LILJENQUIST SEWER AND EXCAVATING

30477 State Hwy 47
Aitkin, MN 56431 | 320-684-2692 | liljenquistsewer@yahoo.com

April 20, 2022

Aitkin County Environmental Services

Permittee:
F.I.R.M. (Farm Island Repair and Marine)
28965 US – 169
Aitkin, MN 56431

Dear Aitkin County Environmental Services:

F.I.R.M. (Farm Island Repair & Marine) is applying for a new septic system. The following pertains to the new system that Liljenquist Sewer and Excavating will be installing.

- System designed with 3 stools
- 2 Bathrooms for customers. 1 for employees only
- Designed at 750 gallons per day
- Temporarily reusing existing 1500-gallon tank to pump over to new system
- Pump will be 20 G.P.M. at 5' of head
- When new building is built, we will install a new lift tank
- Septic tank sized at 750 G.P.D. x 3 = 2250
- We are pumping to tank 2250 x 1 ½ = 3375 gallons
- We will install 2-1870 combo tanks = 3740 gallons
- Tanks will be tarred and sealed around cover of tank for watertight integrity
- We will install a 760-gallon pump tank
- Tank will be tarred and sealed around cover of tank for watertight intergrity
- 2 pumps will be installed using an alternating control panel
- 3' sand base for the mound system due to saturated conditions.

Thank you,
Larry Liljenquist LIC# 127
Liljenquist Sewer and Excavating

FIELD EVALUATION SHEET

PRELIMINARY EVALUATION DATE 9-18-22, FIELD EVALUATION DATE 4-18-22
PROPERTY OWNER: OLSON INV. PROPERTIES (SCOTT BOYD) PHONE
ADDRESS: 28965 US HWY 169 CITY,STATE,ZIP: AITKIN MN 56431
LEGAL DESCRIPTION: 300 x 339 FT IN NE CORNER OF NW NW LESS .10 AC HWY
PIN# 07-0-074605 | 074900 SEC 33 T 46 R 27 TWP NAME FARM ISLAND
FIRE# LAKE/RIVER NONE LAKE CLASS NONE OHWL NONE FT.

DESCRIPTION OF SOIL TREATMENT AREAS

	AREA #1	AREA #2
DISTURBED AREAS	YES NO X	YES NO X
COMPACTED AREAS	YES NO X	YES NO X
FLOODING	YES NO X	YES NO X
RUN ON POTENTIAL	YES NO X	YES NO X
SLOPE %	0	
DIRECTION OF SLOPE		
LANDSCAPE POSITION	FIELD / GRASS	
VEGETATION TYPES	GRASS	

REFERENCE BM ELEV. 100 FT
REFERENCE BM DESCRIPTION
GROUND LEVEL BY
EXISTING TANK

DEPTH TO STANDING WATER OR MOTTLED SOIL: BORING# 1 0, 1A 0, 2 0, 2A 0

BOTTOM ELEVATION--FIRST TRENCH OR BOTTOM OF ROCK BED: #1 101 FT., #2 FT.

SOIL SIZING FACTOR: SITE # 1 1.27, SITE #2 1.27

CONSTRUCTION RELATED ISSUES: DESIGN FOR 750 G.P.D. TANKS: 2-1870 COMBOS 1-760 LIFT

LIC# 127 SITE EVALUATOR SIGNATURE: Larry Liljenquist
SITE EVALUATOR NAME: LARRY LILJENQUIST TELEPHONE# 218 820 8886

LUG REVIEW DATE

Comments:

SOIL BORING LOGS ON REVERSE SIDE

Form des 2/20/98

MOUND DESIGN WORK SHEET (For Flows up to 1200 gpd)

A. Average Design FLOW

Estimated 750 gpd *(see figure A-1)*
or measured _____ x 1.5 (safety *factor*) =_____ gpd

A-1: Estimated Sewage Flows in Gallons per Day

number of bedrooms	Class I	Class II	Class III	Class IV
2	300	225	180	60%
3	450	300	218	of the
4	600	375	256	values
5	750	450	294	in the
6	900	525	332	Class I,
7	1050	600	370	II, or III
8	1200	675	408	columns.

B. SEPTIC TANK Capacity

2-1870 gallons *(see figure C-1)*

C-1: Septic Tank Capacities (in gallons)

Number of Bedrooms	Minimum Liquid Capacity	Liquid capacity with garbage disposal	Liquid capacity with disposal& lift inside
2 or less	750	1125	1500
3 or 4	1000	1500	2000
5 or 6	1500	2250	3000
7, 8 or 9	2000	3000	4000

C. SOILS *(refer to site evaluation)*

1. Depth to restricting layer = 0 feet
2. Depth of percolation tests = _____ feet
3. Texture SANDY LOAM
   Percolation rate 6-15 mpi
4. Soil loading rate .79 gpd/sqft *(see figure D-33)*
5. Percent land slope 0 %

