

FIELD EVALUATION SHEET

PRELIMINARY EVALUATION DATE _____, FIELD EVALUATION DATE 8/19/2020
PROPERTY OWNER: Joshi STEVENSON PHONE 651-335-2118
ADDRESS: 347 River Rd CITY, STATE, ZIP: Hudson WI 54016
LEGAL DESCRIPTION: _____
PIN# 11-0-074100 SEC 33 T 45 R 27 TWP NAME Hazelton
FIRE# 43562 LAKE/RIVER Mille Lacs LAKE CLASS _____ OHWL _____ FT.

DESCRIPTION OF SOIL TREATMENT AREAS

	AREA #1	AREA #2	REFERENCE BM ELEV. _____ FT.
DISTURBED AREAS	YES ___ NO <u>X</u>	YES ___ NO ___	REFERENCE BM DESCRIPTION _____
COMPACTED AREAS	YES ___ NO <u>✓</u>	YES ___ NO ___	_____
FLOODING	YES ___ NO <u>X</u>	YES ___ NO ___	_____
RUN ON POTENTIAL	YES ___ NO <u>✓</u>	YES ___ NO ___	_____
SLOPE %	<u>3</u>	_____	_____
DIRECTION OF SLOPE	<u>N</u>	_____	_____
LANDSCAPE POSITION	<u>Shoulder</u>	_____	_____
VEGETATION TYPES	<u>Maple Pine</u>	_____	_____

DEPTH TO STANDING WATER OR MOTTLED SOIL: BORING# 1 2', 1A 2', 2 _____, 2A _____

BOTTOM ELEVATION--FIRST TRENCH OR BOTTOM OF ROCK BED: #1 _____ FT., #2 _____ FT.

SOIL SIZING FACTOR: SITE #1 1.67, SITE #2 _____

CONSTRUCTION RELATED ISSUES: _____

LIC# 747 SITE EVALUATOR SIGNATURE: Raymond Schrupp

SITE EVALUATOR NAME: Raymond Schrupp TELEPHONE# 218-587-4038

LUG REVIEW _____ DATE _____

Comments: _____

SOIL BORING LOGS ON REVERSE SIDE

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SOILS CHARTS FOR BOTH PROPOSED AND ALTERNATE SITES

1 (PROPOSED) SOILS DATA

DEPTH (INCHES)	TEXTURE	MUNSELL COLOR
0 to 8	T.S	7.5R3/3
8 to 24"	loam	7.5R4/4
	Mottled at 28"	

2 (PROPOSED) SOILS DATA

DEPTH (INCHES)	TEXTURE	MUNSELL COLOR
Same Sals as 1		

1 (ALTERNATE) SOILS DATA

DEPTH (INCHES)	TEXTURE	MUNSELL COLOR
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2 (ALTERNATE) SOILS DATA

DEPTH (INCHES)	TEXTURE	MUNSELL COLOR
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ADDITIONAL SOIL BORINGS MAY BE REQUIRED

MOUND DESIGN WORK SHEET (For Flows up to 1200 gpd)

A. Average Design FLOW

Estimated 600 gpd (see figure A-1)
 or measured _____ x 1.5 (safety factor) = _____ gpd

A-1: Estimated Sewage Flows in Gallons per Day

number of bedrooms	Class I	Class II	Class III	Class IV
2	300	225	180	60%
3	450	300	218	of the
4	600	375	256	values
5	750	450	294	in the
6	900	525	332	Class I,
7	1050	600	370	II, or III
8	1200	675	408	columns.

B. SEPTIC TANK Capacity

1500 gallons (see figure C-1)

C-1: Septic Tank Capacities (in gallons)

Number of Bedrooms	Minimum Liquid Capacity	Liquid capacity with garbage disposal	Liquid capacity with disposal & lift inside
2 or less	750	1125	1500
3 or 4	1000	1500	2000
5 or 6	1500	2250	3000
7, 8 or 9	2000	3000	4000

C. SOILS (refer to site evaluation)

- Depth to restricting layer = 2 feet
- Depth of percolation tests = _____ feet
- Texture loam
 Percolation rate 1.67 mpi
- Soil loading rate 60 gpd/sqft (see figure D-33)
- Percent land slope 3 %

D. ROCK LAYER DIMENSIONS

- Multiply average design flow (A) by 0.83 to obtain required rock layer area.
600 gpd x 0.83 sqft/gpd = 500 sqft
- Determine rock layer width = 0.83 sqft/gpd x linear Loading Rate (LLR)
 0.83 sqft/gpd x 600 gpd/sqft = 500 ft
- Length of rock layer = area ÷ width =
500 sqft (D1) ÷ 10 ft (D2) = 50 ft

Mound LLR	
< 120 MPI	≤ 12
≥ 120 MPI	≤ 6

E. ROCK VOLUME

- Multiply rock area (D1) by rock depth of 1 ft to get cubic feet of rock
500 sqft x 1 ft = 500 cuft
- Divide cuft by 27 cuft/cuyd to get cubic yards
500 cuft ÷ 27 cuyd/cuft = 19 cuyd
- Multiply cubic yards by 1.4 to get weight of rock in tons
19 cuyd x 1.4 ton/cuyd = 26 tons

