

Type 3 System Similar to an inground mound

FIELD EVALUATION SHEET

PRELIMINARY EVALUATION DATE August 2, 2023, FIELD EVALUATION DATE July 12, 2023

PROPERTY OWNER: Timothy & Pauline Anderson PHONE 218-838-9493

ADDRESS: 31901-416th Avenue CITY, STATE, ZIP: Aitkin, Mn, 56431

LEGAL DESCRIPTION: PIN# 07-0-028802 SEC 14 T 46 R 27 TWP NAME Farm Island

FIRE# LAKE/RIVER Bachelor LAKE CLASS OHWL FT.

DESCRIPTION OF SOIL TREATMENT AREAS

Table with 4 columns: DISTURBED AREAS, COMPACTED AREAS, FLOODING, RUN ON POTENTIAL, SLOPE %, DIRECTION OF SLOPE, LANDSCAPE POSITION, VEGETATION TYPES. Includes sub-columns for AREA #1 and AREA #2 (YES/NO) and REFERENCE BM ELEV. and REFERENCE BM DESCRIPTION.

DEPTH TO STANDING WATER OR MOTTLED SOIL: BORING# 140-48, 1A, 2, 2A

BOTTOM ELEVATION-FIRST TRENCH OR BOTTOM OF ROCK BED: #1 99 FT., #2 FT.

SOIL SIZING FACTOR: SITE #1 83, SITE #2

CONSTRUCTION RELATED ISSUES: Area must be dug out 48" and filled with 36" of washed sand and will have 12" of rock on top. Top of rock bed will be at the existing ground level or very close

LIC# L2132 SITE EVALUATOR SIGNATURE: Tom O'Neil

SITE EVALUATOR NAME: Tom O'Neil TELEPHONE# 218-927-6070

LUG REVIEW #1 DATE 8-18-23

Comments: When the house and garage were built, any dirt removed from their areas was spread over the original ground to blend in with the buildings and create a nicer area around them. That is why the dirt is to be dug out and replaced with washed sand. System must have a filter and alarm

SOIL BORING LOGS ON REVERSE SIDE

Existing tanks were pumped & inspected by Timber Lakes and found to be good. a larger pump tank will need to be added (600gal. minimum) for the new 4 bedroom system. The existing pump tank is only about a 350 gallon or slightly larger

7/12/2023

CR 76 Almost across
from Tim Woodrow

Soil investigation w/ Jeff Brunner

Site 1 51491.190th Pl., McGregor Pit

A 0-4" 10YR 3/2 LS 5-10% cf
 Bw 4-30" 5YR 4/4 S 5-10% cf
 C 30-69" 7.5YR 4/6 S 5-10% cf