D. ROCK LAYER DIMENSIONS

1. Multiply average design flow (A) by 0.83 to obtain required rock layer area.
   750 gpd x 0.83 sqft/gpd – 630 sqft
2. Determine rock layer width = 0.83 sqft/gpd x linear Loading Rate (LLR)
   0.83 sqft/gpd x 12 gpd/sqft = 10 ft
3. Length of rock layer = area ÷ width =
   630 sqft (D1) ÷ 10 ft (D2) = 63 ft

Mound LLR	
< 120 MPI	≤12
≥ 120 MPI	≤6

E. ROCK VOLUME

1. Multiply rock area (D1) by rock depth of 1 ft to get cubic feet of rock
   630 sqft x 1 ft = 630 cuft
2. Divide cuft by 27 cuft/cuyd to get cubic yards
   630 cuft ÷ 27 cuyd/cuft = 24 cuyd
3. Multiply cubic yards by 1.4 to get weight of rock in tons
   24 cuyd x 1.4 ton/cuyd = 34 tons

F. SEWAGE ABSORPTION WIDTH

Absorption width equals absorption ratio (See Figure D-33) times rock layer width (D2)

1.5 x 10 ft = 15 ft

D-33: Absorption Width Sizing Table

Percolation Rate in Minutes per Inch (MPI)	Soil Texture	Loading Rate Gallons per day per square foot	Absorption Ratio
Faster than 5	Coarse Sand Medium Sand Loamy Sand Fine Sand	1.20	1.00
6 to 15	Sandy Loam	0.79	1.50
16 to 30	Loam	0.60	2.00
31 to 45	Silt Loam Silt	0.50	2.40
46 to 60	Sandy Clay Loam Silty Clay Loam Clay Loam	0.45	2.67
61 to 120	Silty Clay Sandy Clay Clay	0.24	5.00
Slower than 120*			

*System designed for these soils must be other or performance

SOILS CHARTS FOR BOTH PROPOSED AND ALTERNATE SITES

1 (PROPOSED) SOILS DATA

DEPTH (INCHES)	TEXTURE	MUNSELL COLOR
0-5	TOPSOIL	
5-13	SANDY LOAM	10YR 5/6

MOTTLING @ 6"

2 (PROPOSED) SOILS DATA

DEPTH (INCHES)	TEXTURE	MUNSELL COLOR
0-5	TOPSOIL	
5-13	SANDY LOAM	10YR 5/6

MOTTLING @ 6"

1 (ALTERNATE) SOILS DATA

DEPTH (INCHES)	TEXTURE	MUNSELL COLOR
0-5	TOPSOIL	
5-12	SANDY LOAM	10YR 5/6

MOTTLING @ 8"

2 (ALTERNATE) SOILS DATA

DEPTH (INCHES)	TEXTURE	MUNSELL COLOR
0-5	TOPSOIL	
5-12	SANDY LOAM	10YR 5/6

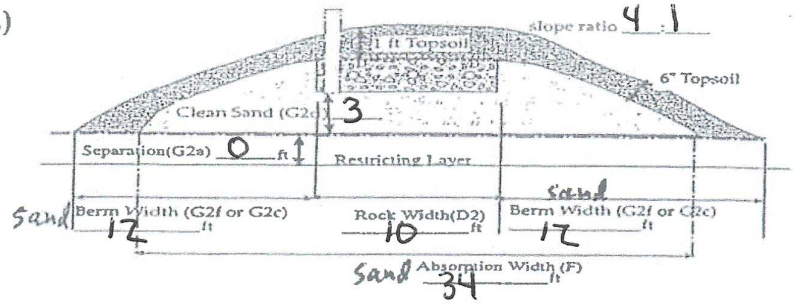
MOTTLING @ 8"

ADDITIONAL SOIL BORINGS MAY BE REQUIRED

<=1% land slope

3:1 SAND

G. Mound Slope Width and Length (landslope less than or equal to 1%)



1. Absorption width (F) 15 ft

2. Calculate mound size

a. Determine depth of clean sand fill at upslope edge of rock layer = 3 ft minus the distance to restricting layer (C1)

3 ft - 0 ft = 3 ft

b. Mound height at the upslope edge of rock layer = depth of clean sand for separation (G2a) at upslope edge plus depth of rock layer (1 ft) plus depth of cover (1 ft)

3 ft + 1ft + 1ft = 5 ft

c. Berm width = upslope mound height (G2b) times 4 *(4 is recommended, but could be 3-12)*

5 x 4 = 20 ft

d. The total landscape width is the sum of berm (G2c) width plus rock layer width (D2) plus berm wic (G2c): 20 ft + 10 ft + 20 ft = 50 ft

e. Additional width necessary for absorption = absorption width (F) minus the landscape width (G2d)

15 ft - 50 ft = -35 ft, *if number is negative (<0) skip to g*

f. Final berm width = additional width (G2e) plus the berm width (G2c)