F. SEWAGE ABSORPTION WIDTH

Absorption width equals absorption ratio (See Figure D-33) times rock layer width (D2)

200 x 10 ft = 20 ft

D-33: Absorption Width Sizing Table

Percolation Rate in Minutes per Inch (MPI)	Soil Texture	Loading Rate Gallons per day per square foot	Absorption Ratio
Faster than 5	Coarse Sand Medium Sand Loamy Sand Fine Sand	1.20	1.00
6 to 15	Sandy Loam	0.79	1.50
16 to 30	Loam	0.60	2.00
31 to 45	Silt Loam Silt	0.50	2.40
46 to 60	Sandy Clay Loam Silty Clay Loam Clay Loam	0.45	2.67
61 to 120	Silty Clay Sandy Clay Clay	0.24	5.00
Slower than 120*			

*System designed for these soils must be other or performance

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G. MOUND SLOPE WIDTH & LENGTH

(landslope greater than 1%)

1. Downslope absorption width = absorption width (F) minus rock layer width (D2)

$20 \text{ ft} - 10 \text{ ft} = 10 \text{ ft}$

2. Calculate mound size

UPSLOPE

a. Depth of clean sand fill at upslope edge of rock layer = 3 ft minus the distance to restricting layer (C1)

$3 \text{ ft} - 2 \text{ ft} = 1 \text{ ft}$

b. Mound height at the upslope edge of rock layer = depth of clean sand for separation (G2a) at upslope edge plus depth of rock layer (1 ft) plus depth of cover (1 ft)

$1 \text{ ft} + 1 \text{ ft} + 1 \text{ ft} = 3 \text{ ft}$

c. Upslope berm multiplier based on land slope

2.75 (see figure D-34)

d. Upslope width = berm multiplier (G2c) x upslope mound height (G2b):

$2.75 \times 3 \text{ ft} = 8.2 \text{ ft}$

DOWNSLOPE

e. Drop in elevation = rock layer width (D2) x percent landslope (C5) ÷ 100

$10 \text{ ft} \times 3\% \div 100 = 3 \text{ ft}$

f. Downslope mound height = depth of clean sand for slope difference (G2e) at downslope rock edge plus the mound height at the upslope edge of rock layer (G2b)

$3 \text{ ft} + 3 \text{ ft} = 6 \text{ ft}$

g. Downslope berm multiplier based on percent land slope

3.30 (see figure D-34)

h. Downslope width = downslope multiplier (G2g) times downslope mound height (G2f)

$3.30 \times 6 \text{ ft} = 11 \text{ ft}$

i. Select the greater of G1 and G2h as the downslope width: 11 ft

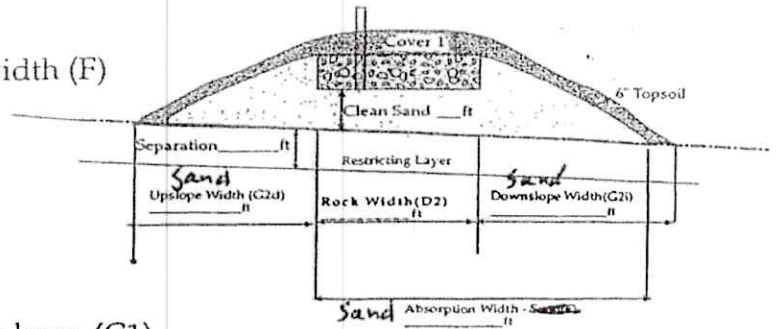
j. Total mound width is the sum of upslope width (G2d) width plus rock layer width (D2) plus downslope width (G2i)

$8.2 \text{ ft} + 10 \text{ ft} + 11 \text{ ft} = 30 \text{ ft}$

k. Total mound length is the sum of upslope width (G2d) plus rock layer length (D3) plus upslope width (G2d)

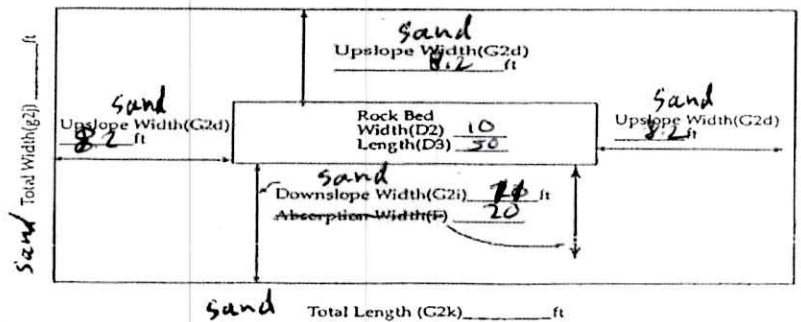
$8.2 \text{ ft} + 50 \text{ ft} + 8.2 \text{ ft} = 67 \text{ feet}$

