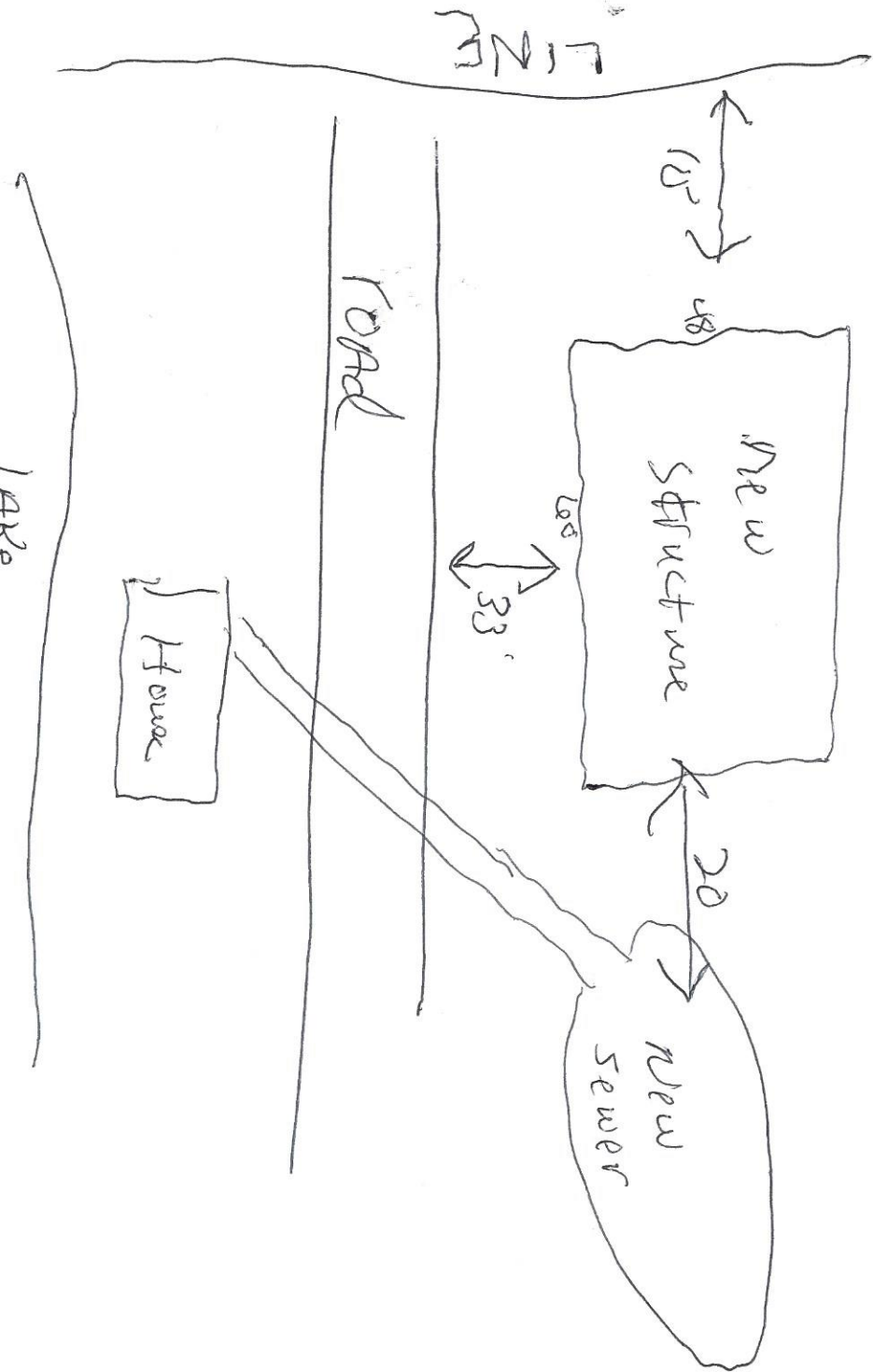


Site Plan



FIELD EVALUATION SHEET

PRELIMINARY EVALUATION DATE 6-10-22, FIELD EVALUATION DATE 6-10-22
 PROPERTY OWNER: Mrs. Feunaber
 ADDRESS: 45778 220th Lane CITY, STATE, ZIP: Arka
 LEGAL DESCRIPTION: Long Part 1: 2 01k1 Hoggs Run, Har. Keweenaw Lake Sub.
 PIN# 11-125660 SEC 31T 95 R27 TWP NAME Hoggs Run LAKE CLASS RD OHWL FT.
 FIRE# Round Lake LAKE/RIVER

DESCRIPTION OF SOIL TREATMENT AREAS

AREA #1	AREA #2
YES <input checked="" type="checkbox"/>	YES <input type="checkbox"/>
NO <input type="checkbox"/>	NO <input type="checkbox"/>
YES <input checked="" type="checkbox"/>	YES <input type="checkbox"/>
NO <input type="checkbox"/>	NO <input type="checkbox"/>
YES <input checked="" type="checkbox"/>	YES <input type="checkbox"/>
NO <input type="checkbox"/>	NO <input type="checkbox"/>
YES <input checked="" type="checkbox"/>	YES <input type="checkbox"/>
NO <input type="checkbox"/>	NO <input type="checkbox"/>
YES <input checked="" type="checkbox"/>	YES <input type="checkbox"/>
NO <input type="checkbox"/>	NO <input type="checkbox"/>
YES <input checked="" type="checkbox"/>	YES <input type="checkbox"/>
NO <input type="checkbox"/>	NO <input type="checkbox"/>

DISTURBED AREAS
 COMPACTED AREAS
 FLOODING
 RUN ON POTENTIAL
 SLOPE %
 DIRECTION OF SLOPE
 LANDSCAPE POSITION
 VEGETATION TYPES

DEPTH TO STANDING WATER OR MOTTLED SOIL: BORING# 1 8" 1A, 210" 2A

