

# Ruyak Enterprises, Inc.

6909 State 6 NE  
PO Box 156  
Remer, MN 56672

Licensed, Bonded, Insured  
MPCA Installer L3206, Designer

Date: 11 Aug 2023  
Owner: Tony Petroske  
PID #: 33-0-035400

The individual sewage treatment system being installed on this property must follow MPCA Chapter 7080 Code or any local zoning codes that may be more restrictive. If the design has to change due to unknown site conditions during construction, please contact me as soon as possible. It is the installers responsibility to call for an inspection on this system and to have it property inspected before backfill.

Maintenance performed on the system over it's life span will provide an individual sewage treatment system with a much better chance of future hydraulic longevity.

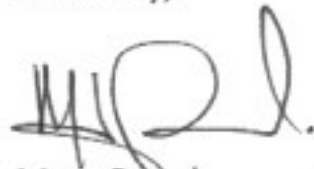
Please contact me if you have any questions regarding this design before commencing construction.

The septic tank should be pumped for sludge accumulation at a minimum every 3 years. This must be done by a licensed pumper and records must be kept by the owner. Water conservation is always recommended and a good idea to increase the life of your system.

If the original site soils have been altered, compacted, or disturbed in any way or the drain field areas change location this design is null and void.

It is the owner's responsibility to make sure there is a county permit prior to installation.

Sincerely,



Marc Ruyak  
Ruyak Enterprises, Inc.  
6909 State 6 NE  
Remer, MN 56672  
(218) 566-2913  
Changemyland@hotmail.com

FIELD EVALUATION SHEET

9 JUN 23

PRELIMINARY EVALUATION DATE \_\_\_\_\_, FIELD EVALUATION DATE 10 Aug 23
PROPERTY OWNER: TONY PETROSKE PHONE 641-436-0710
ADDRESS: 60933 240th AVE CITY, STATE, ZIP: JACOBSON, MN 55752
LEGAL DESCRIPTION: (NE NW) LOT 1
PIN# 33-0-035400 SEC 23 T 51 R 24 TWP NAME VERNDON
FIRE# 60933 LAKE/RIVER MISSISSIPPI RIVER LAKE CLASS \_\_\_\_\_ OHWL \_\_\_\_\_ FT.

DESCRIPTION OF SOIL TREATMENT AREAS

Table with 4 columns: Feature, Area #1, Area #2, Reference. Rows include DISTURBED AREAS, COMPACTED AREAS, FLOODING, RUN ON POTENTIAL, SLOPE %, DIRECTION OF SLOPE, LANDSCAPE POSITION, and VEGETATION TYPES.

DEPTH TO STANDING WATER OR MOTTLED SOIL: BORING# 1 6", 1A 8", 2 6", 2A

BOTTOM ELEVATION--FIRST TRENCH OR BOTTOM OF ROCK BED: #1 -3 FT., #2

SOIL SIZING FACTOR: SITE #1 1.07, SITE #2

CONSTRUCTION RELATED ISSUES: NONE

LIC# 1-3206 SITE EVALUATOR SIGNATURE: [Signature]

SITE EVALUATOR NAME: MARC RUYDK TELEPHONE# 218 566 2913

LUG REVIEW \_\_\_\_\_ DATE \_\_\_\_\_

Comments: MENTORED BY DAN SWITZER

SOIL BORING LOGS ON REVERSE SIDE

# SOILS CHARTS FOR BOTH PROPOSED AND ALTERNATE SITES

1 (PROPOSED) SOILS DATA

DEPTH (INCHES)	TEXTURE	MUNSELL COLOR
0-6	TOPSOIL LOAM	10YR 2/1
6-8	LOAM	• MOTTLES ↓
8-	CLAY	DEPLETIONS AT BOTTOM OF TOPSOIL

2 (PROPOSED) SOILS DATA

DEPTH (INCHES)	TEXTURE	MUNSELL COLOR
0-8	TOPSOIL LOAM	10YR 2/1
8-12	LOAM	• MOTTLES ↓
12-	CLAY	DEPLETIONS AT BOTTOM OF TOPSOIL

#3

1 (ALTERNATE) SOILS DATA

DEPTH (INCHES)	TEXTURE	MUNSELL COLOR
0-6	TOPSOIL LOAM	10YR 2/1
6-12	LOAM	• MOTTLES ↓
12-	CLAY	DEPLETIONS AT BOTTOM OF TOPSOIL

2 (ALTERNATE) SOILS DATA

DEPTH (INCHES)	TEXTURE	MUNSELL COLOR

ADDITIONAL SOIL BORINGS MAY BE REQUIRED

# MOUND DESIGN WORK SHEET (For Flows up to 1200 gpd)

## A. Average Design FLOW

Estimated 300 gpd (see figure A-1)  
 or measured \_\_\_\_\_ x 1.5 (safety factor) = \_\_\_\_\_ gpd

number of bedrooms	Class I	Class II	Class III	Class IV
<u>2</u>	300	225	180	60%
3	450	300	218	of the
4	600	375	256	values
5	750	450	294	in the
6	900	525	332	Class I,
7	1050	600	370	II, or III
8	1200	675	408	columns.