Landslope > 1% slope



D-34: SLOPE MULTIPLIER TABLE

Land Slope in %	UPSLOPE multipliers for various slope ratios						DOWNSLOPE multipliers for various slope ratios				
	3:1	4:1	5:1	6:1	7:1	8:1	3:1	4:1	5:1	6:1	7:1
0	3.0	4.0	5.0	6.0	7.0	8.0	3.0	4.0	5.0	6.0	7.0
1	2.91	3.85	4.76	5.66	6.54	7.41	3.09	4.17	5.26	6.38	7.53
2	2.83	3.70	4.54	5.36	6.14	6.90	3.19	4.35	5.56	6.82	8.14
3	2.75	3.57	4.35	5.08	5.79	6.45	3.30	4.54	5.88	7.32	8.86
4	2.68	3.45	4.17	4.84	5.46	6.06	3.41	4.76	6.25	7.89	9.72
5	2.61	3.33	4.00	4.62	5.19	5.71	3.53	5.00	6.67	8.57	10.77
6	2.54	3.23	3.85	4.41	4.93	5.41	3.66	5.26	7.14	9.38	12.07
7	2.48	3.12	3.70	4.23	4.70	5.13	3.80	5.56	7.69	10.34	13.73
8	2.42	3.03	3.57	4.05	4.49	4.88	3.95	5.88	8.33	11.54	15.91
9	2.36	2.94	3.45	3.90	4.30	4.65	4.11	6.25	9.09	13.04	18.92
10	2.31	2.86	3.33	3.75	4.12	4.44	4.29	6.67	10.00	15.00	23.33
11	2.26	2.78	3.23	3.61	3.95	4.26	4.48	7.14	11.11	17.65	30.43
12	2.21	2.70	3.12	3.49	3.80	4.08	4.69	7.69	12.50	21.43	43.75



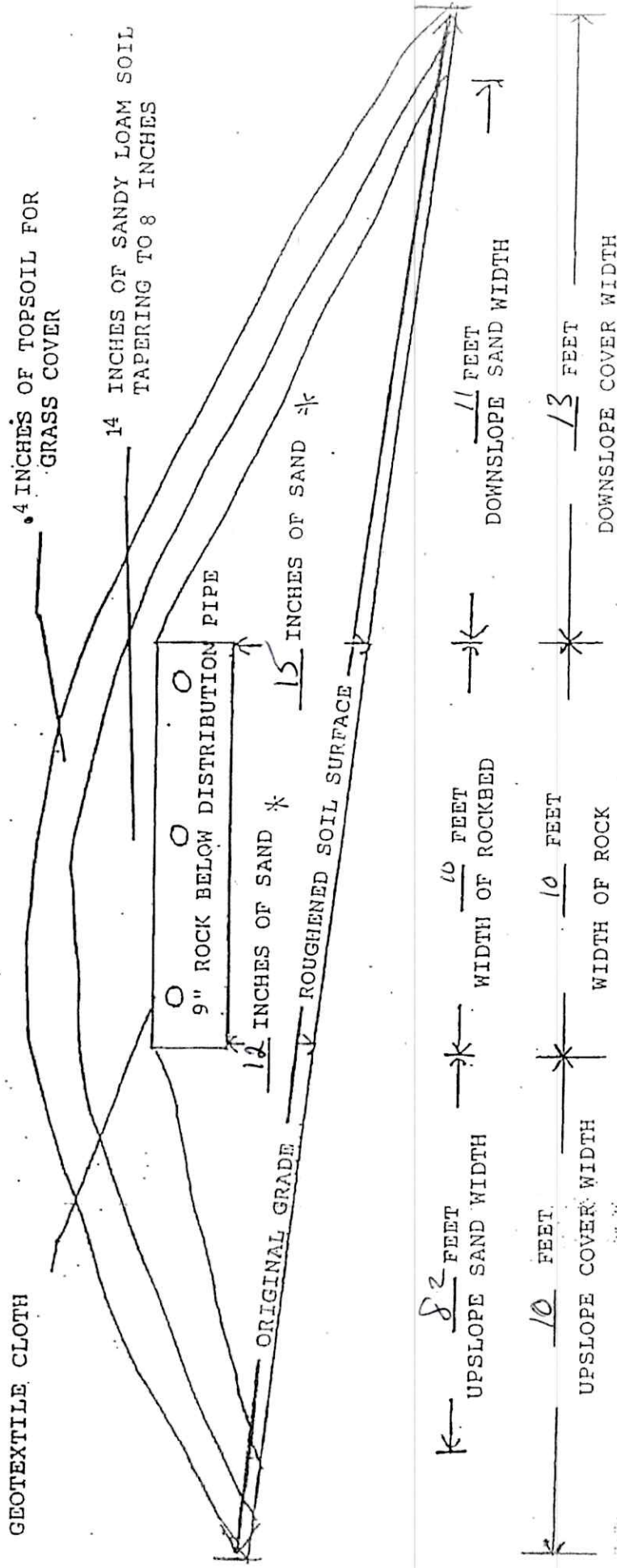
Final Dimensions:
30 x 67

I hereby certify that I have completed this work in accordance with applicable ordinances, rules and laws.
 (signature) 747 (license #) 8-19-2020 (date)