BOTTOM ELEVATION--FIRST TRENCH OR BOTTOM OF ROCK BED: #1 FT., #2 FT.

SOIL SIZING FACTOR: SITE # 1 1.27, SITE #2 _____

CONSTRUCTION RELATED ISSUES: 1650 cars to 10x50' Rock Bed - 3' sand
I need operating permit
 LIC# 2082
 SITE EVALUATOR SIGNATURE: Bob Bork
 SITE EVALUATOR NAME: Bob Bork
 TELEPHONE# 218-831-6130
 DATE _____

LUG REVIEW
 Comments: _____

SOIL BORING LOGS ON REVERSE SIDE

MOUND DESIGN WORK SHEET (For Flows up to 1200 gpd)

11-1-125600

A. Average Design Flow

Estimated 600 gpd (see figure A-1) x 1.5 (safety factor) = 900 gpd

B. SEPTIC TANK CAPACITY

1656 gallons (see figure C-1)

C. SOILS (refer to site evaluation)

1. Depth to restricting layer = 8" feet
2. Depth of percolation tests = 1.27 feet
3. Texture = 1.27 mpi
4. Soil loading rate = gpd/sqft (see figure D-33)
5. Percent land slope = %

A-1: Estimated Sewage Flows in Gallons per Day

number of bedrooms	Class I	Class II	Class III	Class IV
8	1200	675	408	columns.
7	1050	600	370	ii. or iii.
6	900	525	332	Class I.
5	750	450	294	in the
4	600	375	256	volumes
3	450	300	218	of the
2	300	225	180	60%
				Class IV

C-1: Septic Tank Capacities (in gallons)

Number of Bedrooms	Minimum Liquid Capacity	Liquid capacity with garbage disposal	Liquid capacity with disposal pit inside
2 or less	750	1125	1500
3 or 4	1000	1500	2000
5 or 6	1500	2250	3000
7, 8 or 9	2000	3000	4000