## B. SEPTIC TANK Capacity

750 gallons (see figure C-1)

Number of Bedrooms	Minimum Liquid Capacity	Liquid capacity with garbage disposal	Liquid capacity with disposal & lift inside
2 or less	750	1125	1500
3 or 4	1000	1500	2000
5 or 6	1500	2250	3000
7, 8 or 9	2000	3000	4000

## C. SOILS (refer to site evaluation)

- Depth to restricting layer = 0 feet
- Depth of percolation tests = — feet
- Texture LOAM  
 Percolation rate — mpi
- Soil loading rate 1.67 gpd/sqft (see figure D-33)
- Percent land slope 0 %

## D. ROCK LAYER DIMENSIONS

- Multiply average design flow (A) by 0.83 to obtain required rock layer area.  
300 gpd x 0.83 sqft/gpd = 249 sqft
- Determine rock layer width = 0.83 sqft/gpd x linear Loading Rate (LLR)  
 0.83 sqft/gpd x 12 gpd/sqft = 10 ft
- Length of rock layer = area ÷ width =  
249 sqft (D1) ÷ 10 ft (D2) = 24.9 ft

< 120 MPI	≤ 12
≥ 120 MPI	≤ 6

## E. ROCK VOLUME

- Multiply rock area (D1) by rock depth of 1 ft to get cubic feet of rock  
249 sqft x 1 ft = 249 cuft
- Divide cuft by 27 cuft/cuyd to get cubic yards  
249 cuft ÷ 27 cuyd/cuft = 9.22 cuyd
- Multiply cubic yards by 1.4 to get weight of rock in tons  
9.22 cuyd x 1.4 ton/cuyd = 12.91 tons

## F. SEWAGE ABSORPTION WIDTH

Absorption width equals absorption ratio (See Figure D-33) times rock layer width (D2)

2 x 10 ft = 20 ft

Percolation Rate in Minutes per Inch (MPI)	Soil Texture	Loading Rate Gallons per day per square foot	Absorption Ratio
Faster than 5	Coarse Sand Medium Sand Loamy Sand Fine Sand	1.20	1.00
6 to 15	Sandy Loam	0.79	1.50
16 to 30	Loam	0.60	2.00
31 to 45	Silt Loam Silt	0.50	2.40
46 to 60	Sandy Clay Loam Silty Clay Loam Clay Loam	0.45	2.67
61 to 120	Silty Clay Sandy Clay Clay	0.24	5.00
Slower than 120*			

\*System designed for these soils must be other or performance

<=1% land slope

G. Mound Slope Width and Length  
(landslope less than or equal to 1%)

1. Absorption width (F) 20 ft

2. Calculate mound size

a. Determine depth of clean sand fill

at upslope edge of rock layer = 3 ft

minus the distance to restricting layer (C1)

3 ft - 0 ft = 3 ft

b. Mound height at the upslope edge of rock

layer = depth of clean sand for separation (G2a)

at upslope edge plus depth of rock layer (1 ft) plus depth of cover (1 ft)

3 ft + 1ft + 1ft = 5 ft

c. Berm width = upslope mound height (G2b) times 4 (4 is recommended, but could be 3-12)

5 x 4 = 20 ft

d. The total landscape width is the sum of berm (G2c) width plus rock layer width (D2) plus berm width (G2c): 20 ft + 10 ft + 20 ft = 50 ft

e. Additional width necessary for absorption = absorption width (F) minus the landscape width (G2d)