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mound cross-section

3 PERCENT SLOPE OF ORIGINAL SOIL 10 FT. X 50 FT. SIZE OF ROCKBED 36 FT. X 66 FT. SIZE OF SANDBASE



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PRESSURE DISTRIBUTION SYSTEM

- Select number of perforated laterals 3
- Select perforation spacing = _____ ft
- Since perforations should not be placed closer than 1 foot to the edge of the rock layer (see diagram), subtract 2 feet from the rock layer length.

$$\frac{50}{\text{Rock layer length}} - 2 \text{ ft} = \underline{48} \text{ ft}$$

- Determine the number of spaces between perforations. Divide the length (3) by perforation spacing (2) and round down to nearest whole number.

$$\text{Perforation spacing} = \underline{48} \text{ ft} \div \underline{3} \text{ ft} = \underline{16} \text{ spaces}$$

- Number of perforations is equal to one plus the number of perforation spaces(4). Check figure E-4 to assure the number of perforations per lateral guarantees <10% discharge variation.

$$\underline{16} \text{ spaces} + 1 = \underline{17} \text{ perforations/lateral}$$

- A. Total number of perforations = perforations per lateral (5) times number of laterals (1)

$$\underline{17} \text{ perfs/lat} \times \underline{3} \text{ lat} = \underline{51} \text{ perforations}$$

- B. Calculate the square footage per perforation. Should be 6-10 sqft/perf. Does not apply to at-grades.

$$\text{Rock bed area} = \text{rock width (ft)} \times \text{rock length (ft)}$$

$$\underline{10} \text{ ft} \times \underline{50} \text{ ft} = \underline{500} \text{ sqft}$$

$$\text{Square foot per perforation} = \text{Rock bed area} \div \text{number of perfs (6)}$$

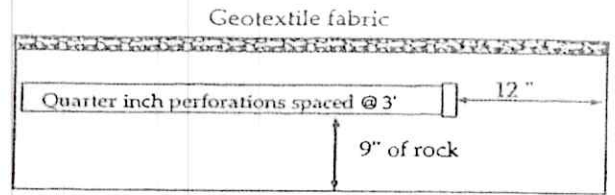
$$\underline{500} \text{ sqft} \div \underline{51} \text{ perfs} = \underline{10} \text{ sqft/perf}$$

- Determine required flow rate by multiplying the total number of perforations (6A) by flow per perforation (see figure E-6)

$$\underline{51} \text{ perfs} \times \underline{24} \text{ gpm/perfs} = \underline{38} \text{ gpm}$$

- If laterals are connected to header pipe as shown on upper example, to select minimum required lateral diameter; enter figure E-4 with perforation spacing (2) and number of perforations per lateral (5) Select minimum diameter for perforated lateral = 1 1/2 inches.

- If perforated lateral system is attached to manifold pipe near the center, lower diagram, perforated lateral length (3) and number of perforations per lateral (5) will be approximately one half of that in step 8. Using these values, select minimum diameter for perforated lateral = _____ inches.



Perf Sizing 3/16" - 1/4"
Perf Spacing 1.5' - 5'

E-4: Maximum allowable number of 1/4-inch perforations per lateral to guarantee <10% discharge variation

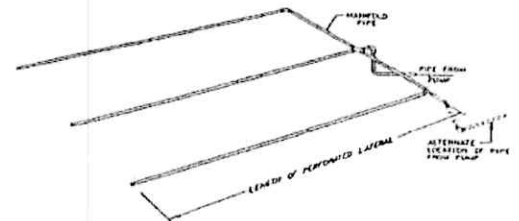
perforation spacing (feet)	1 inch	1.25 inch	1.5 inch	2.0 inch
2.5	8	14	18	28
3.0	8	13	17	26
3.3	7	12	16	25
4.0	7	11	15	23
5.0	6	10	14	22

E-6: Perforation Discharge in gpm

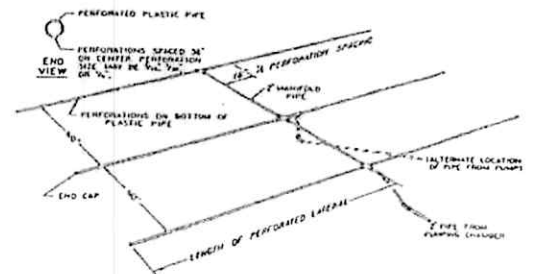
head (feet)	perforation diameter (Inches)			
	1/8	3/16	7/32	1/4
1.0 ^a	0.18	0.42	0.56	0.74
2.0 ^b	0.26	0.59	0.80	1.04
5.0	0.41	0.94	1.26	1.65

^a Use 1.0 foot for single-family homes.
^b Use 2.0 feet for anything else.

MANIFOLD LOCATED AT END OF PRESSURE DISTRIBUTION SYSTEM



LAYOUT OF PERFORATED PIPE LATERALS FOR PRESSURE DISTRIBUTION IN MOUND



I hereby certify that I have completed this work in accordance with applicable ordinances, rules and laws.