D. ROCK LAYER DIMENSIONS

1. Multiply average design flow (A) by 0.83 to obtain required rock layer area. $600 \text{ gpd} \times 0.83 \text{ sqft/gpd} = 500 \text{ sqft}$
2. Determine rock layer width = $0.83 \text{ sqft/gpd} \times \text{linear Loading Rate (LLR)}$
3. Length of rock layer = $\text{area} \div \text{width} = \frac{500 \text{ sqft}}{10 \text{ ft}} = 50 \text{ ft}$

Mound LLR

$> 120 \text{ MPI}$	$< 120 \text{ MPI}$
$\geq 120 \text{ MPI}$	≤ 6

E. ROCK VOLUME

1. Multiply rock area (D1) by rock depth of 1 ft to get cubic feet of rock $500 \text{ sqft} \times 1 \text{ ft} = 500 \text{ cuft}$
2. Divide cuft by 27 cuft/cuyd to get cubic yards $\frac{500 \text{ cuft}}{27} = 19 \text{ cuyd}$
3. Multiply cubic yards by 1.4 to get weight of rock in tons $19 \text{ cuyd} \times 1.4 \text{ ton/cuyd} = 26 \text{ tons}$

F. SEWAGE ABSORPTION WIDTH

Absorption width equals absorption ratio (See Figure D-33) times rock layer width (D2)

$1.50 \times 10 \text{ ft} = 15 \text{ ft}$

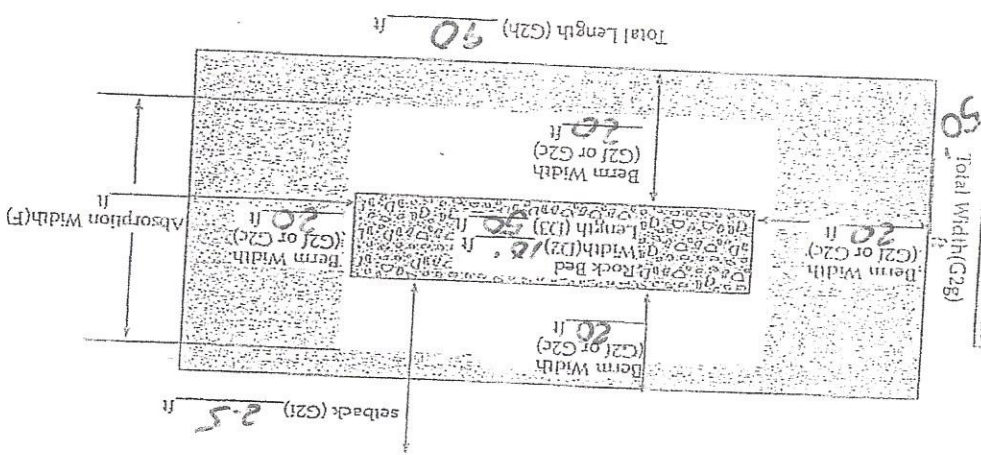
D-33: Absorption Width Sizing Table

Percolation Rate (MPI)	Soil Texture	Loading Rate (gallons per day per square foot)	Absorption Ratio
Faster than 5	Coarse Sand Medium Sand Loamy Sand Fine Sand	1.20	1.00
0 to 1.5	Sandy Loam Silt Loam	0.75	1.30
1.6 to 3.0	Silt Loam	0.60	2.00
3.1 to 4.5	Silt Loam	0.50	2.40
4.6 to 6.0	Sandy Clay Loam Silty Clay Loam	0.45	2.67
6.1 to 12.0	Silty Clay Sandy Clay	0.24	5.00
Slower than 12.0	Clay		

