088 (license #) 8-11-23 (date)

36-1-078200

PRESSURE DISTRIBUTION SYSTEM

- Select number of perforated laterals 3
- Select perforation spacing = 3 ft
- Since perforations should not be placed closer than 1 foot to the edge of the rock layer (see diagram), subtract 2 feet from the rock layer length.

$\frac{38}{\text{Rock layer length}} - 2 \text{ ft} = 36 \text{ ft}$

- Determine the number of spaces between perforations. Divide the length (3) by perforation spacing (2) and round down to nearest whole number.

Perforation spacing =  $36 \text{ ft} \div 3 = 12$  spaces

- Number of perforations is equal to one plus the number of perforation spaces(4). Check figure E-4 to assure the number of perforations per lateral guarantees <10% discharge variation.

$12 \text{ spaces} + 1 = 13$  perforations/lateral

- A. Total number of perforations = perforations per lateral (5) times number of laterals (1)

$13 \text{ perms/lat} \times 3 \text{ lat} = 39$  perforations

- B. Calculate the square footage per perforation. Should be 6-10 sqft/perf. Does not apply to at-grades.

Rock bed area = rock width (ft) x rock length (ft)

$10 \text{ ft} \times 38 \text{ ft} = 380 \text{ sqft}$

Square foot per perforation = Rock bed area + number of perms (6)

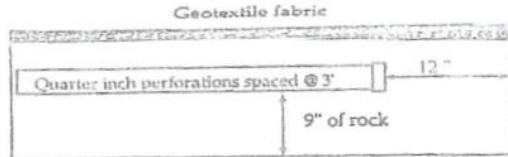
$380 \text{ sqft} \div 39 \text{ perms} = 9.7 \text{ sqft/perf}$

- Determine required flow rate by multiplying the total number of perforations (6A) by flow per perforation (see figure E-6)

$39 \text{ perms} \times .74 \text{ gpm/perf} = 29 \text{ gpm}$

- If laterals are connected to header pipe as shown on upper example, to select minimum required lateral diameter; enter figure E-4 with perforation spacing (2) and number of perforations per lateral (5) Select minimum diameter for perforated lateral = 2 inches.

- If perforated lateral system is attached to manifold pipe near the center, lower diagram, perforated lateral length (3) and number of perforations per lateral (5) will be approximately one half of that in step 8. Using these values, select minimum diameter for perforated lateral = \_\_\_\_\_ inches.



E-4: Maximum allowable number of 1/4-inch perforations per lateral to guarantee <10% discharge variation

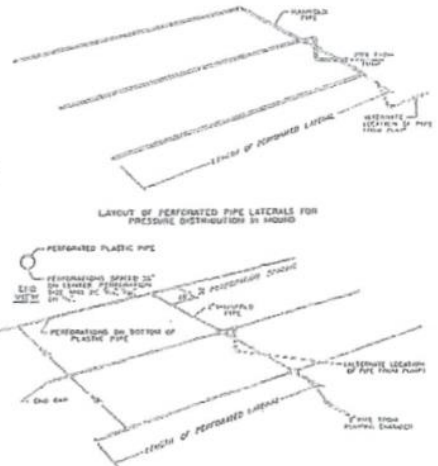
| perforation spacing (feet) | 1 inch | 1.25 inch | 1.5 inch | 2.0 inch |
|----------------------------|--------|-----------|----------|----------|
| 2.5                        | 8      | 14        | 18       | 26       |
| 3.0                        | 8      | 13        | 17       | 26       |
| 3.3                        | 7      | 12        | 16       | 25       |
| 4.0                        | 7      | 11        | 15       | 23       |
| 5.0                        | 6      | 10        | 14       | 22       |

E-6: Perforation Discharge in gpm

| head (feet)      | perforation diameter (inches) |      |      |      |
|------------------|-------------------------------|------|------|------|
|                  | 1/8                           | 3/16 | 7/32 | 1/4  |
| 1.0 <sup>a</sup> | 0.18                          | 0.42 | 0.56 | 0.74 |
| 2.0 <sup>b</sup> | 0.26                          | 0.59 | 0.80 | 1.04 |
| 5.0              | 0.41                          | 0.94 | 1.26 | 1.65 |

<sup>a</sup> Use 1.0 foot for single-family homes.  
<sup>b</sup> Use 2.0 feet for anything else.