Josh Stevenson (signature)

(signature)

747 (license #)

(license #)

8-19-2020 (date)

(date)

PUMP SELECTION PROCEDURE

1. Determine pump capacity:

A. Gravity distribution

1. Minimum required discharge is 10 gpm
2. Maximum suggested discharge is 45 gpm. For other establishments at least 10% greater than the water supply rate, but no faster than the rate at which effluent will flow out of the distribution device.

B. Pressure distribution

See pressure distribution work sheet

From A or B Selected pump capacity: 38 gpm

2. Determine pump head requirements:

A. Elevation difference between pump and point of discharge?

10 feet

B. Special head requirement? (See Figure at right - Special Head Requirements)

5 feet

C. Calculate Friction loss

1. Select pipe diameter 2 in

2. Enter Figure E-9 with gpm (1A or B) and pipe diameter (C1).

Read friction loss in feet per 100 feet from Figure E-9

Friction Loss = 2.06 ft/100ft of pipe

3. Determine total pipe length from pump discharge to soil treatment discharge point. Estimate by adding 25 percent to pipe length for fitting loss. Total pipe length times 1.25 = equivalent pipe length

25 feet x 1.25 = 31 feet

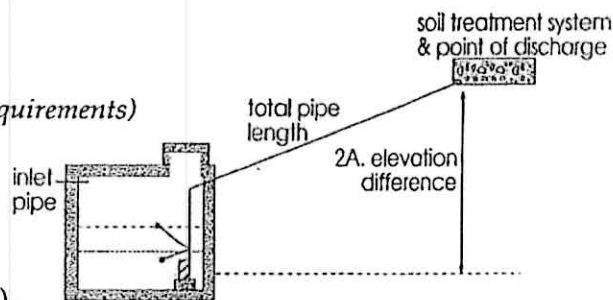
4. Calculate total friction loss by multiplying friction loss (C2) in ft/100 ft by the equivalent pipe length (C3) and divide by 100.

= 2.06 ft/100ft x 31 ÷ 100 = 1 ft

D. Total head required is the sum of elevation difference (A), special head requirements (B), and total friction loss (C4)

10 ft + 5 ft + 1 ft = 16

Total head: 16 feet



Special Head Requirements	
Gravity Distribution	0 ft
Pressure Distribution	5 ft

flow rate gpm	E-9: Friction Loss in Plastic Pipe Per 100 feet		
	nominal pipe diameter		
	1.5"	2"	3"
20	2.47	0.73	0.11
25	3.73	1.11	0.16
30	5.23	1.55	0.23
35	6.96	2.06	0.30
40	8.91	2.64	0.39
45	11.07	3.28	0.48
50	13.46	3.99	0.58
55		4.76	0.70
60		5.60	0.82
65		6.48	0.95
70		7.44	1.09

3. Pump selection

A pump must be selected to deliver at least 38 gpm (1A or B) with at least 16 feet of total head (2D)

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Josh Stevenson (signature)

(signature)

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(license #)

8-19-2020 (date)

(date)

DOSING CHAMBER SIZING

1. Determine area

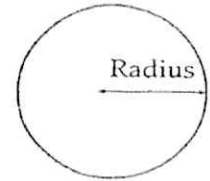
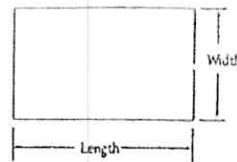
A. Rectangle area = L x W

$5 \times 6.5 = 32.5$ square feet

B. Circle area = π (3.14) x radius in feet x radius in feet

$3.14 \times \text{ft} \times \text{ft} = \text{sqft}$

C. Get area from manufacturer _____ sqft



2. Calculate gallons per inch

There are 7.5 gallons per cubic foot of volume, therefore multiply the area (1A, B or C) times the conversion factor and divide by 12 inches per foot to calculate gallon per inch.

Area x 7.5 ÷ 12 = _____ sqft x 7.5 ÷ 12 in/ft = 15.5 gallon per inch

3. Calculate total tank volume

A. Depth from bottom of inlet pipe to tank bottom 42 in

B. Total tank volume = depth from bottom of inlet pipe to tank bottom (3A) x gal/in (2)
= 42 in x 15.5 gal/in = 651 gal

4. Calculate gallons to cover pump (with 2-3 inches of water covering pump)

(Pump and block height (inch) + 2 inch) x gallon/inch

(16 in + 2 in) x 15.5 gal/in = 279 gallon

5. Calculate total pumpout volume

A. Select pump size for 4-5 does per day. Gallon per dose = gpd (see figure A-1) / doses per day = 600 gpd ÷ 4 doses/day = 150 gallons

B. Calculate drainback

1. Determine total pipe length, 20 feet

2. Determine liquid volume of pipe, 0.12 gal per ft (see figure E-20)

3. Drainback quantity = 20 ft (5B1) x 0.12 gal per ft (5B2) = 2.4 gal

C. Total pump out volume = dose volume (5A) + drainback (5B3)

150 gal + 2.4 gal = 154 Total gallon

6. Float separation distance (using total pumpout volume)

Total pumpout volume (5C) ÷ gal/inch (2)