         ft -          ft =          ft, if number is negative (<0) skip to g

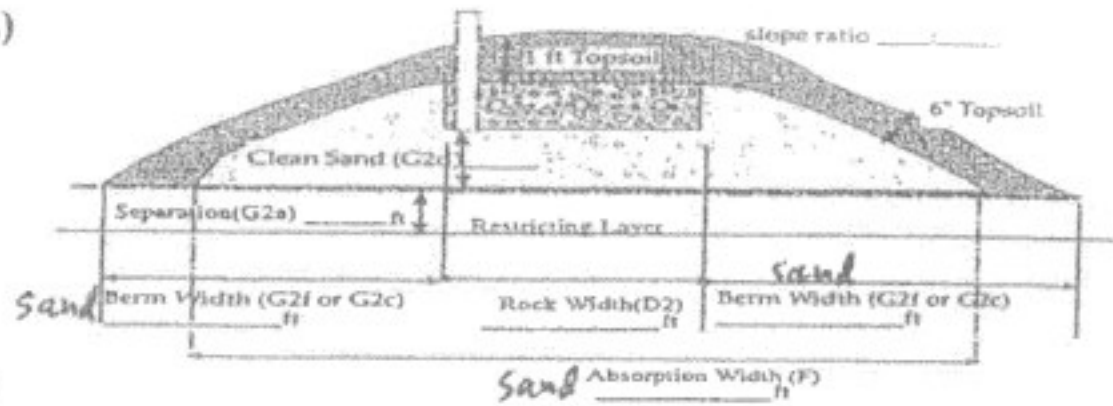
f. Final berm width = additional width (G2e) plus the berm width (G2c)

         ft +          ft =          ft

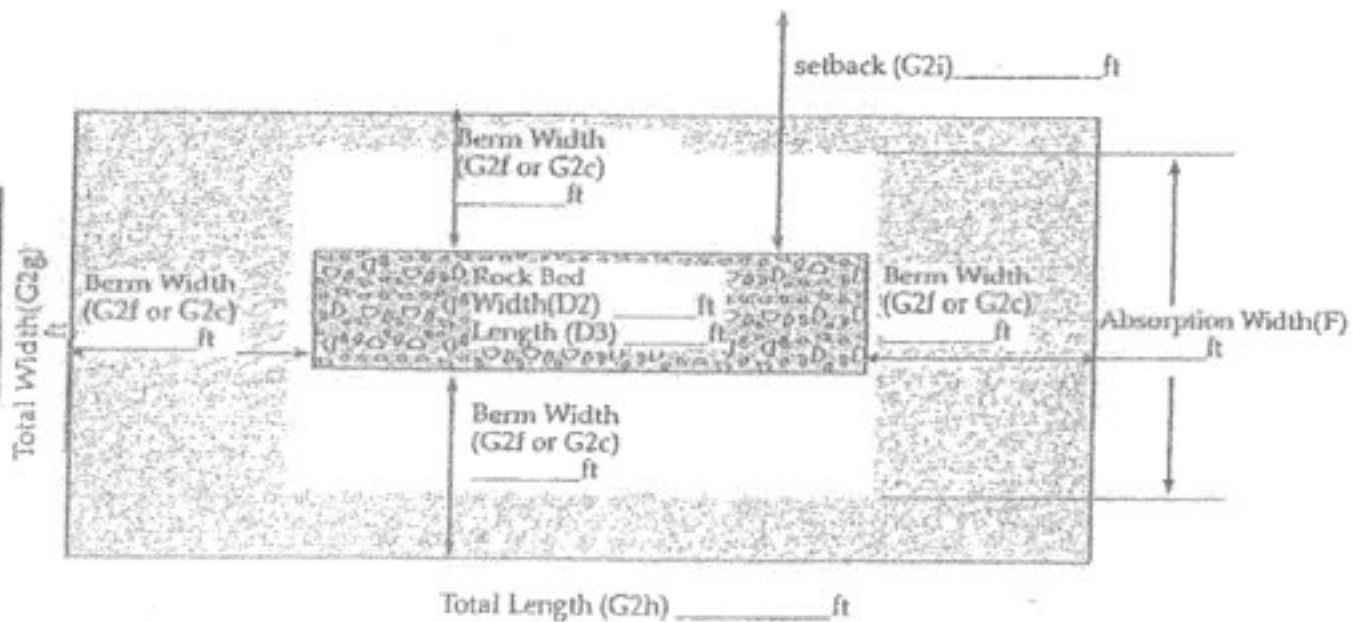
g. Total mound width is the sum of berm width (G2f or G2c) plus rock layer width (D2) plus berm width (G2f or G2c): 20 ft + 10 ft + 20 ft = 50 ft

h. Total mound length is the sum of berm (G2f or G2c) plus rock layer length (D3) plus berm (G2f or G2c): 20 ft + 24.9 ft + 20 ft = 64.9 ft

i. Setbacks from the rockbed are calculated as follows: the absorption width (F) minus the rock bed width (D2) divided by 2: (20 ft - 10 ft) ÷ 2 = 5 ft



