Home

FIELD EVALUATION SHEET

PRELIMINARY EVALUATION DATE 5/24/22, FIELD EVALUATION DATE 5/24/22
PROPERTY OWNER: WYATT - CARA BAYERLE, PHONE 612-390-2050
ADDRESS: 20810 124TH LN CITY,STATE,ZIP: MCGRATH 56350
LEGAL DESCRIPTION:
PIN# 38-0-031600 SEC 20 T 43 R 27 TWP NAME WILLIAMS
FIRE# — LAKE/RIVER — LAKE CLASS — OHWL — FT

DESCRIPTION OF SOIL TREATMENT AREAS

	AREA #1	AREA #2
DISTURBED AREAS	YES___NO X	YES___NO X
COMPACTED AREAS	YES___NO X	YES___NO X
FLOODING	YES___NO X	YES___NO X
RUN ON POTENTIAL	YES___NO X	YES___NO X
SLOPE %	2%	1-3%
DIRECTION OF SLOPE	NORTH	EAST
LANDSCAPE POSITION	SMALL KNOWL	SMALL RIDGE
VEGETATION TYPES	HAY FIELD	WOODED

REFERENCE BM ELEV. 100 F
REFERENCE BM DESCRIPTION
Top of Septic Cover.

DEPTH TO STANDING WATER OR MOTTLED SOIL: BORING# 1 1.5, 1A 1.5, 2 1+, 2A 1+

BOTTOM ELEVATION--FIRST TRENCH OR BOTTOM OF ROCK BED: #1 1.75 FT., #2 2 FT.

SOIL SIZING FACTOR: SITE # 1 1.67, SITE #2 1.67

CONSTRUCTION RELATED ISSUES:

LIC# L 2006 SITE EVALUATOR SIGNATURE: Dave Engdahl

SITE EVALUATOR NAME: DAVE ENGDAHL TELEPHONE# 592-3606

LUG REVIEW DATE

Comments:

SOIL BORING LOGS ON REVERSE SIDE

Form des 2/20/98

MOUND DESIGN WORK SHEET (For Flows up to 1200 gpd)

A. Average Design FLOW

Estimated 450 gpd *(see figure A-1)*
or measured ——— x 1.5 (safety *factor*) = ——— gpd

A-1: Estimated Sewage Flows in Gallons per Day

number of bedrooms	Class I	Class II	Class III	Class IV
2	300	225	180	60%
3	450	300	218	of the
4	600	375	256	values
5	750	450	294	in the
6	900	525	332	Class I,
7	1050	600	370	II, or III
8	1200	675	408	columns.

B. SEPTIC TANK Capacity

1000 (EXISTING) gallons *(see figure C-1)*

C-1: Septic Tank Capacities (in gallons)

Number of Bedrooms	Minimum Liquid Capacity	Liquid capacity with garbage disposal	Liquid capacity with disposal & lift inside
2 or less	750	1125	1500
3 or 4	1000	1500	2000
5 or 6	1500	2250	3000
7, 8 or 9	2000	3000	4000

C. SOILS *(refer to site evaluation)*

1. Depth to restricting layer = 1.25 feet
2. Depth of percolation tests = ——— feet
3. Texture Sandy Loam
   Percolation rate 1.67 mpi
4. Soil loading rate 0.0 gpd/sqft *(see figure D-33)*
5. Percent land slope 2 %

D. ROCK LAYER DIMENSIONS

1. Multiply average design flow (A) by 0.83 to obtain required rock layer area.
   450 gpd x 0.83 sqft/gpd = 375 sqft
2. Determine rock layer width = 0.83 sqft/gpd x linear Loading Rate (LLR)
   0.83 sqft/gpd x 450 gpd/sqft = 375 ft
3. Length of rock layer = area ÷ width =
   375 sqft (D1) ÷ 10 ft (D2) = 37.5 ft

Mound LLR	
< 120 MPI	≤ 12
≥ 120 MPI	≤ 6

E. ROCK VOLUME

1. Multiply rock area (D1) by rock depth of 1 ft to get cubic feet of rock
   375 sqft x 1 ft = 375 cuft
2. Divide cuft by 27 cuft/cuyd to get cubic yards
   375 cuft ÷ 27 cuyd/cuft = 13.8 cuyd
3. Multiply cubic yards by 1.4 to get weight of rock in tons
   13.8 cuyd x 1.4 ton/cuyd = 19.4 tons

F. SEWAGE ABSORPTION WIDTH

Absorption width equals absorption ratio (See Figure D-33) times rock layer width (D2)