154 gal ÷ 15.5 gal/in = 10 inch

7. Calculate volume for alarm (typically 2 to 3 inches)

Alarm depth (inch) x gallon/inch (2) = 15.5 in x 3 gal/in = 45 gal

8. Calculate total gallon = gallons over pump (4) + gallons pumpout (5C) + gallons alarm (7)

279 gal + 154 gal + 45 gal = 478 gallons

9. Total Tank Depth = total gallon (8) ÷ gallon/inch (2)

478 gal ÷ 15.5 gal/in = 31 in

Recommended:

Calculate reserve capacity (75% the daily flow)

Daily flow x .75 = 600 x .75 = 450 gallons

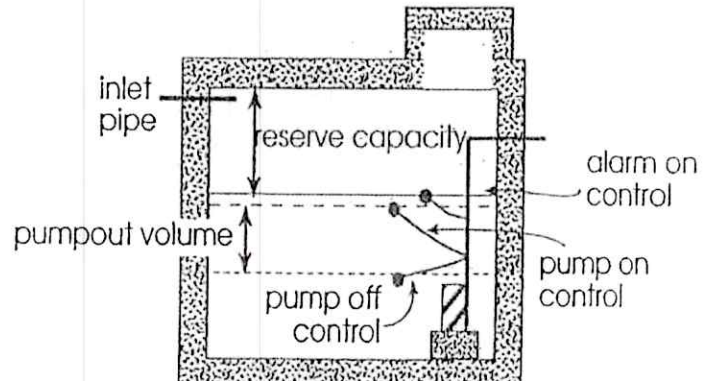
Legal Tank:
500 gallons or
100% the Daily flow
or
Alternating Pumps

A-1: Estimated Sewage Flows in Gallons per Day

number of bedrooms	Class I	Class II	Class III	Class IV
2	300	225	180	60%
3	450	300	218	of the
4	600	375	256	values
5	750	450	294	in the
6	900	525	332	Class I,
7	1050	600	370	II, or III
8	1200	675	408	columns.

E-20: Volume of Liquid in Pipe

Pipe Diameter inches	Gallons per foot
1	0.045
1.25	0.078
1.5	0.11
2	0.17
2.5	0.25
3	0.38
4	0.66



I hereby certify that I have completed this work in accordance with applicable ordinances, rules and laws.

Josh Stevenson (signature)

(signature)

747 (license #)

(license #)

8-19-2020 (date)

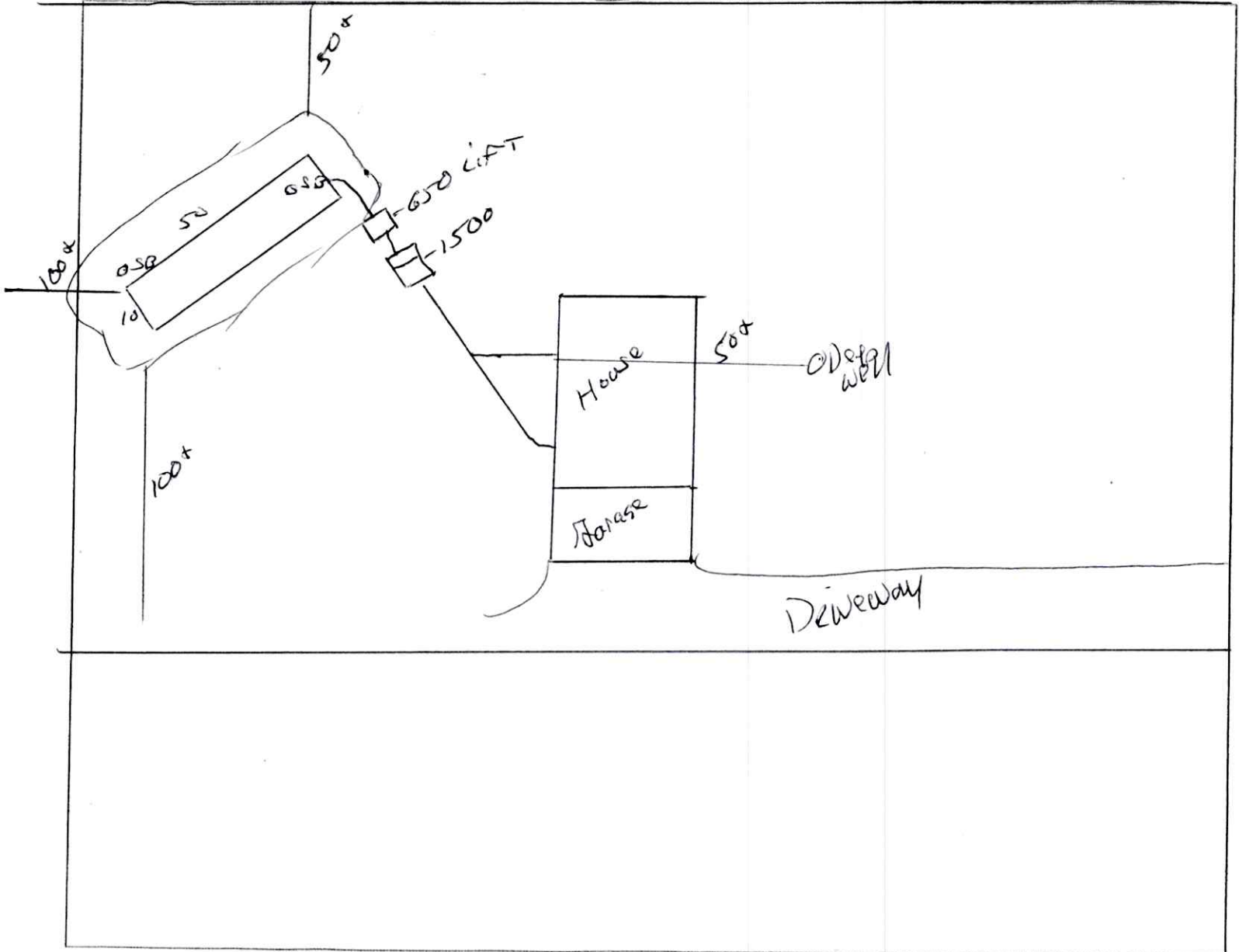
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SKETCH SHEET