Site 2 51767/186th Pl., McGregor

A 0-4" 10YR 3/2 LS 10-15% cf
 Bw 4-28" 7.5YR 4/4

Tom O'Neil

7/12/2023 Tom O'Neil
31901 CR 76, Aitkin

Site for possible septic system

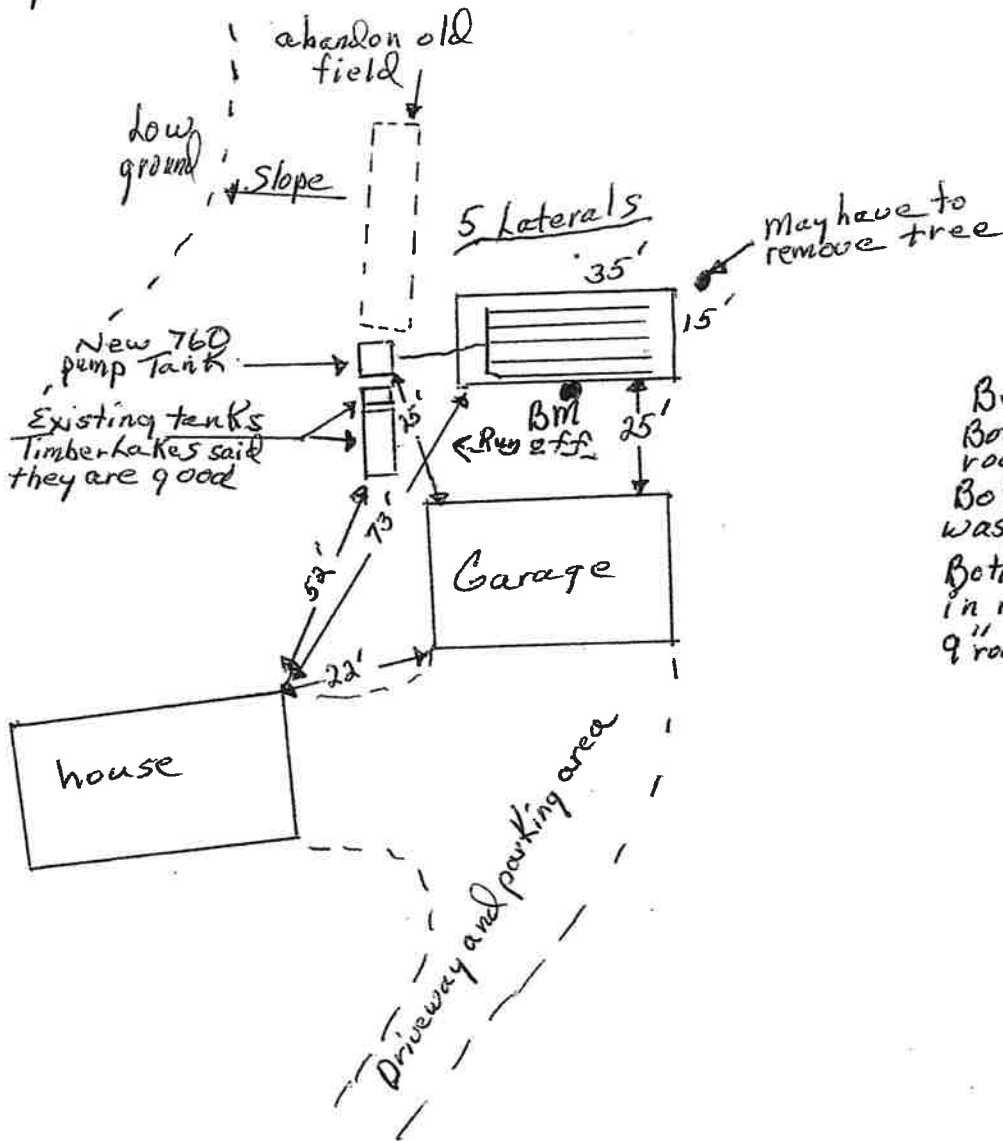
Fill 0-26" Put in place in 1978

A 26-30" 10YR 2/2 Sil 10-15% cf
 E 30-37" 10YR 5/3 Sil 5-10% cf
 Bt1 37-40" 10YR 4/4 CL 5-10% cf
 Bt2 40"+ 10YR 4/4 CL
 7.5YR 5/6 + 6/2 mottles.

I suggested to Tom that he take out the upper 24-30"
 of material and replace with clean sand - kind of
 an inground mound - 12' wide and however long he
 needs for a 3 BR requirement.

A pit was dug and Brian and I both did the soils

If homeowner would prefer,
 a new 1820 tank could replace
 everything



- Bench mark 100
- Bottom of rock bed - 99
- Bottom of washed sand - 96
- Bottom of pipe in rock bed 99.75
- 9" rock under pipe

PUMP SELECTION PROCEDURE

A. Determine pump capacity

Gravity distribution

1. Minimum is 10 GPM
2. Maximum is 45 GPM

Pressure Distribution

3. a. Select number of perforated laterals 5.
- b. Select perforation spacing = 3 ft.
- c. Subtract 2 ft from rock layer length:
 $\underline{35} - 2 = \underline{33}$ feet. (length of lateral)

ROCK LAYER LENGTH

- d. Determine the number of spaces between perfs:

$$\frac{\underline{33}}{\text{(length of lateral)}} \div \frac{\underline{3}}{\text{(perf. spacing)}} = \underline{11} \text{ spaces}$$

- e. $\underline{11} \text{ spaces} + 1 = \underline{12}$ perforations per lateral

- f. Multiply perforations per lateral by number of laterals to get total number of perforations:

$$\frac{\underline{12}}{\text{(perfs/lateral)}} \times \frac{\underline{5}}{\text{(laterals)}} = \underline{60} \text{ (perforations)}$$

- g. $\underline{60} \times \underline{0.74} = \underline{45}$ GPM
 (Perforations) x (gpm/perfs)

SELECTED PUMP CAPACITY 45 GPM

B. Determine head requirements:

1. Elevation difference between pump & point of discharge:

$$\underline{11} \text{ feet}$$

2. If pumping to a pressure distribution system, add 5 feet; for gravity add zero: 5 feet

3. Friction Loss

- a. Enter friction loss table with GPM and pipe diameter. Read friction loss in feet per 100 ft in table.

$$\text{F.L.} = \underline{3.28} \text{ ft/100 of pipe}$$

- b. Determine total pipe length from pump to discharge point. Add 25% to pipe length for fitting loss.