10 x 2.0 ft = 20.0 ft

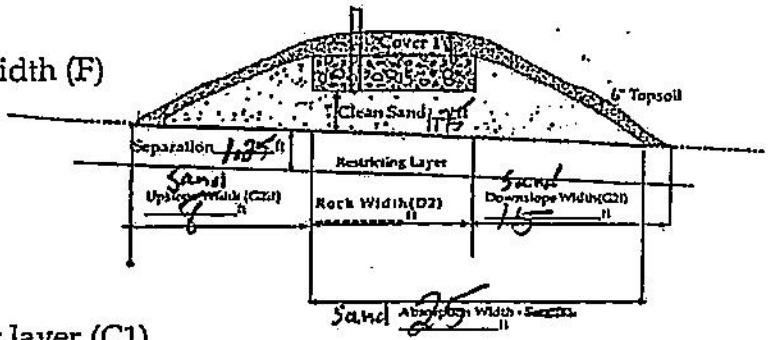
D-33: Absorption Width Sizing Table

Percolation Rate in Minutes per Inch (MPI)	Soil Texture	Loading Rate Gallons per day per square foot	Absorption Ratio
Faster than 5	Coarse Sand Medium Sand Loamy Sand Fine Sand	1.20	1.00
6 to 15	Sandy Loam	0.79	1.50
16 to 30	Loam	0.60	2.00
31 to 45	Silt Loam Silt	0.50	2.40
46 to 60	Sandy Clay Loan Silty Clay Loam Clay Loam	0.45	2.67
61 to 120	Silty Clay Sandy Clay Clay	0.24	5.00
Slower than 120*			

*System designed for these soils must be other or performance

G. MOUND SLOPE WIDTH & LENGTH
(landslope greater than 1%)

Landslope > 1% slope



1. Downslope absorption width = absorption width (F) minus rock layer width (D2)
20 ft - 10 ft = 10 ft

2. Calculate mound size

UPSLOPE

a. Depth of clean sand fill at upslope edge of rock layer = 3 ft minus the distance to restricting layer (C1)
3 ft - 1.25 ft = 1.75 ft

b. Mound height at the upslope edge of rock layer = depth of clean sand for separation (G2a) at upslope edge plus depth of rock layer (1 ft) plus depth of cover (1 ft)
1.75 ft + 1ft + 1ft = 3.75 ft

c. Upslope berm multiplier based on land slope
3.7 *(see figure D-34)*

d. Upslope width = berm multiplier (G2c) x upslope mound height (G2b):
3.75 x 3.7 ft = 14 ft

DOWNSLOPE

e. Drop in elevation = rock layer width (D2) x percent landslope (C5) ÷ 100
10 ft x 2 % ÷ 100 = .2 ft

f. Downslope mound height = depth of clean sand for slope difference (G2e) at downslope rock edge plus the mound height at the upslope edge of rock layer (G2b)
3.75 ft + .2 ft = 4.0 ft

g. Downslope berm multiplier based on percent land slc
4.25 *(see figure D-34)*

h. Downslope width = downslope multiplier (G2g) times downslope mound height (G2f) 33
4.25 x 4.0 ft = 17 ft

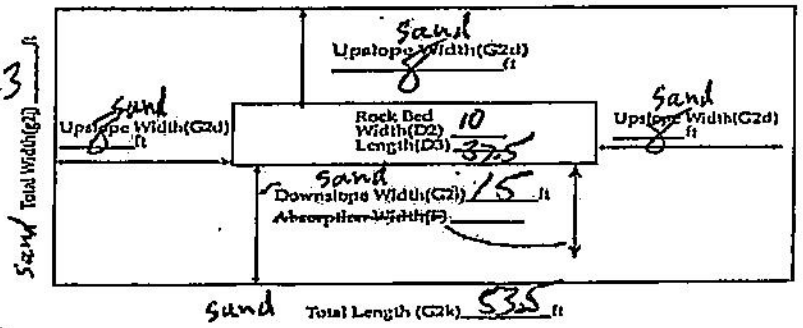
i. Select the greater of G1 and G2h as the downslope width: 17 ft

j. Total mound width is the sum of upslope width (G2d) width plus rock layer width (D2) plus downslope width (G2i)
14 ft + 10 ft + 17 ft = 41 ft

k. Total mound length is the sum of upslope width (G2d) plus rock layer length (D3) plus upslope width (G2d)
16 ft + 37.5 ft + 16 ft = 70 feet