**Final Dimensions:**  
64.9 x 50



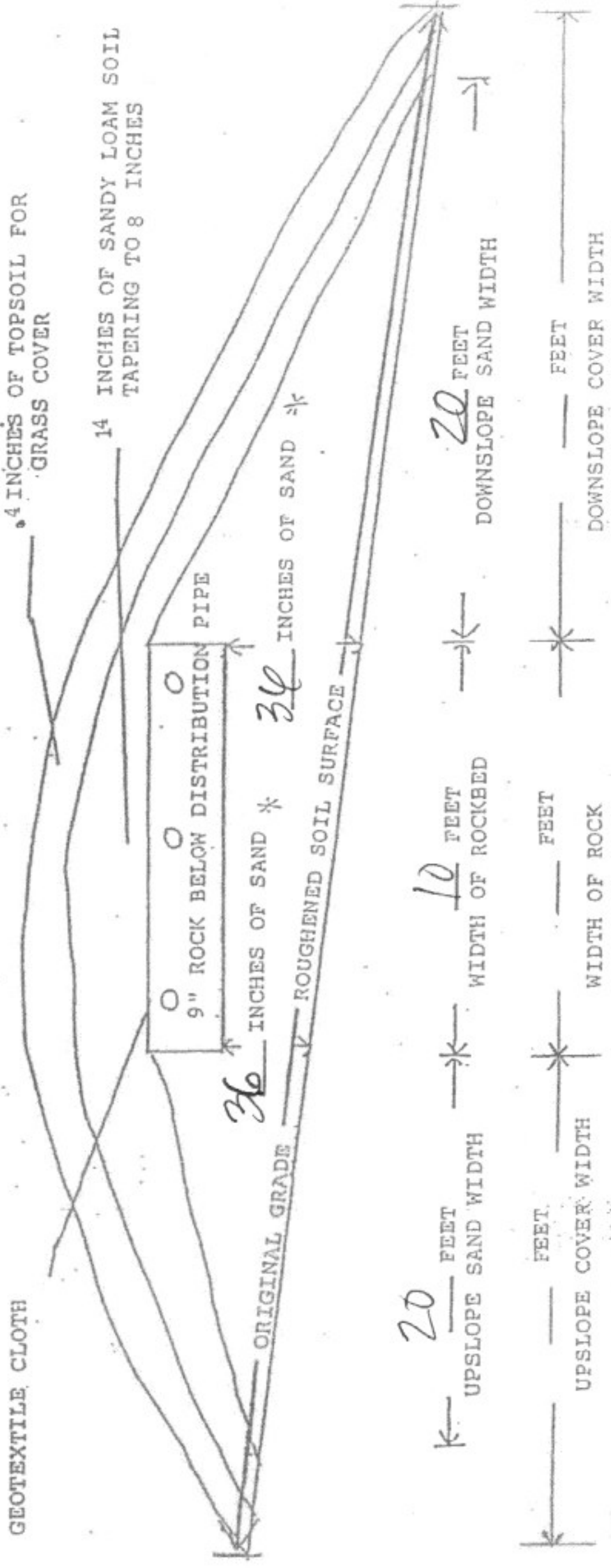
I hereby certify that I have completed this work in accordance with applicable ordinances, rules and laws.

[Signature] (signature) 13204 (license #) 11 AUG 23 (date)

MENTORED BY DAN SWITZER

MOUND CROSS-SECTION

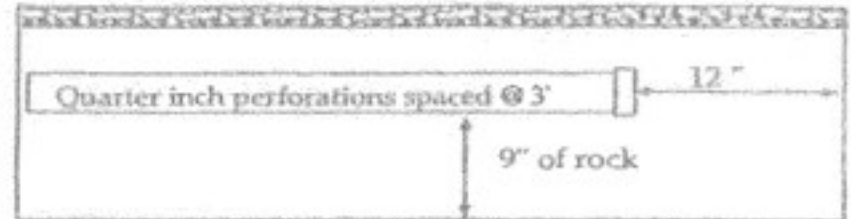
0 PERCENT SLOPE OF ORIGINAL SOIL  
10 FT. X 24.9 FT. SIZE OF ROCKBED 50 FT. X 64.9 FT. SIZE OF SANDBASE