____ ft + ____ ft = ____ ft

g. Total mound width is the sum of berm width (G2f or G2c) plus rock layer width (D2) plus berm width (G2f or G2c): 20 ft + 10 ft + 20 ft = 50 ft

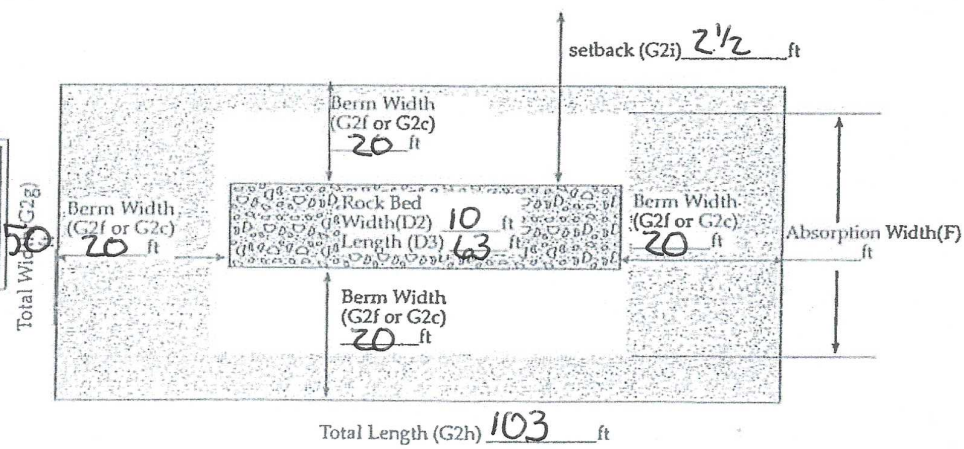
h. Total mound length is the sum of berm (G2f or G2c) plus rock layer length (D3) plus berm (G2f or G2c): 20 ft + 63 ft + 20 ft = 103 ft

i. Setbacks from the rockbed are calculated as follows: the absorption width (F) minus the rock bed wid (D2) divided by 2: (15 ft - 10 ft) ÷ 2 = 2½ ft

Final Dimensions:

50 x 103

SANDBASE
34' x 87'



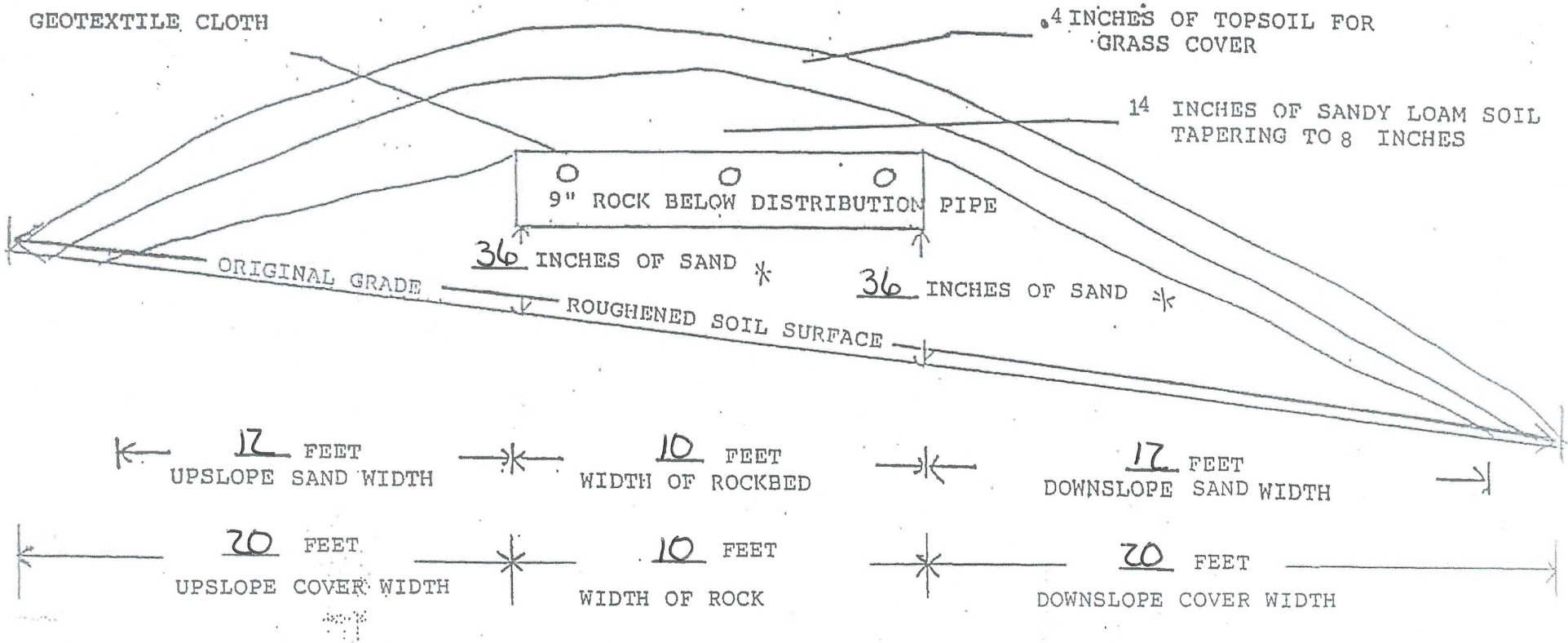
I hereby certify that I have completed this work in accordance with applicable ordinances, rules and laws.