MANIFOLD LOCATED AT END OF PRESSURE DISTRIBUTION SYSTEM



I hereby certify that I have completed this work in accordance with applicable ordinances, rules and laws.

Bob Bauld (signature) 2088 (license #) 8-11-23 (date)



# PUMP SELECTION PROCEDURE

36-1-078200

1. Determine pump capacity:

A. Gravity distribution

1. Minimum required discharge is 10 gpm
2. Maximum suggested discharge is 45 gpm. For other establishments at least 10% greater than the water supply rate, but no faster than the rate at which effluent will flow out of the distribution device.

B. Pressure distribution

See pressure distribution work sheet

From A or B Selected pump capacity: 29 gpm

2. Determine pump head requirements:

A. Elevation difference between pump and point of discharge?

4 feet

B. Special head requirement? (See Figure at right - Special Head Requirements)

5 feet

C. Calculate Friction loss

1. Select pipe diameter 2 in

2. Enter Figure E-9 with gpm (1A or B) and pipe diameter (C1).

Read friction loss in feet per 100 feet from Figure E-9

Friction Loss = 1.55 ft/100ft of pipe

3. Determine total pipe length from pump discharge to soil treatment discharge point. Estimate by adding 25 percent to pipe length for fitting loss. Total pipe length times 1.25 = equivalent pipe length

30 feet x 1.25 = 37.5 feet

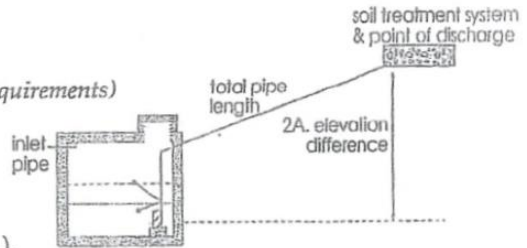
4. Calculate total friction loss by multiplying friction loss (C2) in ft/100 ft by the equivalent pipe length (C3) and divide by 100.

= 1.55 ft/100ft x 37.5 + 100 = .6 ft

D. Total head required is the sum of elevation difference (A), special head requirements (B), and total friction loss (C4)

4 ft + 5 ft + .6 ft =

Total head: 10 feet



| Special Head Requirements |      |
|---------------------------|------|
| Gravity Distribution      | 0 ft |
| Pressure Distribution     | 5 ft |

| flow rate<br>gpm | E-9: Friction Loss in Plastic Pipe<br>Per 100 feet |      |      |
|------------------|--|------|------|
|                  | nominal<br>pipe diameter                           |      |      |
|                  | 1.5"   | 2"   | 3"   |
| 20               | 2.47   | 0.73 | 0.11 |
| 25               | 3.73   | 1.11 | 0.16 |
| 30               | 5.23   | 1.55 | 0.23 |
| 35               | 6.96   | 2.06 | 0.30 |
| 40               | 8.91   | 2.64 | 0.39 |
| 45               | 11.07  | 3.28 | 0.48 |
| 50               | 13.46  | 3.99 | 0.58 |
| 55               |  | 4.76 | 0.70 |
| 60               |  | 5.60 | 0.82 |
| 65               |  | 6.48 | 0.95 |
| 70               |  | 7.44 | 1.09 |

**3. Pump selection**

A pump must be selected to deliver at least 29 gpm (1A or B) with at least 10 feet of total head (2D)

I hereby certify that I have completed this work in accordance with applicable ordinances, rules and laws.

Bob Bull (signature)

(signature)

2082 (license #)

(license #)

2088 (date)

(date)

# 36-1-078000 DOSING CHAMBER SIZING

1. Determine area

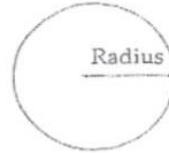
A. Rectangle area =  $L \times W$

\_\_\_\_\_ x \_\_\_\_\_ = \_\_\_\_\_ square feet

B. Circle area =  $\pi (3.14) \times \text{radius in feet} \times \text{radius in feet}$

3.14 x \_\_\_\_\_ ft x \_\_\_\_\_ ft = \_\_\_\_\_ sqft

C. Get area from manufacturer \_\_\_\_\_ sqft



2. Calculate gallons per inch

There are 7.5 gallons per cubic foot of volume, therefore multiply the area (1A, B or C) times the conversion factor and divide by 12 inches per foot to calculate gallon per inch.