CLIENT: Josh Stevenson 11-0-074100

DATE: 8-19-2020

MAP DRAWN TO SCALE WITH A NORTH ARROW



CHECK OFF LIST--HAVE ALL OF THE FOLLOWING BEEN DRAWN ON THE MAP??

SHOW EXISTING OR PROPOSED

- WATER WELLS WITHIN 100 FT OF TREATMENT AREAS
- PRESSURE WATER LINES WITHIN 10 FT OF TREATMENT AREAS
- STRUCTURES
- ALL SOIL TREATMENT AREAS
- HORIZONTAL AND VERTICAL REFERENCE
- POINT OF SOIL BORINGS
- LOT EASEMENTS
- DISTURBED/ COMPACTED AREAS
- SITE PROTECTION--LATHE AND RIBBON EVERY 15 FT
- ACCESS ROUTE FOR TANK MAINTENANCE
- LOT IMPROVEMENTS
- ALL ISTS COMPONENTS
- DIRECTION OF SLOPE
- ALL LOT DIMENSIONS

REQUIRED SETBACKS

- STRUCTURES
- OHWL
- PROPERTY LINES

COMMENTS:

INDICATE ELEVATIONS

- BENCHMARK = 100 Garage floor
- ELEVATION OF SEWER LINE @ HOUSE 96
- ELEVATION @ TANK INLET 95
- ELEVATION @ BOTTOM OF ROCK LAYER 102
- ELEVATION @ BOTTOM OF BORING OR RESTRICTIVE LAYER 99
- ELEVATION OF PUMP 91
- ELEVATION OF DISTRIBUTION DEVICE 103

DESIGNER SIGNATURE *Paul Sch...*
 LICENSE# 747

DATE 8-19-2020

Subsurface Sewage Treatment System Management Plan

Property Owner: Josh Stevenson Phone: 651-335-2118 Date: 08-19-2020
Mailing Address: 347 River Road City: Hudson WI Zip: 54016
Site Address: 43562 Conifer Street City: Aitkin MN Zip: 56431

This management plan will identify the operation and maintenance activities necessary to ensure long-term performance of your septic system. Some of these activities must be performed by you, the homeowner. Other tasks must be performed by a licensed septic service provider.

System Designer: check every _____ months.
Local Government: check every _____ months.
State Requirement: check every 36 months.

**My System needs to be checked
every 36 months.**

Homeowner Management Tasks

Leaks – Check (look, listen) for leaks in toilets and dripping faucets. Repair leaks promptly.

Surfacing sewage – Regularly check for wet or spongy soil around your soil treatment area.

Effluent filter – *Inspect and clean twice a year or more.*

Alarms – Alarm signals when there is a problem. Contact a service provider any time an alarm signals.

Event counter or water meter – Record your water use.

-recommend meter readings be conducted (circle one: DAILY WEEKLY MONTHLY)

Professional Management Tasks

- Check to make sure tank is not leaking
- Check and clean the in-tank effluent filter
- Check the sludge/scum layer levels in all septic tanks
- Recommend if tank should be pumped
- Check inlet and outlet baffles
- Check the drainfield effluent levels in the rock layer
- Check the pump and alarm system functions
- Check wiring for corrosion and function
- Check dissolved oxygen and effluent temperature in tank
- Provide homeowner with list of results and any action to be taken
- Flush and clean laterals if cleanouts exist

"I understand it is my responsibility to properly operate and maintain the sewage treatment system on this property, utilizing the Management Plan. If requirements in the Management Plan are not met, I will promptly notify the permitting authority and take necessary corrective actions. If I have a new system, I agree to adequately protect the reserve area for future use as a soil treatment system."