$$\underline{35} \text{ length} \times 1.25 = \underline{44} \text{ feet.}$$

- c. Calculate total friction loss by multiplying friction loss in 100 ft. of pipe by equivalent pipe length (B):

$$\text{Total friction loss} = \underline{3.28} \times \underline{44} / 100 = \underline{2} \text{ feet}$$

4. Total head required is the sum of the elevation difference, special head requirements and total friction loss:

$$\frac{\underline{11}}{(1)} + \frac{\underline{5}}{(2)} + \frac{\underline{2}}{(3c)} \text{ TOTAL HEAD } \underline{18}$$

SELECT A PUMP TO DELIVER AT LEAST 45 GPM WITH AT LEAST 18 FEET OF TOTAL HEAD.

If laterals are connected to a header pipe in a pressure system, select the minimum size lateral diameter; enter the table with perforation spacing and the number of perforations per lateral.

Select minimum size of lateral use 1 1/2" or 2"
 For a center manifold system the values will be 1/2 of above.

Perforation Discharges in GPM

Head (feet)	Perforation diameter (inches)	
	7/32	1/4
1.0a	0.56	0.74
1.5	0.69	0.90
2.0b	0.80	1.04

- a. Use 1.0 foot single homes
- b. Use 2.0 feet for anything else

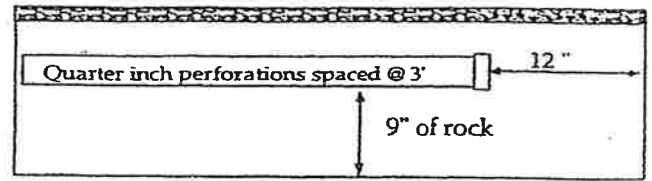
FRICTION LOSS IN PLASTIC PIPE

Flow Rate GPM	1.5"	2"	3"
20	2.47	0.73	0.11
25	3.73	1.11	0.16
30	5.23	1.55	0.23
35	6.96	2.06	0.30
40	8.91	2.64	0.39
45	11.07	3.28	0.48
50	13.46	3.99	0.58
55		4.76	0.70
60		5.60	0.82
65		6.48	0.95
70		7.44	1.09

Max. No. of 1/4" perfs per lateral. (10%var)

Perforation spacing (feet)	1	1	2"
	1/4"	1/2"	
2.5 feet	14	18	28
3.0 feet	13	17	26
3.3 feet	12	16	25
4.0 feet	11	15	23
5.0 feet	10	14	22

- Select number of perforated laterals 5
- Select perforation spacing = 3 ft
- Since perforations should not be placed closer than 1 foot to the edge of the rock layer (see diagram), subtract 2 feet from the rock layer length.



Perf Sizing 3/16" - 1/4"
Perf Spacing 1.5' - 5'

$$\frac{35}{\text{Rock layer length}} - 2 \text{ ft} = \underline{33} \text{ ft}$$

- Determine the number of spaces between perforations. Divide the length (3) by perforation spacing (2) and round down to nearest whole number.
Perforation spacing = 33 ft ÷ 3 ft = 11 spaces
- Number of perforations is equal to one plus the number of perforation spaces(4). Check figure E-4 to assure the number of perforations per lateral guarantees <10% discharge variation.

E-4: Maximum allowable number of 1/4-inch perforations per lateral to guarantee <10% discharge variation

perforation spacing (feet)	1 inch	1.25 inch	1.5 inch	2.0 inch
2.5	8	14	18	28
<u>3.0</u>	8	<u>13</u>	17	26
3.3	7	12	16	25
4.0	7	11	15	23
5.0	6	10	14	22

$$\underline{11} \text{ spaces} + 1 = \underline{12} \text{ perforations/lateral}$$

- A. Total number of perforations = perforations per lateral (5) times number of laterals (1)

$$\underline{12} \text{ perfs/lat} \times \underline{5} \text{ lat} = \underline{60} \text{ perforations}$$

- B. Calculate the square footage per perforation. Should be 6-10 sqft/perf. Does not apply to at-grades.