Larry Lyngaas (signature) 127 (license #) 4-18-22 (date)

MOUND CROSS-SECTION

0 PERCENT SLOPE OF ORIGINAL SOIL

10 FT. x 63 FT. SIZE OF ROCKBED 34 FT. x 87 FT. SIZE OF SANDBASE



PUMP SELECTION PROCEDURE

1. Determine pump capacity: - PUMP FROM EXISTING TANK TO NEW SYSTEM

A. Gravity distribution

1. Minimum required discharge is 10 gpm
2. Maximum suggested discharge is 45 gpm. For other establishments at least 10% greater than the water supply rate, but no faster than the rate at which effluent will flow out of the distribution device.

B. Pressure distribution

See pressure distribution work sheet

From A or B Selected pump capacity: 20 gpm

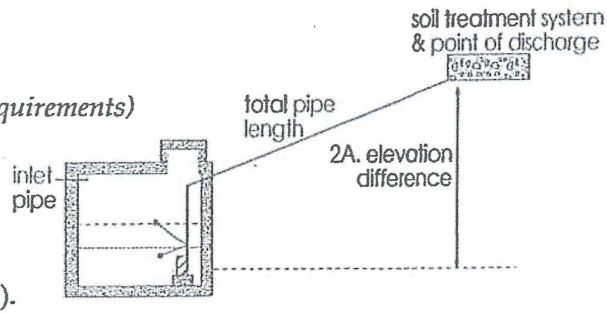
2. Determine pump head requirements:

A. Elevation difference between pump and point of discharge?

3 feet

B. Special head requirement? *(See Figure at right - Special Head Requirements)*

0 feet



Special Head Requirements	
Gravity Distribution	0 ft
Pressure Distribution	5 ft

C. Calculate Friction loss

1. Select pipe diameter 2 in
2. Enter *Figure E-9* with gpm (1A or B) and pipe diameter (C1).
   Read friction loss in feet per 100 feet from Figure E-9
   Friction Loss = .73 ft/100ft of pipe
3. Determine total pipe length from pump discharge to soil treatment discharge point. Estimate by adding 25 percent to pipe length for fitting loss. Total pipe length times 1.25 = equivalent pipe length
   210 feet x 1.25 = 262.5 feet
4. Calculate total friction loss by multiplying friction loss (C2) in ft/100 ft by the equivalent pipe length (C3) and divide by 100.
   = .73 ft/100ft x 262.5 ÷100 = 1.92 ft

E-9: Friction Loss in Plastic Pipe Per 100 feet			
	nominal pipe diameter		
flow rate gpm	1.5"	2"	3"
20	2.47	0.73	0.11
25	3.73	1.11	0.16
30	5.23	1.55	0.23
35	6.96	2.06	0.30
40	8.91	2.64	0.39
45	11.07	3.28	0.48
50	13.46	3.99	0.58
55		4.76	0.70
60		5.60	0.82
65		6.48	0.95
70		7.44	1.09

D. Total head required is the sum of elevation difference (A), special head requirements (B), and total friction loss (C4)

3 ft + 0 ft + 1.92 ft =

Total head: 4.92 feet

3. Pump selection

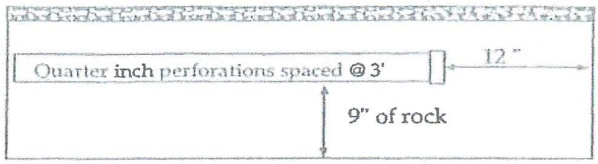
A pump must be selected to deliver at least 20 gpm (1A or B) with at least 4.92 feet of total head (2D)

I hereby certify that I have completed this work in accordance with applicable ordinances, rules and laws.

Larry Lyngus (signature) 127 (license #) 4-18-22 (date)

Pump 1 TO DRAINFIELD

PRESSURE DISTRIBUTION SYSTEM



Perf Sizing 3/16" - 1/4"
Perf Spacing 1.5'- 5'

1. Select number of perforated laterals 3

2. Select perforation spacing = 3 ft

3. Since perforations should not be placed closer than 1 foot to the edge of the rock layer (see diagram), subtract 2 feet from the rock layer length.

   31 1/2 (Rock layer length) - 2 ft = 29 1/2 ft

4. Determine the number of spaces between perforations. Divide the length (3) by perforation spacing (2) and round down to nearest whole number.