D-34: SLOPE MULTIPLIER TABLE

Land Slope, in %	UPSLOPE multipliers for various slope ratios						DOWNSLOPE multipliers for various slope ratios				
	3:1	4:1	5:1	6:1	7:1	8:1	3:1	4:1	5:1	6:1	7:1
0	3.0	4.0	5.0	6.0	7.0	8.0	3.0	4.0	5.0	6.0	7.0
1	2.91	3.85	4.76	5.66	6.54	7.41	3.09	4.17	5.26	6.38	7.53
2	2.83	3.70	4.54	5.36	6.14	6.90	3.19	4.35	5.56	6.82	8.14
3	2.75	3.57	4.35	5.08	5.79	6.45	3.30	4.51	5.88	7.32	8.86
4	2.68	3.45	4.17	4.84	5.46	6.06	3.41	4.76	6.25	7.89	9.72
5	2.61	3.33	4.00	4.62	5.19	5.71	3.53	5.00	6.67	8.57	10.77
6	2.54	3.23	3.85	4.41	4.93	5.41	3.66	5.26	7.14	9.38	12.07
7	2.48	3.12	3.70	4.23	4.70	5.13	3.80	5.56	7.69	10.34	13.73
8	2.42	3.03	3.57	4.05	4.49	4.88	3.95	5.88	8.33	11.54	15.91
9	2.36	2.94	3.45	3.90	4.30	4.65	4.11	6.25	9.09	13.01	18.92
10	2.31	2.86	3.33	3.75	4.12	4.44	4.29	6.67	10.00	15.00	23.33
11	2.26	2.78	3.23	3.61	3.95	4.26	4.48	7.14	11.11	17.65	30.43
12	2.21	2.70	3.12	3.49	3.80	4.08	4.69	7.69	12.50	21.43	43.75



Final Dimensions:
41 x 70

I hereby certify that I have completed this work in accordance with applicable ordinances, rules and laws.

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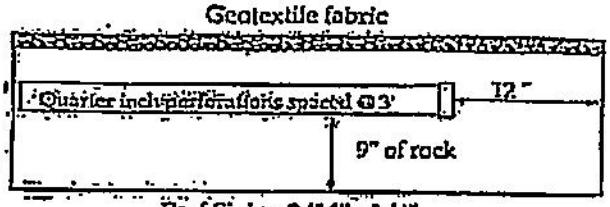
PRESSURE DISTRIBUTION SYSTEM



Perf Sizing 3/16" - 1/4"
Perf Spacing 1.5'- 5'

1. Select number of perforated laterals 3

2. Select perforation spacing = 3 ft

3. Since perforations should not be placed closer than 1 foot to the edge of the rock layer (see diagram), subtract 2 feet from the rock layer length.

   38 (Rock layer length) - 2 ft = 36 ft

4. Determine the number of spaces between perforations. Divide the length (3) by perforation spacing (2) and round down to nearest whole number.

   Perforation spacing = 36 ft ÷ 3 ft = 12 spaces

5. Number of perforations is equal to one plus the number of perforation spaces(4). *Check figure E-4 to assure the number of perforations per lateral guarantees <10% discharge variation.*

   12 spaces + 1 = 13 perforations/lateral

6. A. Total number of perforations = perforations per lateral (5) times number of laterals (1)

   13 perfs/lat x 3 lat = 39 perforations

   B. Calculate the square footage per perforation.
   Should be 6-10 sqft/perf. *Does not apply to at-grades.*
   Rock bed area = rock width (ft) x rock length (ft)
   10 ft x 38 ft = 380 sqft
   Square foot per perforation = Rock bed area ÷ number of perfs (6)
   380 sqft ÷ 39 perfs = 9.7 sqft/perf

7. Determine required flow rate by multiplying the total number of perforations (6A) by flow per perforation *(see figure E-6)*

   39 perfs x .74 gpm/perfs = 28.86 gpm

8. If laterals are connected to header pipe as shown on upper example, to select minimum required lateral diameter; enter figure E-4 with perforation spacing (2) and number of perforations per lateral (5) Select minimum diameter for perforated lateral = 1½ inches.

9. If perforated lateral system is attached to manifold pipe near the center, lower diagram, perforated lateral length (3) and number of perforations per lateral (5) will be approximately one half of that in step 8. Using these values, select minimum diameter for perforated lateral = _____ inches.