Rock bed area = rock width (ft) x rock length (ft)

$$\underline{15} \text{ ft} \times \underline{35} \text{ ft} = \underline{500} \text{ sqft}$$

Square foot per perforation = Rock bed area ÷ number of perfs (6)

$$\underline{500} \text{ sqft} \div \underline{60} \text{ perfs} = \underline{8} \text{ sqft/perf}$$

- Determine required flow rate by multiplying the total number of perforations (6A) by flow per perforation (see figure E-6)

$$\underline{60} \text{ perfs} \times \underline{0.74} \text{ gpm/perfs} = \underline{45} \text{ gpm}$$

- If laterals are connected to header pipe as shown on upper example, to select minimum required lateral diameter; enter figure E-4 with perforation spacing (2) and number of perforations per lateral (5) Select minimum diameter for " or 2" perforated lateral = 1.25 inches. use 1 1/2 or 2

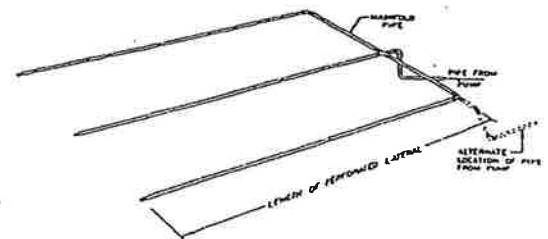
- If perforated lateral system is attached to manifold pipe near the center, lower diagram, perforated lateral length (3) and number of perforations per lateral (5) will be approximately one half of that in step 8. Using these values, select minimum diameter for perforated lateral = _____ inches.

E-6: Perforation Discharge in gpm

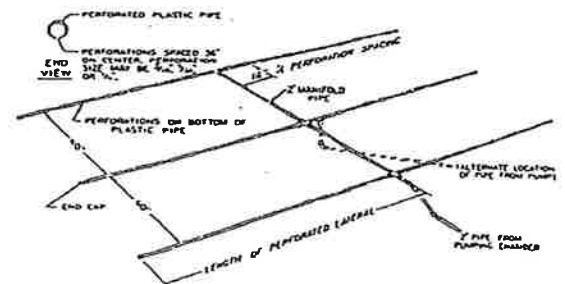
head (feet)	perforation diameter (inches)			
	1/8	3/16	7/32	1/4
1.0 ^a	0.18	0.42	0.56	<u>0.74</u>
2.0 ^b	0.26	0.59	0.80	1.04
5.0	0.41	0.94	1.26	1.65

^a Use 1.0 foot for single-family homes.
^b Use 2.0 feet for anything else.

MANIFOLD LOCATED AT END OF PRESSURE DISTRIBUTION SYSTEM



LAYOUT OF PERFORATED PIPE LATERALS FOR PRESSURE DISTRIBUTION IN MOUND



I hereby certify that I have completed this work in accordance with applicable ordinances, rules and laws.

Tom O'Neil

(signature)

22132

(license #)

August 2, 2023 (date)

TRENCH AND BED WORKSHEET

1. AVERAGE DESIGN FLOW

- A. Estimated 600 gpd (see figure A-1)
or measured x 1.5 (safety factor) = gpd
- B. Septic tank capacity 1,000 min. gal (see figure C-1)

A-1: Estimated Sewage Flows in Gallons per Day

number of bedrooms	Class I	Class II	Class III	Class IV
2	300	225	180	60%
3	450	300	218	of the
4	600	375	256	values
5	750	450	294	in the
6	900	525	332	Class I
7	1050	600	370	II or III
8	1200	675	408	columns.

2. SOILS (Site evaluation data)

- C. Depth to restricting layer = 4 ft
- D. Max depth of system Item 2C - 3 ft = 4 ft - 3 ft = 1 ft
- E. Texture Washed Sand Percolation rate 0.83 MPI
- F. Soil Sizing Factor (SSF) 0.83 sqft/gpd (see figure D-15)
- G. % Land Slope 1 %

C-1: Septic Tank Capacities (in gallons)

Number of Bedrooms	Minimum Liquid Capacity	Liquid capacity with garbage disposal	Liquid capacity with disposal & lift inside
2 or less	750	1125	1500
3 or 4	1000	1500	2000
5 or 6	1500	2250	3000
7, 8 or 9	2000	3000	4000

3. TRENCH or BED BOTTOM AREA

- H. For trenches with 6 inches of rock below the pipe:
 $A \times F = \text{ } \text{ gpd} \times \text{ } \text{ sqft/gpd} = \text{ } \text{ sqft}$
- I. For trenches with 12 inches of rock below the pipe:
 $A \times F \times 0.8 = \text{ } \text{ gpd} \times \text{ } \text{ sqft/gpd} \times 0.8 = \text{ } \text{ sqft}$
- J. For trenches with 18 inches of rock below the pipe:
 $A \times F \times 0.66 = \text{ } \text{ gpd} \times \text{ } \text{ sqft/gpd} \times 0.66 = \text{ } \text{ sqft}$
- K. For trenches with 24 inches of rock below the pipe:
 $A \times F \times 0.6 = \text{ } \text{ gpd} \times \text{ } \text{ sqft/gpd} \times 0.6 = \text{ } \text{ sqft}$
- L. For gravity beds with 6 or 12 inches of rock below the pipe:
 $1.5 \times A \times F = 1.5 \times \text{ } \text{ gpd} \times \text{ } \text{ sqft/gpd} = \text{ } \text{ sqft}$
- For pressure beds with 6 or 12 inches of rock below the pipe:
 $A \times F = \text{600 gpd} \times \text{0.83 sqft/gpd} = \text{500 sqft}$