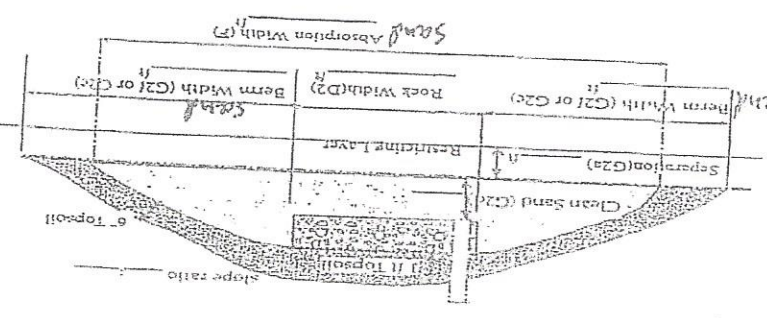
* System designed for these soils must be either of performance

I hereby certify that I have completed this work in accordance with applicable ordinances, rules and laws.
 (signature) Bob Paul (license #) 2088 (date) 6-10-22

Final Dimensions:
50' x 90'



- i. Setbacks from the rockbed are calculated as follows: the absorption width (F) minus the rock bed width (D2) divided by 2: $(15 \text{ ft} - 10 \text{ ft}) \div 2 = 2.5 \text{ ft}$
- h. Total mound length is the sum of berm (G2f or G2c) plus rock layer length (D3) plus berm (G2f or G2c): $20 \text{ ft} + 50 \text{ ft} + 20 \text{ ft} = 90 \text{ ft}$
- g. Total mound width is the sum of berm width (G2f or G2c) plus rock layer width (D2) plus berm width (G2f or G2c): $20 \text{ ft} + 10 \text{ ft} + 20 \text{ ft} = 50 \text{ ft}$
- f. Final berm width = additional width (G2e) plus the berm width (G2c)
 $15 \text{ ft} - 50 \text{ ft} = -35 \text{ ft}$, if number is negative (<0) skip to g
- e. Additional width necessary for absorption = absorption width (F) minus the landscape width (G2d) (G2c): $20 \text{ ft} + 10 \text{ ft} + 20 \text{ ft} = 50 \text{ ft}$
- d. The total landscape width is the sum of berm (G2c) width plus rock layer width (D2) plus berm width (G2c): $5 \text{ ft} \times 4 = 20 \text{ ft}$
- c. Berm width = upslope mound height (G2b) times 4 (4 is recommended, but could be 3-12)
 $3 \text{ ft} + 1 \text{ ft} + 1 \text{ ft} = 5 \text{ ft}$
- b. Mound height at the upslope edge of rock layer = depth of clean sand and for separation (G2a) at upslope edge plus depth of rock layer (1 ft) plus depth of cover (1 ft)
- a. Determine depth of clean sand fill at upslope edge of rock layer = 3 ft minus the distance to restricting layer (C1)



- 1. Absorption width (F) 15 ft
- 2. Calculate mound size
- 3. Determine depth of clean sand fill at upslope edge of rock layer = 3 ft
- 4. Mound Slope Width and Length (andslope less than or equal to 1%)
 $11 \cdot 1 \cdot 125 \cdot 60 = 1\%$ land slope

11-1-125600

PUMP SELECTION PROCEDURE

1. Determine pump capacity: Gravity distribution

1. Minimum required discharge is 10 gpm
2. Maximum suggested discharge is 45 gpm. For other establishments at least 10% greater than the water supply rate, but no faster than the rate at which effluent will flow out of the distribution device.

B. Pressure distribution

See pressure distribution work sheet

From A or B Selected pump capacity: 38 gpm

2. Determine pump head requirements:

A. Elevation difference between pump and point of discharge? 90 feet

B. Special head requirements? (See Figure at right - Special Head Requirements) 5 feet

C. Calculate Friction loss 5 feet

1. Select pipe diameter 2 in

2. Enter Figure E-9 with gpm (1A or B) and pipe diameter (C1). Read friction loss in feet per 100 feet from Figure E-9 Friction Loss = 267 ft/100ft of pipe

3. Determine total pipe length from pump discharge to soil treatment discharge point. Estimate by adding 25 percent to pipe length for fitting loss. Total pipe length times 1.25 = equivalent pipe length 115 feet $\times 1.25 = 56$ feet