20 FEET UPSLOPE SAND WIDTH  
 20 FEET DOWNSLOPE SAND WIDTH  
 10 FEET WIDTH OF ROCKBED  
 36 INCHES OF SAND \*  
 34 INCHES OF SAND \*  
 20 FEET UPSLOPE COVER WIDTH  
 FEET UPSLOPE COVER WIDTH  
 FEET DOWNSLOPE COVER WIDTH

# PRESSURE DISTRIBUTION SYSTEM

Geotextile fabric



Perf Sizing 3/16" - 1/4"  
Perf Spacing 1.5' - 5'

E-4: Maximum allowable number of 1/4-inch perforations per lateral to guarantee <10% discharge variation

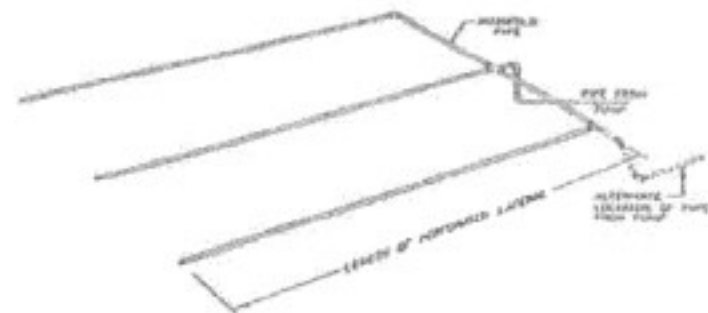
perforation spacing (feet)	1 inch	1.25 inch	1.5 inch	2.0 inch
2.5	8	14	18	28
3.0	8	13	17	26
3.3	7	12	16	25
4.0	7	11	15	23
5.0	6	10	14	22

E-6: Perforation Discharge in gpm

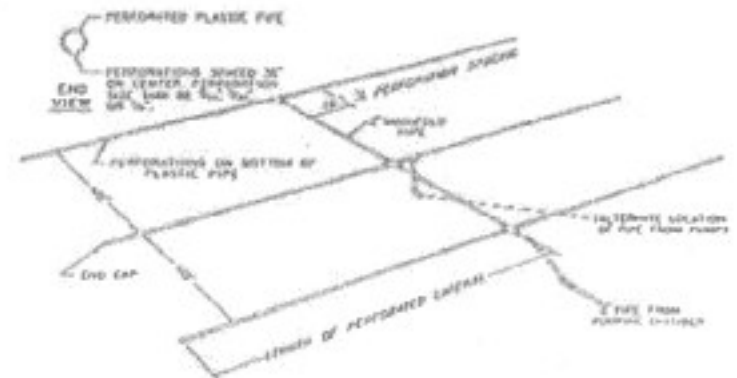
head (feet)	perforation diameter (inches)			
	1/8	3/16	7/32	1/4
1.0 <sup>a</sup>	0.18	0.42	0.56	0.74
2.0 <sup>b</sup>	0.26	0.59	0.80	1.04
5.0	0.41	0.94	1.26	1.65

<sup>a</sup> Use 1.0 foot for single-family homes.  
<sup>b</sup> Use 2.0 feet for anything else.

MANIFOLD LOCATED AT END OF PRESSURE DISTRIBUTION SYSTEM



LAYOUT OF PERFORATED PIPE LATERALS FOR PRESSURE DISTRIBUTION IN MANIFOLD



- Select number of perforated laterals 3
- Select perforation spacing = 3 ft
- Since perforations should not be placed closer than 1 foot to the edge of the rock layer (see diagram), subtract 2 feet from the rock layer length.  
 $\frac{24.9}{\text{Rock layer length}} - 2 \text{ ft} = \underline{22.9} \text{ ft}$
- Determine the number of spaces between perforations. Divide the length (3) by perforation spacing (2) and round down to nearest whole number.  
Perforation spacing =  $\frac{22.9 \text{ ft}}{3 \text{ ft}} = \underline{7}$  spaces
- Number of perforations is equal to one plus the number of perforation spaces(4). Check figure E-4 to assure the number of perforations per lateral guarantee <10% discharge variation.  
 $\underline{7}$  spaces + 1 = 8 perforations/lateral
- A. Total number of perforations = perforations per lateral (5) times number of laterals (1)  
 $\underline{8}$  perfs/lat x 3 lat = 24 perforations
- B. Calculate the square footage per perforation.  
Should be 6-10 sqft/perf. Does not apply to at-grades.  
Rock bed area = rock width (ft) x rock length (ft)  
 $\underline{10} \text{ ft} \times \underline{24.9} \text{ ft} = \underline{249} \text{ sqft}$   
Square foot per perforation = Rock bed area ÷ number of perfs (6)  
 $\frac{249 \text{ sqft}}{24 \text{ perfs}} = \underline{10} \text{ sqft/perf}$
- Determine required flow rate by multiplying the total number of perforations (6A) by flow per perforation (see figure E-6)  
 $\underline{24} \text{ perfs} \times \underline{.74} \text{ gpm/perfs} = \underline{17.76} \text{ gpm}$
- If laterals are connected to header pipe as shown on upper example, to select minimum required lateral diameter; enter figure E-4 with perforation spacing (2) and number of perforations per lateral (5) Select minimum diameter for perforated lateral = 1.5 inches.
- If perforated lateral system is attached to manifold pipe near the center, lower diagram, perforated lateral length (3) and number of perforations per lateral (5) will be approximately one half of that in step 8. Using these values, select minimum diameter for perforated lateral = \_\_\_\_\_ inches.