   Perforation spacing = 29 1/2 ft ÷ 3 ft = 9 spaces

5. Number of perforations is equal to one plus the number of perforation spaces(4). *Check figure E-4 to assure the number of perforations per lateral guarantees <10% discharge variation.*

   9 spaces + 1 = 10 perforations/lateral

6. A. Total number of perforations = perforations per lateral (5) times number of laterals (1)

   10 perfs/lat x 3 lat = 30 perforations

   B. Calculate the square footage per perforation.
   Should be 6-10 sqft/perf. *Does not apply to at-grades.*
   Rock bed area = rock width (ft) x rock length (ft)
   10 ft x 31 1/2 ft = 315 sqft
   Square foot per perforation = Rock bed area ÷ number of perfs (6)
   315 sqft ÷ 30 perfs = 10.5 sqft/perf

7. Determine required flow rate by multiplying the total number of perforations (6A) by flow per perforation *(see figure E-6)*

   30 perfs x .74 gpm/perfs = 22.2 gpm

8. If laterals are connected to header pipe as shown on upper example, to select minimum required lateral diameter; enter figure E-4 with perforation spacing (2) and number of perforations per lateral (5) Select minimum diameter for perforated lateral = 1 1/4 inches.

9. If perforated lateral system is attached to manifold pipe near the center, lower diagram, perforated lateral length (3) and number of perforations per lateral (5) will be approximately one half of that in step 8. Using these values, select minimum diameter for perforated lateral = _____ inches.

E-4: Maximum allowable number of 1/4-inch perforations per lateral to guarantee <10% discharge variation

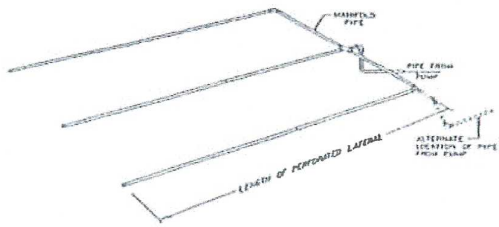
perforation spacing (feet)	1 inch	1.25 inch	1.5 inch	2.0 inch
2.5	8	14	18	28
3.0	8	13	17	26
3.3	7	12	16	25
4.0	7	11	15	23
5.0	6	10	14	22

E-6: Perforation Discharge in gpm

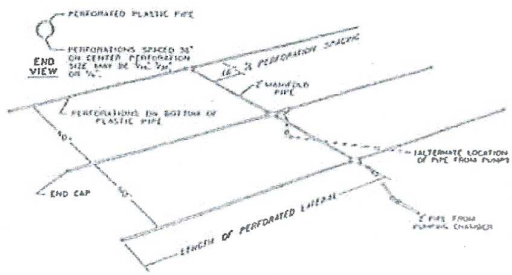
head (feet)	perforation diameter (inches) 1/8	3/16	7/32	1/4
1.0[a]	0.18	0.42	0.56	0.74
2.0[b]	0.26	0.59	0.80	1.04
5.0	0.41	0.94	1.26	1.65

[a] Use 1.0 foot for single-family homes.
[b] Use 2.0 feet for anything else.

MANIFOLD LOCATED AT END OF PRESSURE DISTRIBUTION SYSTEM



LAYOUT OF PERFORATED PIPE LATERALS FOR PRESSURE DISTRIBUTION IN MOUND



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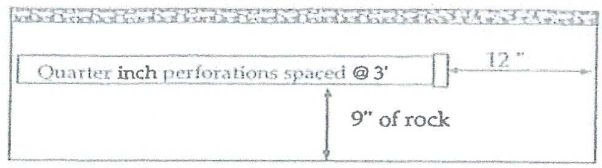
Larry Lymgual (signature) 127 (license #) 4-18-22 (date)

Pump 2 TO DRAINFIELD

PRESSURE DISTRIBUTION SYSTEM



Perf Sizing 3/16" - 1/4"
Perf Spacing 1.5'- 5'

1. Select number of perforated laterals 3

2. Select perforation spacing = 3 ft

3. Since perforations should not be placed closer than 1 foot to the edge of the rock layer (see diagram), subtract 2 feet from the rock layer length.

   31 1/2 (Rock layer length) - 2 ft = 29 1/2 ft

4. Determine the number of spaces between perforations. Divide the length (3) by perforation spacing (2) and round down to nearest whole number.