4. Calculate total friction loss by multiplying friction loss (C2) in ft/100 ft by the equivalent pipe length (C3) and divide by 100. = 267 ft/100ft $\times 56$ ft $\div 100 = 1$ ft

D. Total head required is the sum of elevation difference (A), special head requirements (B), and total friction loss (C4) = 9 ft + 5 ft + 1 ft = 15 feet

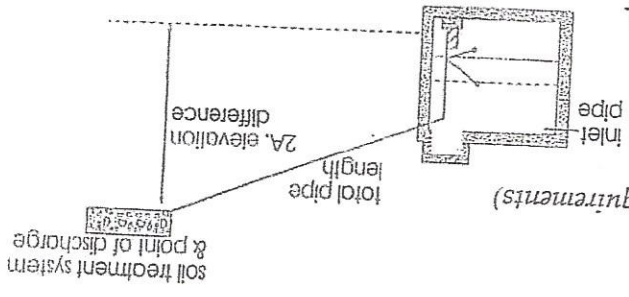
A pump must be selected to deliver at least 38 gpm (1A or B) with at least 15 feet of total head (2D)

3. Pump selection

Per 100 feet nominal pipe diameter	flow rate gpm	friction loss
3"	2.47	0.73
2"	3.73	1.11
1.5"	5.23	1.55
1"	6.96	2.06
0.75"	8.91	2.64
0.5"	11.07	3.28
0.375"	13.46	3.99
0.25"	17.44	5.60
0.1875"	21.42	7.44
0.125"	25.40	9.28
0.09375"	29.38	11.11
0.0625"	33.36	13.46
0.046875"	37.34	15.81
0.03125"	41.32	18.16
0.0234375"	45.30	20.51
0.0175"	49.28	22.86
0.013125"	53.26	25.21
0.009375"	57.24	27.56
0.00703125"	61.22	29.91
0.0051875"	65.20	32.26
0.0039375"	69.18	34.61
0.0029375"	73.16	36.96
0.0021875"	77.14	39.31
0.001640625"	81.12	41.66
0.00123046875"	85.10	44.01
0.0009228515625"	89.08	46.36
0.0006921875"	93.06	48.71
0.0005190625"	97.04	51.06
0.00039428125"	101.02	53.41
0.000295703125"	105.00	55.76
0.000221775"	108.98	58.11
0.00016643125"	112.96	60.46
0.000124824375"	116.94	62.81
0.00009361875"	120.92	65.16
0.0000702140625"	124.90	67.51
0.000052660625"	128.88	69.86
0.0000399453125"	132.86	72.21
0.0000299589375"	136.84	74.56
0.0000224690625"	140.82	76.91
0.000016851875"	144.80	79.26
0.000012639375"	148.78	81.61
0.000009478125"	152.76	83.96
0.0000071089375"	156.74	86.31
0.000005331875"	160.72	88.66
0.000004000625"	164.70	91.01
0.00000300046875"	168.68	93.36
0.0000022503515625"	172.66	95.71
0.0000016877640625"	176.64	98.06
0.000001265823046875"	180.62	100.41
0.0000009493671875"	184.60	102.76
0.000000712025390625"	188.58	105.11
0.0000005340190625"	192.56	107.46
0.00000040051440625"	196.54	109.81

E-9: Friction Loss in Plastic Pipe

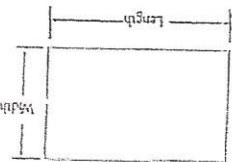
Special Head Requirements	0 ft
Gravity Distribution	5 ft
Pressure Distribution	0 ft



I hereby certify that I have completed this work in accordance with applicable ordinances, rules and laws.
 (signature) Bob Boly (license #) 2088 (date) 6-10-22