Area x 7.5 ÷ 12 = \_\_\_\_\_ sqft x 7.5 ÷ 12 in/ft = 16.58 gallon per inch *mfj*

3. Calculate total tank volume

A. Depth from bottom of inlet pipe to tank bottom 5.22 mps = 31 1/8"

B. Total tank volume = depth from bottom of inlet pipe to tank bottom (3A) x gal/in (2)  
= 31.6 in x 16.58 gal/in = 522 gal

4. Calculate gallons to cover pump (with 2-3 inches of water covering pump)

(Pump and block height (inch) + 2 inch) x gallon/inch  
(12 in + 2 in) x 16.58 gal/in = 224 gallon

5. Calculate total pumpout volume

A. Select pump size for 4-5 does per day. Gallon per dose = gpd (see figure A-1)  
/ doses per day = 450 gpd ÷ 4 doses/day = 112.5 gallons

B. Calculate drainback

1. Determine total pipe length, 30 feet

2. Determine liquid volume of pipe, .17 gal per ft (see figure E-20)

3. Drainback quantity = 30 ft (5B1) x .17 gal per ft (5B2) = 5 gal

C. Total pump out volume = dose volume (5A) + drainback (5B3)  
112.5 gal + 5 gal = 117 Total gallon

6. Float separation distance (using total pumpout volume)

Total pumpout volume (5C) ÷ gal/inch (2)  
117 gal ÷ 16.58 gal/in = 7 inch

7. Calculate volume for alarm (typically 2 to 3 inches)

Alarm depth (inch) x gallon/inch (2) = 16.58 in x 2 gal/in = 33.16 gal

8. Calculate total gallon = gallons over pump (4) + gallons pumpout (5C) + gallons alarm (7)

224 gal + 117 gal + 33.16 gal = 374 gallons

9. Total Tank Depth = total gallon (8) ÷ gallon/inch (2)

374 gal ÷ 16.58 gal/in = 23 in

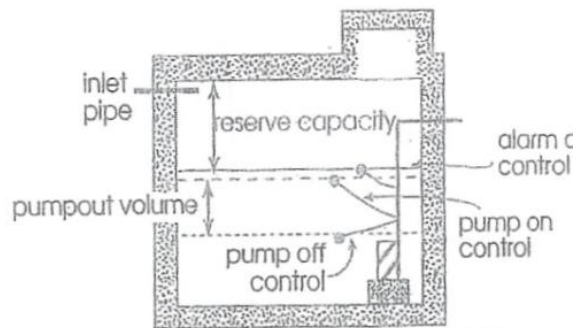
### Recommended:

Calculate reserve capacity (75% the daily flow)

Daily flow x .75 = 450 x .75 = 337.5 gallons

| number of bedrooms | Class I | Class II | Class III | Class IV   |
|--------------------|---------|----------|-----------|------------|
| 2                  | 300     | 225      | 180       | 60%        |
| 3                  | 450     | 300      | 216       | of the     |
| 4                  | 600     | 375      | 256       | values     |
| 5                  | 750     | 450      | 294       | in the     |
| 6                  | 900     | 525      | 332       | Class I,   |
| 7                  | 1050    | 600      | 370       | II, or III |
| 8                  | 1200    | 675      | 408       | columns.   |

| Pipe Diameter inches | Gallons per foot |
|----------------------|------------------|
| 1                    | 0.045            |
| 1.25                 | 0.078            |
| 1.5                  | 0.11             |
| 2                    | 0.17             |
| 2.5                  | 0.25             |
| 3                    | 0.38             |
| 4                    | 0.66             |



I hereby certify that I have completed this work in accordance with applicable ordinances, rules and laws.