   Perforation spacing = 29 1/2 ft ÷ 3 ft = 9 spaces

5. Number of perforations is equal to one plus the number of perforation spaces(4). *Check figure E-4 to assure the number of perforations per lateral guarantees <10% discharge variation.*

   9 spaces + 1 = 10 perforations/lateral

6. A. Total number of perforations = perforations per lateral (5) times number of laterals (1)

   10 perfs/lat x 3 lat = 30 perforations

   B. Calculate the square footage per perforation.
   Should be 6-10 sqft/perf. *Does not apply to at-grades.*
   Rock bed area = rock width (ft) x rock length (ft)
   10 ft x 31 1/2 ft = 315 sqft
   Square foot per perforation = Rock bed area ÷ number of perfs (6)
   315 sqft ÷ 30 perfs = 10.5 sqft/perf

7. Determine required flow rate by multiplying the total number of perforations (6A) by flow per perforation *(see figure E-6)*

   30 perfs x .74 gpm/perfs = 22.2 gpm

8. If laterals are connected to header pipe as shown on upper example, to select minimum required lateral diameter; enter figure E-4 with perforation spacing (2) and number of perforations per lateral (5) Select minimum diameter for perforated lateral = 1 1/4 inches.

9. If perforated lateral system is attached to manifold pipe near the center, lower diagram, perforated lateral length (3) and number of perforations per lateral (5) will be approximately one half of that in step 8. Using these values, select minimum diameter for perforated lateral = _____ inches.

E-4: Maximum allowable number of 1/4-inch perforations per lateral to guarantee <10% discharge variation

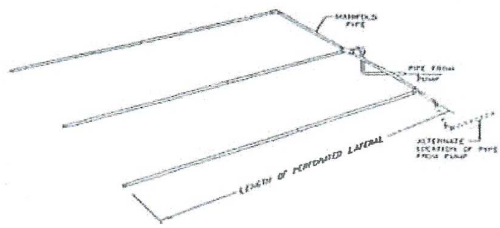
perforation spacing (feet)	1 inch	1.25 inch	1.5 inch	2.0 inch
2.5	8	14	18	28
3.0	8	13	17	26
3.3	7	12	16	25
4.0	7	11	15	23
5.0	6	10	14	22

E-6: Perforation Discharge in gpm

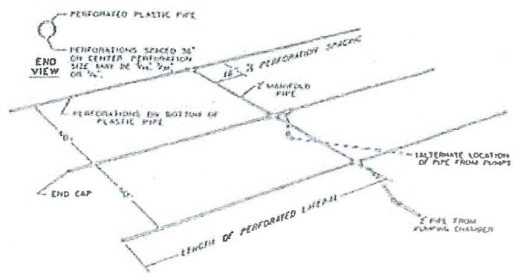
head (feet)	perforation diameter (inches) 1/8	3/16	7/32	1/4
1.0[a]	0.18	0.42	0.56	0.74
2.0[b]	0.26	0.59	0.80	1.04
5.0	0.41	0.94	1.26	1.65

[a] Use 1.0 foot for single-family homes.
[b] Use 2.0 feet for anything else.

MANIFOLD LOCATED AT END OF PRESSURE DISTRIBUTION SYSTEM



LAYOUT OF PERFORATED PIPE LATERALS FOR PRESSURE DISTRIBUTION IN MOUND



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Larry Ljungust (signature) 127 (license #) 4-18-22 (date)

PUMP SELECTION PROCEDURE

1. Determine pump capacity: – BOTH PUMPS ARE AT THE SAME ELEVATION AND DISTANCE

A. Gravity distribution

1. Minimum required discharge is 10 gpm
2. Maximum suggested discharge is 45 gpm. For other establishments at least 10% greater than the water supply rate, but no faster than the rate at which effluent will flow out of the distribution device.

B. Pressure distribution

See pressure distribution work sheet

From A or B Selected pump capacity: 22.2 gpm

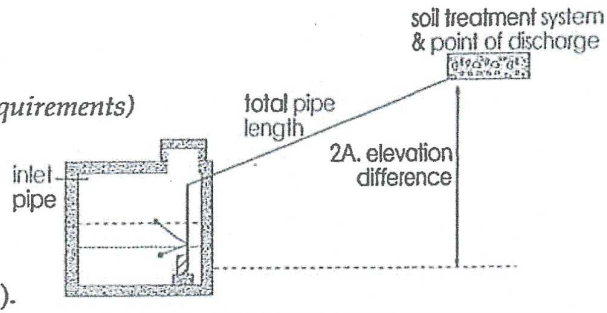
2. Determine pump head requirements:

A. Elevation difference between pump and point of discharge?

8 feet

B. Special head requirement? *(See Figure at right - Special Head Requirements)*

5 feet



Special Head Requirements	
Gravity Distribution	0 ft
Pressure Distribution	5 ft

C. Calculate Friction loss

1. Select pipe diameter 2 in
2. Enter *Figure E-9* with gpm (1A or B) and pipe diameter (C1).
   Read friction loss in feet per 100 feet from Figure E-9
   Friction Loss = 1.11 ft/100ft of pipe
3. Determine total pipe length from pump discharge to soil treatment discharge point. Estimate by adding 25 percent to pipe length for fitting loss. Total pipe length times 1.25 = equivalent pipe length
   57 feet x 1.25 = 71.25 feet
4. Calculate total friction loss by multiplying friction loss (C2) in ft/100 ft by the equivalent pipe length (C3) and divide by 100.
   = 1.11 ft/100ft x 71.25 ÷100 = .8 ft