DOSING CHAMBER SIZING

11-1-125600



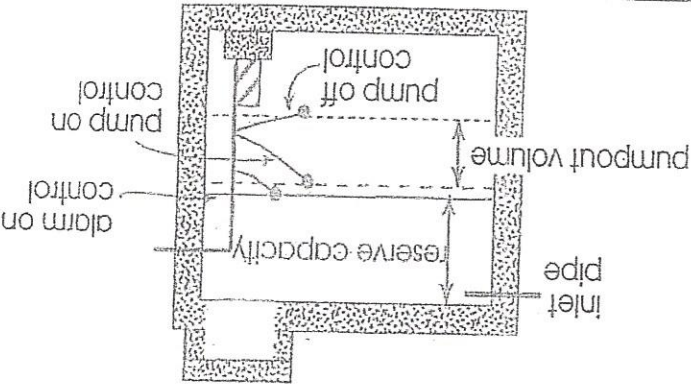
Legal Tank:
500 gallons or
100% the Daily flow
or
Alternating Pumps

A-1: Estimated Sewage Flows in Gallons per Day

number of bedrooms	Class I	Class II	Class III	Class IV
2	300	225	180	60%
3	450	300	218	of the
4	600	375	256	volumes
5	750	450	294	in the
6	900	525	332	Class I,
7	1050	600	370	II, or III
8	1200	675	408	columns.

B-20: Volume of Liquid in Pipe

Pipe Diameter inches	Gallons per foot
1	0.045
1.25	0.078
1.5	0.11
2	0.17
2.5	0.25
3	0.38
4	0.66



- Determine area
A. Rectangle area = $L \times W$
B. Circle area = $\pi (3.14) \times \text{radius in feet} \times \text{radius in feet}$
C. Get area from manufacturer

- Calculate gallons per inch
There are 7.5 gallons per cubic foot of volume, therefore multiply the area (1A, B or C) times the conversion factor and divide by 12 inches per foot to calculate gallon per inch.
Area $\times 7.5 \div 12 =$ _____ sqft $\times 7.5 \div 12 \text{ in/ft} = 12.69$ gallon per inch *mg*

- Calculate total tank volume
A. Depth from bottom of inlet pipe to tank bottom _____ in
B. Total tank volume = depth from bottom of inlet pipe to tank bottom (3A) \times gal/in (2) = $48.5 \text{ in} \times 12.69 \text{ gal/in} = 615$ gal

- Calculate gallons to cover pump (with 2-3 inches of water covering pump)
(Pump and block height (inch) + 2 inch) \times gallon/inch
(12 in + 2 in) $\times 12.69 \text{ gal/in} = 178$ gallon

- Calculate total pumpout volume
A. Select pump size for 4-5 doses per day. Gallon per dose = gpd (see figure A-1) / doses per day = $600 \text{ gpd} \div 5 = 120$ gallons
B. Calculate drainback
1. Determine total pipe length, 45 feet
2. Determine liquid volume of pipe, 17 gal per ft (see figure E-20)
3. Drainback quantity = 45 ft (5B1) $\times 17 \text{ gal per ft (5B2)} = 8$ gal
C. Total pump out volume = dose volume (5A) + drainback (5B3)
120 gal + 8 gal = 128 Total gallon

- Float separation distance (using total pumpout volume)
Total pumpout volume (5C) \div gal/inch (2)
 $128 \text{ gal} \div 12.69 \text{ gal/in} = 10$ inch

- Calculate volume for alarm (typically 2 to 3 inches)
Alarm depth (inch) \times gallon/inch (2) = 2 in $\times 12.69 \text{ gal/in} = .25$ gal

- Calculate total gallon = gallons over pump (4) + gallons pumpout (5C) + gallons alarm (7)
 $128 \text{ gal} + 25 \text{ gal} + 178 \text{ gal} = 331$ gallons

- Total Tank Depth = total gallon (8) \div gallon/inch (2)
 $331 \text{ gal} \div 12.69 \text{ gal/in} = 26$ in

Recommended:
Calculate reserve capacity (75% the daily flow)
Daily flow $\times .75 = 600 \times .75 = 450$ gallons

I hereby certify that I have completed this work in accordance with applicable ordinances, rules and laws.
 Bob Gault (signature) 2088 (license #) 6-16-22 (date)