E-4: Maximum allowable number of 1/4-inch perforations per lateral to guarantee <10% discharge variation.

perforation spacing (feet)	1 inch	1.25 inch	1.5 inch	2.0 inch
2.5	8	14	18	28
3.0	8	13	17	26
3.3	7	12	16	25
4.0	7	11	15	23
5.0	6	10	14	22

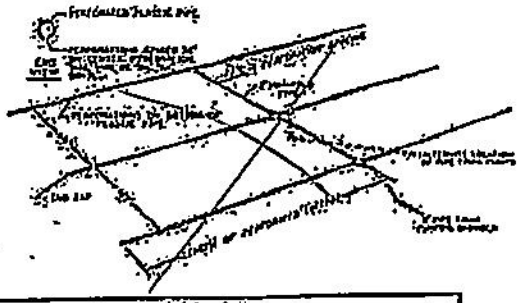
E-6: Perforation Discharge in gpm

head (feet)	perforation diameter (inches) 1/8	3/16	7/32	1/4
1.0[a]	0.18	0.42	0.56	0.74
2.0[b]	0.26	0.59	0.80	1.04
5.0	0.41	0.94	1.26	1.65

[a] Use 1.0 foot for single-family homes.
[b] Use 2.0 feet for anything else.

MANIFOLD LOCATED AT END OF PRESSURE DISTRIBUTION SYSTEM

LAYOUT OF RECOMMENDED PVC LATERALS FOR PRESSURE DISTRIBUTION SYSTEMS



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Dave Lydahl (Signature) L2086 (license #) 5/28/22 (date)

PUMP SELECTION PROCEDURE

1. Determine pump capacity:

A. Gravity distribution

1. Minimum required discharge is 10 gpm
2. Maximum suggested discharge is 45 gpm. For other establishments at least 10% greater than the water supply rate, but no faster than the rate at which effluent will flow out of the distribution device.

B. Pressure distribution

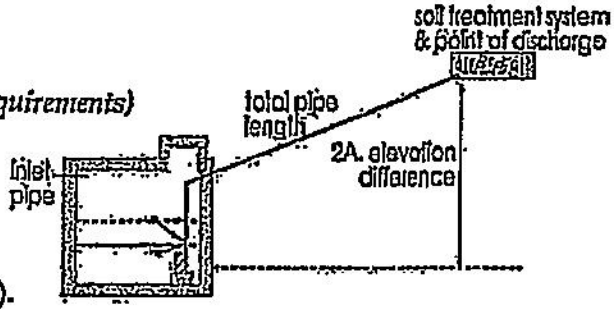
See pressure distribution work sheet

From A or B Selected pump capacity: 28.86 gpm

2. Determine pump head requirements:

A. Elevation difference between pump and point of discharge?
7.1 feet

B. Special head requirement? *(See Figure at right - Special Head Requirements)*
5.0 feet



Special Head Requirements	
Gravity Distribution	0 ft
Pressure Distribution	5 ft

C. Calculate Friction loss

1. Select pipe diameter 2 in
2. Enter *Figure E-9* with gpm (1A or B) and pipe diameter (C1).
   Read friction loss in feet per 100 feet from Figure E-9
   Friction Loss = 1.55 ft/100ft of pipe
3. Determine total pipe length from pump discharge to soil treatment discharge point. Estimate by adding 25 percent to pipe length for fitting loss. Total pipe length times 1.25 = equivalent pipe length
   105 feet x 1.25 = 131 feet
4. Calculate total friction loss by multiplying friction loss (C2) in ft/100 ft by the equivalent pipe length (C3) and divide by 100.
   = 131 ft/100ft x 1.55 ÷100 = 2.0 ft

D. Total head required is the sum of elevation difference (A), special head requirements (B), and total friction loss (C4)
7.1 ft + 5 ft + 2.0 ft =

Total head: 14.1 feet

E-9: Friction Loss in Plastic Pipe Per 100 feet

flow rate gpm	nominal pipe diameter 1.5"	2"	3"
20	2.47	0.73	0.11
25	3.73	1.11	0.16
30	5.23	1.55	0.23
35	6.96	2.06	0.30
40	8.91	2.64	0.39
45	11.07	3.28	0.48
50	13.46	3.99	0.58
55		4.76	0.70
60		5.60	0.82
65		6.48	0.95
70		7.44	1.09

3. Pump selection

A pump must be selected to deliver at least 14.1 gpm (1A or B) with at least 28.86 feet of total head (2D)