E-9: Friction Loss in Plastic Pipe Per 100 feet

flow rate gpm	nominal pipe diameter 1.5"	2"	3"
20	2.47	0.73	0.11
25	3.73	1.11	0.16
30	5.23	1.55	0.23
35	6.96	2.06	0.30
40	8.91	2.64	0.39
45	11.07	3.28	0.48
50	13.46	3.99	0.58
55		4.76	0.70
60		5.60	0.82
65		6.48	0.95
70		7.44	1.09

D. Total head required is the sum of elevation difference (A), special head requirements (B), and total friction loss (C4)

8 ft + 5 ft+ .8 ft =

Total head: 13.8 feet

3. Pump selection

A pump must be selected to deliver at least 22.2 gpm (1A or B) with at least 13.8 feet of total head (2D)

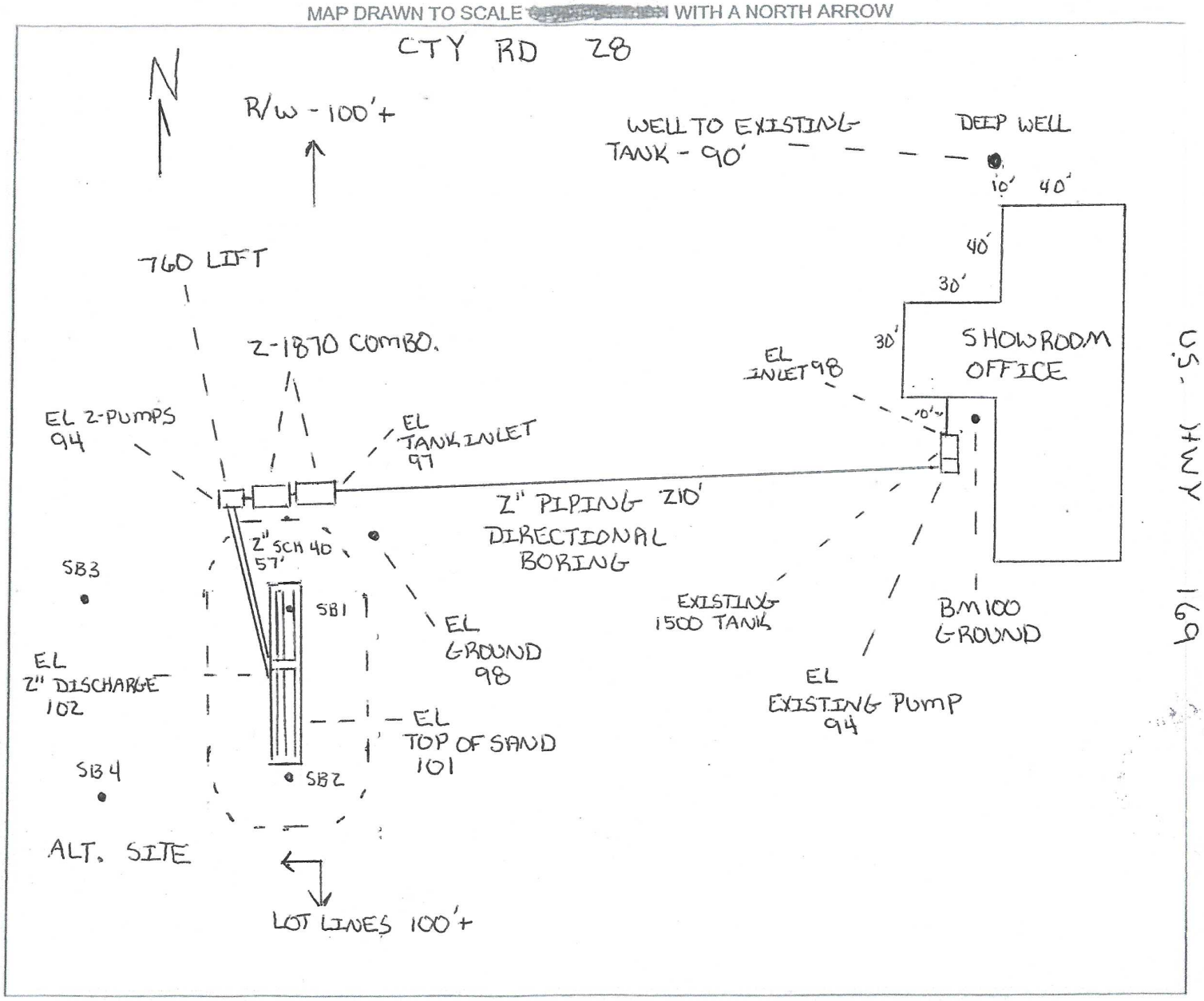
I hereby certify that I have completed this work in accordance with applicable ordinances, rules and laws.