I hereby certify that I have completed this work in accordance with applicable ordinances, rules and laws.

(signature)

13206

(license #)

11 AUG 23

(date)

# PUMP SELECTION PROCEDURE

## 1. Determine pump capacity:

### A. Gravity distribution

1. Minimum required discharge is 10 gpm
2. Maximum suggested discharge is 45 gpm. For other establishments at least 10% greater than the water supply rate, but no faster than the rate at which effluent will flow out of the distribution device.

### B. Pressure distribution

See pressure distribution work sheet

From A or B Selected pump capacity: 17.76 gpm

## 2. Determine pump head requirements:

### A. Elevation difference between pump and point of discharge?

8 feet

### B. Special head requirement? (See Figure at right - Special Head Requirements)

5 feet

### C. Calculate Friction loss

1. Select pipe diameter 2.0 in

2. Enter Figure E-9 with gpm (1A or B) and pipe diameter (C1).

Read friction loss in feet per 100 feet from Figure E-9

Friction Loss = 2.06 ft/100ft of pipe

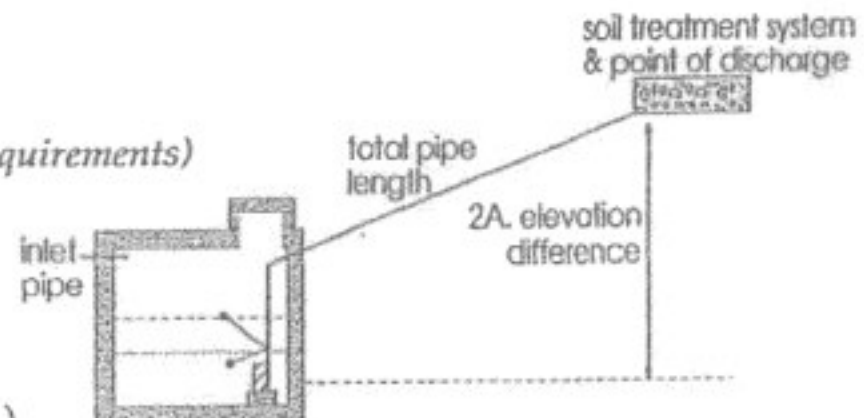
3. Determine total pipe length from pump discharge to soil treatment discharge point. Estimate by adding 25 percent to pipe length for fitting loss. Total pipe length times 1.25 = equivalent pipe length  
25 feet x 1.25 = 31.25 feet

4. Calculate total friction loss by multiplying friction loss (C2) in ft/100 ft by the equivalent pipe length (C3) and divide by 100.  
 = 2.06 ft/100ft x 31.25 +100 = .6438 ft

### D. Total head required is the sum of elevation difference (A), special head requirements (B), and total friction loss (C4)

8 ft + 5 ft + .64 ft =

Total head: 13.64 feet



Special Head Requirements	
Gravity Distribution	0 ft
Pressure Distribution	5 ft

flow rate gpm	E-9: Friction Loss in Plastic Pipe Per 100 feet		
	nominal pipe diameter		
	1.5"	2"	3"
20	2.47	0.73	0.11
25	3.73	1.11	0.16
30	5.23	1.55	0.23
35	6.96	2.06	0.30
40	8.91	2.64	0.39
45	11.07	3.28	0.48
50	13.46	3.99	0.58
55		4.76	0.70
60		5.60	0.82
65		6.48	0.95
70		7.44	1.09

## 3. Pump selection

A pump must be selected to deliver at least 17.76 gpm (1A or B) with at least 13.6 feet of total head (2D)

I hereby certify that I have completed this work in accordance with applicable ordinances, rules and laws.