Bob Burt (signature) 2088 (license #) 8-10-23 (date)

SKETCH SHEET

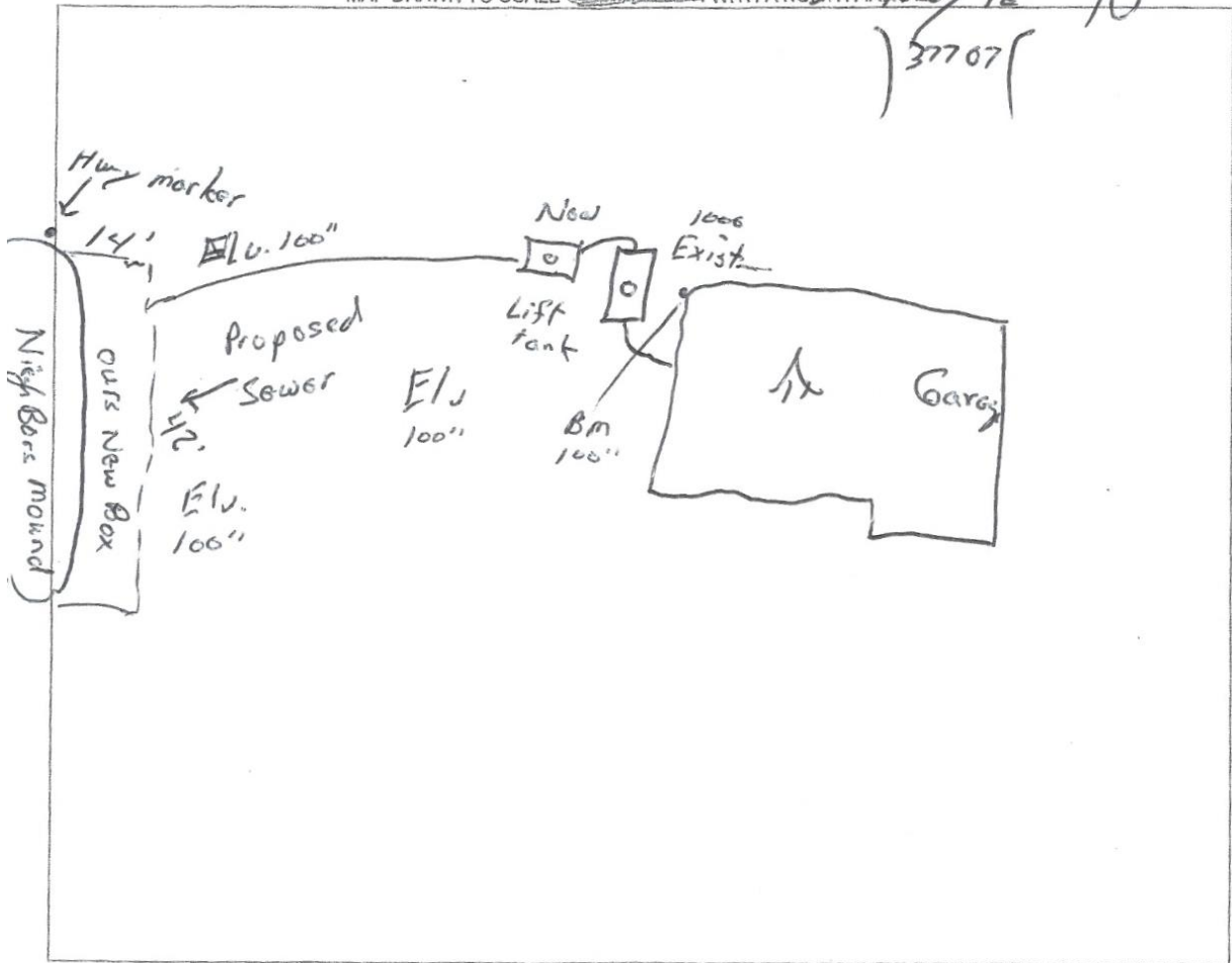
CLIENT: Todd Bohnen

36-1-678200

DATE: 8-10-23

MAP DRAWN TO SCALE WITH A NORTH ARROW

St. Hwy 12  
119'  
37707'



CHECK OFF LIST--HAVE ALL OF THE FOLLOWING BEEN DRAWN ON THE MAP??

- SHOW EXISTING OR PROPOSED
- WATER WELLS WITHIN 100 FT OF TREATMENT AREAS
  - PRESSURE WATER LINES WITHIN 10 FT OF TREATMENT AREAS
  - STRUCTURES  LOT IMPROVEMENTS
  - ALL SOIL TREATMENT AREAS  ALL ISTS COMPONENTS
  - HORIZONTAL AND VERTICAL REFERENCE
  - POINT OF SOIL BORINGS  DIRECTION OF SLOPE
  - LOT EASEMENTS  ALL LOT DIMENSIONS
  - DISTURBED/ COMPACTED AREAS
  - SITE PROTECTION--LATHE AND RIBBON EVERY 15 FT
  - ACCESS ROUTE FOR TANK MAINTENANCE
- REQUIRED SETBACKS
- STRUCTURES  PROPERTY LINES
  - OHWL
- COMMENTS:

INDICATE ELEVATIONS

- BENCHMARK 100
- ELEVATION OF SEWER LINE @ HOUSE Exist.
- ELEVATION @ TANK INLET Exist.
- ELEVATION @ BOTTOM OF ROCK LAYER 118"
- ELEVATION @ BOTTOM OF BORING OR RESTRICTIVE LAYER 79"
- ELEVATION OF PUMP 56"
- ELEVATION OF DISTRIBUTION DEVICE 118"

DESIGNER SIGNATURE B & B Built  
LICENSE# 2088

DATE 8-10-23

31-1-078000

### Subsurface Sewage Treatment System Management Plan

Property Owner: Todd Bohren Phone: 651-747-1457 Date: 8-10-23  
 Mailing Address: \_\_\_\_\_ City: \_\_\_\_\_ Zip: \_\_\_\_\_  
 Site Address: 37707 St. Hwy 18 City: Atkin Zip: 56431

This management plan will identify the operation and maintenance activities necessary to ensure long-term performance of your septic system. Some of these activities must be performed by you, the homeowner. Other tasks must be performed by a licensed septic service provider.

System Designer: check every \_\_\_\_\_ months.  
 Local Government: check every \_\_\_\_\_ months.  
 State Requirement: check every 36 months.

My System needs to be checked every \_\_\_\_\_ months.

*(State requirements are based on MN Rules Chapter 7080.2450, Subp. 2 & 3)*

#### Homeowner Management Tasks

- Leaks* – Check (look, listen) for leaks in toilets and dripping faucets. Repair leaks promptly.
- Surfacing sewage* – Regularly check for wet or spongy soil around your soil treatment area.
- Effluent filter* – Inspect and clean twice a year or more.
- Alarms* – Alarm signals when there is a problem. Contact a service provider any time an alarm signals.
- Event counter or water meter* – Record your water use.  
 -recommend meter readings be conducted (circle one: DAILY WEEKLY MONTHLY)

#### Professional Management Tasks

- Check to make sure tank is not leaking
- Check and clean the in-tank effluent filter
- Check the sludge/scum layer levels in all septic tanks
- Recommend if tank should be pumped
- Check inlet and outlet baffles
- Check the drainfield effluent levels in the rock layer
- Check the pump and alarm system functions
- Check wiring for corrosion and function
- Check dissolved oxygen and effluent temperature in tank
- Provide homeowner with list of results and any action to be taken
- Flush and clean laterals if cleanouts exist

"I understand it is my responsibility to properly operate and maintain the sewage treatment system on this property, utilizing the Management Plan. If requirements in the Management Plan are not met, I will promptly notify the permitting authority and take necessary corrective actions. If I have a new system, I agree to adequately protect the reserve area for future use as a soil treatment system."

Property Owner Signature: \_\_\_\_\_ Date: \_\_\_\_\_

Designer Signature: Bob Bald Date: 8-10-23

See Reverse Side for Management Log

31-1-078200  
Maintenance Log

| Activity                                   | Date Accomplished |
|--|-------------------|
| <i>Check frequently:</i>                   |                   |
| * Leaks: check for plumbing leaks          |                   |
| * Soil treatment area check for surfacing  |                   |
| Lint filter: check, clean if needed        |                   |
| Effluent screen: if owner-maintained       |                   |
| Water usage rate (monitor frequency _____) |                   |
| <i>Check annually:</i>                     |                   |
| * Caps: inspect, replace if needed         |                   |
| * Sludge & Scum/Pump                       |                   |
| * Inlet & Outlet baffles                   |                   |
| * Drainfield effluent leaks                |                   |
| Pump, alarm, wiring                        |                   |
| Flush & clean laterals if cleanouts exists |                   |
| Other: _____                               |                   |
| Other: _____                               |                   |

Notes: \_\_\_\_\_  
 \_\_\_\_\_  
 \_\_\_\_\_  
 \_\_\_\_\_

Mitigation/corrective action plan: \_\_\_\_\_  
 \_\_\_\_\_  
 \_\_\_\_\_  
 \_\_\_\_\_