Property Owner Signature: _____ Date: _____

Designer Signature: Raymond Schrupp Digitally signed by Raymond Schrupp
Date: 2020.08.20 10:55:41 -05'00' Date: _____

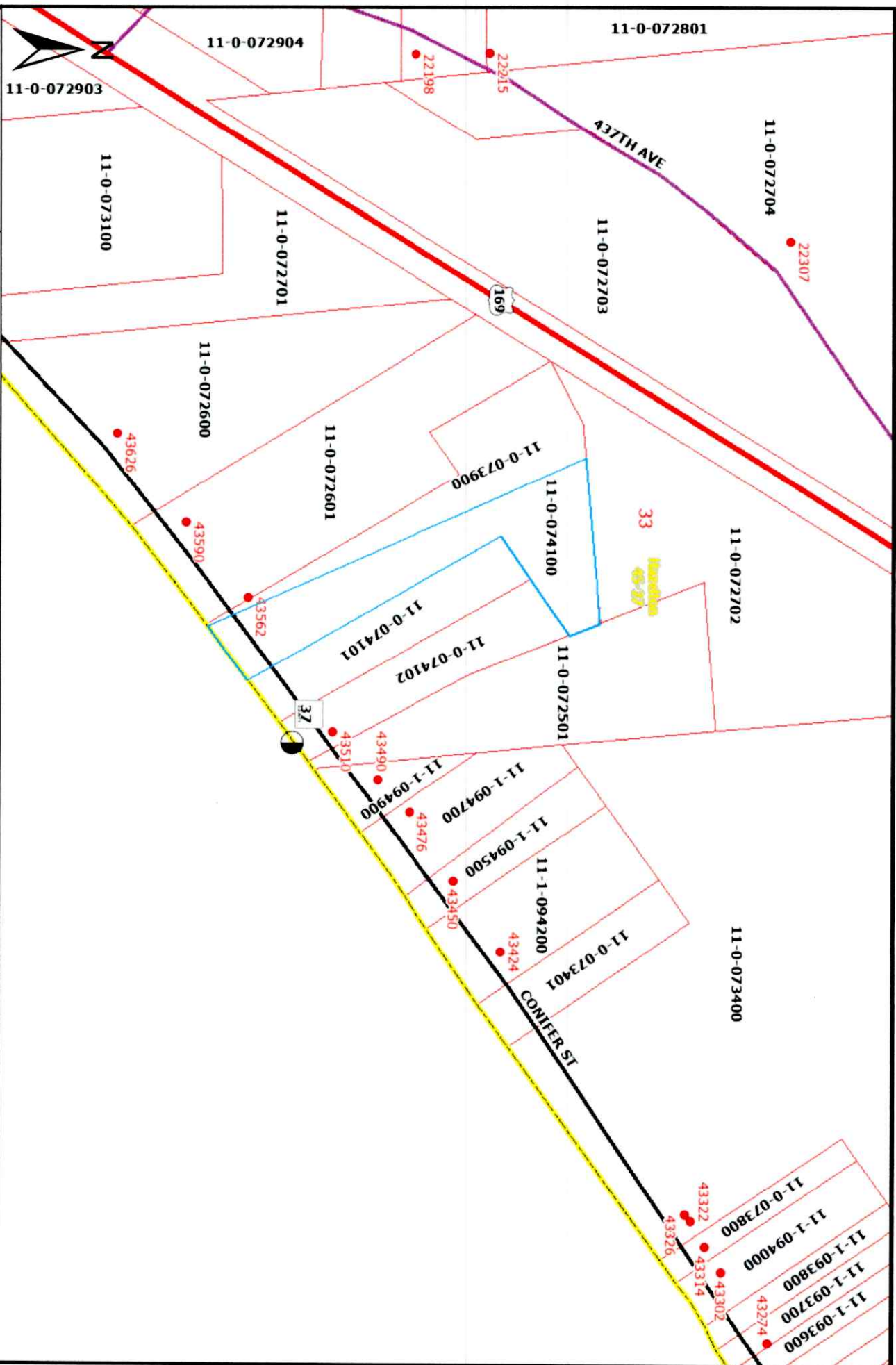
See Reverse Side for Management Log

Maintenance Log

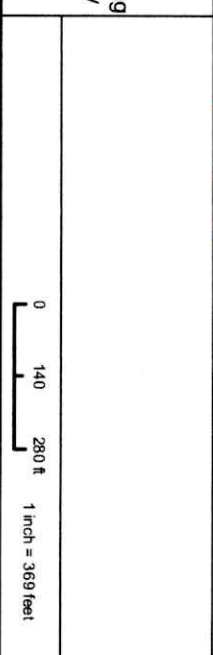
Activity	Date Accomplished									
<i>Check frequently:</i>										
Leaks: check for plumbing leaks										
Soil treatment area check for surfacing										
Lint filter: check, clean if needed										
Effluent screen: if owner-maintained										
Water usage rate (monitor frequency _____)										
<i>Check annually:</i>										
Caps: inspect, replace if needed										
Sludge & Scum/Pump										
Inlet & Outlet baffles										
Drainfield effluent leaks										
Pump, alarm, wiring										
Flush & clean laterals if cleanouts exists										
Other: _____										
Other: _____										

Notes: _____

Mitigation/corrective action plan: _____



These data are provided on an "AS-IS" basis, without warranty of any type, expressed or implied, including but not limited to any warranty as to their performance, merchantability, or fitness for any particular purpose.



0 140 280 ft
1 inch = 369 feet