[Signature]

(signature)

L3206

(license #)

11 Aug 23

(date)

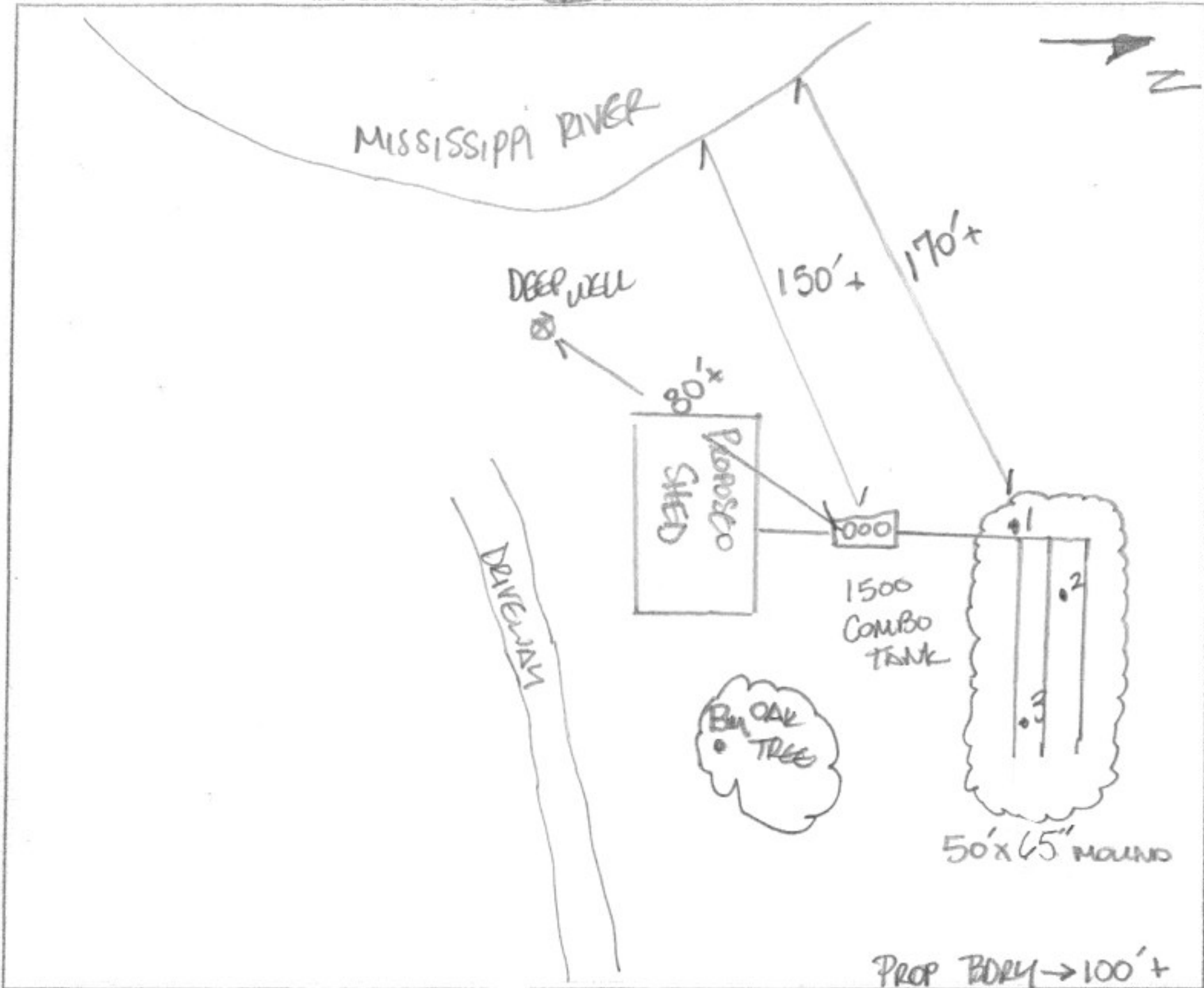


CLIENT: TOMY PETROSKE

SKETCH SHEET

DATE: 11 Aug 23

MAP DRAWN TO SCALE ~~1"=100'~~ WITH A NORTH ARROW



CHECK OFF LIST--HAVE ALL OF THE FOLLOWING BEEN DRAWN ON THE MAP??