Larry Lyngus (signature) 127 (license #) 4-18-22 (date)

SKETCH SHEET

CLIENT: F. I. R. M. DATE: 4-18-22

MAP DRAWN TO SCALE [redacted] WITH A NORTH ARROW



CHECK OFF LIST--HAVE ALL OF THE FOLLOWING BEEN DRAWN ON THE MAP??

SHOW EXISTING OR PROPOSED

- ☐ WATER WELLS WITHIN 100 FT OF TREATMENT AREAS
- ☐ PRESSURE WATER LINES WITHIN 10 FT OF TREATMENT AREAS
- ☐ STRUCTURES
- ☐ LOT IMPROVEMENTS
- ☐ ALL SOIL TREATMENT AREAS
- ☐ ALL ISTS COMPONENTS
- ☐ HORIZONTAL AND VERTICALREFERENCE
- ☐ POINT OF SOIL BORINGS
- ☐ DIRECTION OF SLOPE
- ☐ LOT EASEMENTS
- ☐ ALL LOT DIMENSIONS
- ☐ DISTURBED/ COMPACTED AREAS
- ☐ SITE PROTECTION--LATHE AND RIBBON EVERY 15 FT
- ☐ ACCESS ROUTE FOR TANK MAINTENANCE

REQUIRED SETBACKS

- ☐ STRUCTURES
- ☐ PROPERTY LINES
- ☐ OHWL

COMMENTS:

INDICATE ELEVATIONS

BENCHMARK 100
ELEVATION OF SEWER LINE @ HOUSE
ELEVATION @ TANK INLET 98
ELEVATION @ BOTTOM OF ROCK LAYER 101
ELEVATION @ BOTTOM OF BORING OR RESTRICTIVE LAYER 97
ELEVATION OF PUMP 94
ELEVATION OF DISTRIBUTION DEVICE 102

DESIGNER SIGNATURE Larry Hymquist
LICENSE# 127

DATE 4-18-22

Subsurface Sewage Treatment System Management Plan

Property Owner: OLSON INV. / SCOTT BOYD Phone: _____ Date: 4-18-22

Mailing Address: 28965 US HWY 169 S City: AITKIN Zip: 56431

Site Address: SAME AS ABOVE City: _____ Zip: _____

This management plan will identify the operation and maintenance activities necessary to ensure long-term performance of your septic system. Some of these activities must be performed by you, the homeowner. Other tasks must be performed by a licensed septic service provider or maintenance provider.

System Designer: Recommends SSTS check every 12 months.
Local Government: Recommends SSTS check every _____ months.
State Requirement: Requires SSTS check every 36 months.
(State requirements are based on MN Rules Chapter 7080.2450, Subp. 2 & 3)

My System needs to be checked every 12 months.

Homeowner Management Tasks:

Leaks – Check (look, listen) for leaks in toilets and dripping faucets. Repair leaks promptly.

Surfacing sewage – Regularly check for wet or spongy soil around your soil treatment area.

Effluent filter – Inspect and clean twice a year or more.

Alarms – Alarm signals when there is a problem. Contact a service or maintenance provider any time an alarm signals.

Event counter or water meter – Record your water use.

-recommend meter readings be conducted (*circle one:* DAILY WEEKLY MONTHLY N/A)

Licensed septic service provider or maintenance provider (Check all that apply):

- ☐ Check to make sure tank is not leaking
- ☐ Check and clean the in-tank effluent filter (if exists)
- ☐ Check the sludge/scum layer levels in all septic tanks
- ☐ Recommend if tank should be pumped
- ☐ Check inlet and outlet baffles
- ☐ Check the drainfield effluent levels in the rock layer
- ☐ Check the pump and alarm system functions
- ☐ Check wiring for corrosion and function
- ☐ Check dissolved oxygen and effluent temperature in tank
- ☐ Provide homeowner with list of results and any action to be taken
- ☐ Flush and clean laterals if cleanouts exist

"I understand it is my responsibility to properly operate and maintain the sewage treatment system on this property, utilizing the Management Plan. If requirements in the Management Plan are not met, I will promptly notify the permitting authority and take necessary corrective actions. If I have a new system, I agree to adequately protect the reserve area for future use as a soil treatment system."

Property Owner Signature: _____ Date: _____

Designer Signature: Larry Lynaugh Date: 4-18-22

See Reverse Side for Management Log

Maintenance Log

Activity	Date Accomplished									
Check frequently:										
Leaks: check for plumbing leaks										
Soil treatment area check for surfacing										
Lint filter: check, clean if needed										
Effluent screen: if owner-maintained										
Water usage rate (monitor frequency _____)										
Check annually:										
Caps: inspect, replace if needed										
Sludge & Scum/Pump										
Inlet & Outlet baffles										
Drainfield effluent leaks										
Pump, alarm, wiring										
Flush & clean laterals if cleanouts exists										
Other:_____										
Other:_____										

Notes:_____