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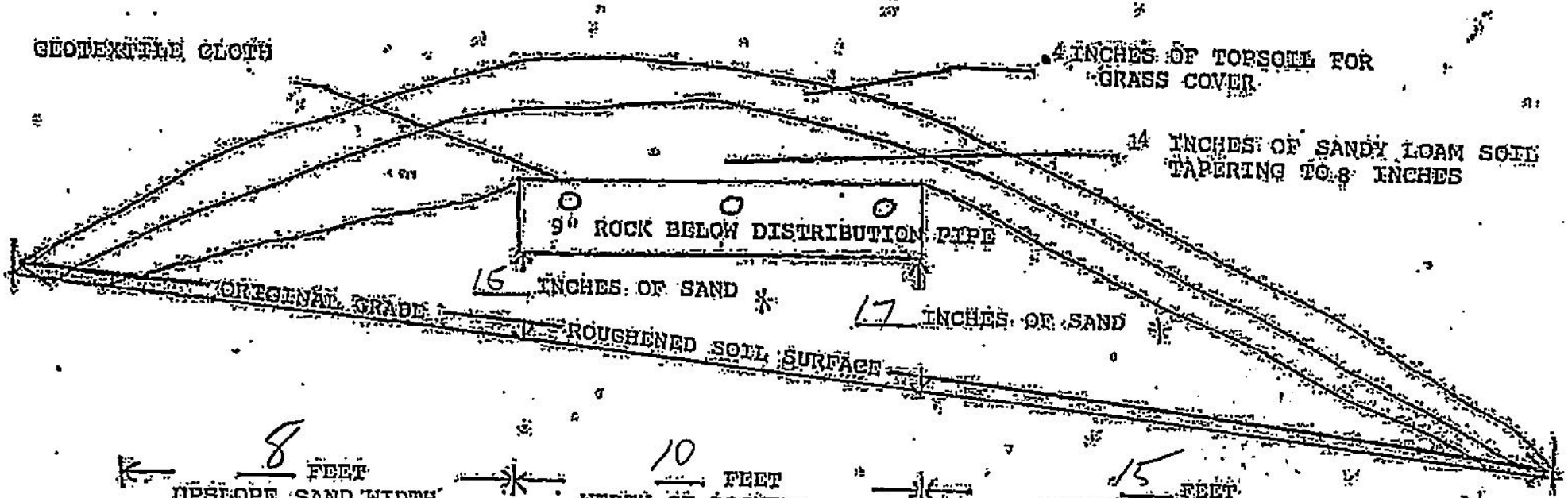
[signature] (signature) L2006 (license #) 5/28/22 (date)

MOUND CROSS-SECTION

2% PERCENT SLOPE OF ORIGINAL SOIL

10 FT. x 38 FT. SIZE OF ROCKBED 33 FT. x 55 FT. SIZE OF SANDBASE



8 FEET
UPSLOPE SAND WIDTH

10 FEET
WIDTH OF ROCKBED

15 FEET
DOWNSLOPE SAND WIDTH

14 FEET
UPSLOPE COVER WIDTH

10 FEET
WIDTH OF ROCK

17 FEET
DOWNSLOPE COVER WIDTH

SOILS CHARTS FOR BOTH PROPOSED AND ALTERNATE SITES

1 (PROPOSED) SOILS DATA

B1 Pit

DEPTH (INCHES)	TEXTURE	MUNSELL COLOR
0-8	T.S Sandy Loam FRIABLE NO ROCKS Roots-Sod	7.5yR 3/2
8-12	Sandy Loam FRIABLE Well DRAINED Some Roots NO ROCKS	7.5yR 4/3
12-18	Sandy Loam FRIABLE NO Rocks	7.5yR 4/4

Redox @ 18" 5Y 4/6

2 (PROPOSED) SOILS DATA

B2 Boring

DEPTH (INCHES)	TEXTURE	MUNSELL COLOR
0-8	T.S Sandy Loam	7.5yR 3/2
8-12	S.L	7.5yR 4/4
12-18	S.L	7.5yR 4/4

Redox 17-18"

Same at Pit

1 (ALTERNATE) SOILS DATA

ALT Site B-

DEPTH (INCHES)	TEXTURE	MUNSELL COLOR
0-8	T.S S.L	7.5yR 3/2
8-14+	Sandy Loam	7.5yR 4/4

Good Drainage for alt Site

2 (ALTERNATE) SOILS DATA

DEPTH (INCHES)	TEXTURE	MUNSELL COLOR
0-8	T.S Sandy Loam	7.5yR 3/2
8-14	Sandy Loam	7.5yR 4/4

Good Drainage for alt Site

ADDITIONAL SOIL BORINGS MAY BE REQUIRED

BM 100 Top of Ex Tank
99.2 Tank Outlet
Excisting Line at home
Excisting Tank inLet
101.7 Ground AT RockBed
99.3 Boreing To Res. Layer
103.0 Bottom of Rock
103.8 Dist Device
96.7 Pump Hght
7.1 Pump Discharge Hght