SHOW EXISTING OR PROPOSED

- WATER WELLS WITHIN 100 FT OF TREATMENT AREAS
- PRESSURE WATER LINES WITHIN 10 FT OF TREATMENT AREAS
- STRUCTURES
- ALL SOIL TREATMENT AREAS
- LOT IMPROVEMENTS
- ALL ISTS COMPONENTS
- HORIZONTAL AND VERTICAL REFERENCE
- POINT OF SOIL BORINGS
- DIRECTION OF SLOPE
- LOT EASEMENTS
- ALL LOT DIMENSIONS
- DISTURBED/ COMPACTED AREAS
- SITE PROTECTION--LATHE AND RIBBON EVERY 15 FT
- ACCESS ROUTE FOR TANK MAINTENANCE

REQUIRED SETBACKS

- STRUCTURES
- PROPERTY LINES
- OHWL

COMMENTS:

INDICATE ELEVATIONS

- 100 BENCHMARK
- 98 ELEVATION OF SEWER LINE @ HOUSE
- 97 ELEVATION @ TANK INLET
- 103 ELEVATION @ BOTTOM OF ROCK LAYER
- 100 ELEVATION @ BOTTOM OF BORING OR RESTRICTIVE LAYER
- 95 ELEVATION OF PUMP
- 103.5 ELEVATION OF DISTRIBUTION DEVICE

DESIGNER SIGNATURE

*[Handwritten Signature]*

LICENSE# L3206

DATE 11 Aug 23

MENTORED BY DAN SWITZER

## Subsurface Sewage Treatment System Management Plan

Property Owner: TONY PETROSKE Phone: 641-436-0710 Date: 11 AUG 23  
Mailing Address: 60933 290TH AVE City: JACOBSON Zip: 55752  
Site Address: SAME City: \_\_\_\_\_ Zip: \_\_\_\_\_

This management plan will identify the operation and maintenance activities necessary to ensure long-term performance of your septic system. Some of these activities must be performed by you, the homeowner. Other tasks must be performed by a licensed septic service provider or maintenance provider.

System Designer: Recommends SSTS check every 36 months.  
Local Government: Recommends SSTS check every 36 months.  
State Requirement: Requires SSTS check every 36 months.  
(State requirements are based on MN Rules Chapter 7080.2450, Subp. 2 & 3)

**My System needs to be checked  
every 36 months.**

### Homeowner Management Tasks:

*Leaks* – Check (look, listen) for leaks in toilets and dripping faucets. Repair leaks promptly.

*Surfacing sewage* – Regularly check for wet or spongy soil around your soil treatment area.

*Effluent filter* – *inspect and clean twice a year or more.*

*Alarms* – Alarm signals when there is a problem. Contact a service or maintenance provider any time an alarm signals.

*Event counter or water meter* – Record your water use.

-recommend meter readings be conducted (circle one): DAILY WEEKLY MONTHLY N/A

### Licensed septic service provider or maintenance provider (Check all that apply):

- Check to make sure tank is not leaking
- Check and clean the in-tank effluent filter (if exists)
- Check the sludge/scum layer levels in all septic tanks
- Recommend if tank should be pumped
- Check inlet and outlet baffles
- Check the drainfield effluent levels in the rock layer
- Check the pump and alarm system functions
- Check wiring for corrosion and function
- Check dissolved oxygen and effluent temperature in tank
- Provide homeowner with list of results and any action to be taken
- Flush and clean laterals if cleanouts exist

"I understand it is my responsibility to properly operate and maintain the sewage treatment system on this property, utilizing the Management Plan. If requirements in the Management Plan are not met, I will promptly notify the permitting authority and take necessary corrective actions. If I have a new system, I agree to adequately protect the reserve area for future use as a soil treatment system."

Property Owner Signature: \_\_\_\_\_ Date: \_\_\_\_\_

Designer Signature: [Signature] Date: 11 AUG 23

See Reverse Side for Management Log

## Maintenance Log

Activity	Date Accomplished									
<b><i>Check frequently:</i></b>										
Leaks: check for plumbing leaks										
Soil treatment area check for surfacing										
Lint filter: check, clean if needed										
Effluent screen: if owner-maintained										
Water usage rate (monitor frequency _____)										
<b><i>Check annually:</i></b>										
Caps: inspect, replace if needed										
Sludge & Scum/Pump										
Inlet & Outlet baffles										
Drainfield effluent leaks										
Pump, alarm, wiring										
Flush & clean laterals if cleanouts exists										
Other: _____										
Other: _____										

Notes: \_\_\_\_\_

\_\_\_\_\_

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