D-15: Soil Characteristics and Soil Sizing Factor (SSF) (> 3" separation)

Percolation Rate (minutes per inch (mpi))	Soil Texture	Soil Sizing Factor (square feet/gallon per day (sqft/gpd))
faster than 0.1*	Coarse sand	0.83
0.1 to 5**	Medium sand	0.83
	Loamy sand	
6 to 15	Fine sand	1.67
16 to 30	Sandy loam	1.27
31 to 45	Loam	1.67
	Silt loam	2.00
46 to 60	Silt	
	Clay loam	2.20
	Sandy clay	
	Silty clay	
over 61 to 120***	Clay	4.20
	Sandy clay	
	Silty clay	
slower than 120***		

*Use systems for rapidly permeable soils: pressure distribution or serial distribution with no trench > 25% of the total system.
**Soil having 50% or more fine sand plus very fine sand
***A mound must be used.
****An other or performance system must be used

4. DISTRIBUTION (Check all that apply)

- Bed (< 6% slope) Drop boxes (any slope) Rock
- Trenches Distribution box (< 3%) Chamber
- Pressure Gravity Gravelless

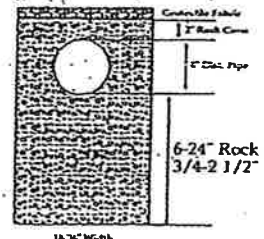
5. SYSTEM WIDTH, LENGTH and VOLUME

- M. Select trench width = 15 ft
- N. If using rock, divide bottom area by width: (H, I, J, K or L) ÷ M =
 $\frac{500 \text{ sqft}}{15 \text{ ft}} = 35 \text{ lineal feet}$
Rock depth below distribution pipe plus 0.5 foot times bottom area:
Rock depth in feet + 0.5 feet x Area (H, I, J, K, or L)
 $(2.5 \text{ ft} + 0.5 \text{ ft}) \times 500 \text{ sqft} = 500 \text{ cuft}$
Volume in cubic yards = cuft ÷ 27
 $\frac{500 \text{ cuft}}{27} = 19 \text{ cu yds}$
Weight of rock in tons = cubic yds x 1.4
 $19 \text{ cu yds} \times 1.4 = 27 \text{ tons}$
- O. If using 10" Gravelless Pipe, Flow (A) x Gravelless SSF (see figure D-9)
 gpd x lineal feet/gpd = lineal feet
- P. If using Chambers, H, I, J, or K (based on height of chamber slats) ÷ width of chamber in feet (M)
 sqft ÷ ft = lineal ft

D-9: Soil Characteristics and Soil sizing factors (SSF) for Gravelless Pipe

percolation rate (minutes/inch)	soil texture	lineal feet/gallon/day
Faster than 0.1*	Coarse Sand	—
0.1 to 5**	Medium Sand	0.28
	Loamy Sand	
6 to 15	Fine Sand**	0.6
16 to 30	Sandy Loam	0.42
31 to 45	Loam	0.56
	Silt Loam	0.67
46 to 60	Silt	
	Clay Loam (CL)	0.74
	Sandy CL	
	Silty CL	
slower than 60***	Clay	—
	Sandy Clay	
	Silty Clay	

*Soil too coarse for sewage treatment.
**Use systems for rapidly permeable soils.
***Soil having 50% or more fine sand + very fine sand.
****Soil with too high a percentage of clay for installation of a standard inground system.



6. LAWN AREA

- Q. Select trench spacing, center to center = 15 feet
- R. Multiply trench spacing by lineal feet R x Q = sqft of lawn area
 $15 \text{ ft} \times 35 \text{ ft} = 500 \text{ sqft}$

7. Include a drawing with scale (one inch = ft). Show pertinent boundaries, right of way, easements, location of house, garage, driveway, all other improvements, existing or proposed soil treatment system, well and dimensions of all elevations, setbacks and separation distances.

I hereby certify that I have completed this work in accordance with applicable ordinances, rules and laws.

Tom O'Neil

(signature)

L2132

(license #)

August 2, 2023 (date)