

## FIELD EVALUATION SHEET

PRELIMINARY EVALUATION DATE 7-12-19, FIELD EVALUATION DATE 7-12-19  
 PROPERTY OWNER: Scott Paulsen PHONE \_\_\_\_\_  
 ADDRESS: 49994 KESTREL AVE. CITY, STATE, ZIP: Aitkin, Mn 56431  
 LEGAL DESCRIPTION:  
 PIN# 10-0-015800 SEC 10 T 49 R 22 TWP NAME HAUGEN  
 FIRE# \_\_\_\_\_ LAKE/RIVER TAMAKA River LAKE CLASS NE OHWL FT.

### DESCRIPTION OF SOIL TREATMENT AREAS

	AREA #1	AREA #2	REFERENCE BM ELEV. FT.
DISTURBED AREAS	YES <u>      </u> NO <u>X</u>	YES <u>      </u> NO <u>X</u>	REFERENCE BM DESCRIPTION _____
COMPACTED AREAS	YES <u>      </u> NO <u>X</u>	YES <u>      </u> NO <u>X</u>	_____
FLOODING	YES <u>      </u> NO <u>X</u>	YES <u>      </u> NO <u>X</u>	_____
RUN ON POTENTIAL	YES <u>      </u> NO <u>X</u>	YES <u>      </u> NO <u>X</u>	_____
SLOPE %	<u>0</u>	<u>0</u>	_____
DIRECTION OF SLOPE	<u>—</u>	<u>—</u>	_____
LANDSCAPE POSITION	<u>Backyard</u>	<u>Backyard</u>	_____
VEGETATION TYPES	<u>GRASS</u>	<u>GRASS + BRUSH</u>	_____

DEPTH TO STANDING WATER OR MOTTLED SOIL: BORING# 1 48", 1A 51", 2 48", 2A 49"

BOTTOM ELEVATION--FIRST TRENCH OR BOTTOM OF ROCK BED: #1 10" FT., #2 10" FT.

SOIL SIZING FACTOR: SITE #1 1.27, SITE #2 1.27

CONSTRUCTION RELATED ISSUES: Mottled Soil at 48-51" Bed must be 10" or less deep

LIC# 1054

SITE EVALUATOR SIGNATURE: Tom Antonsen

SITE EVALUATOR NAME: Tom Antonsen TELEPHONE# 318-851-7757

LUG REVIEW \_\_\_\_\_

DATE 5

Comments: \_\_\_\_\_

SOIL BORING LOGS ON REVERSE SIDE

# SOILS CHARTS FOR BOTH PROPOSED AND ALTERNATE SITES

## 1 (PROPOSED) SOILS DATA

DEPTH (INCHES)	TEXTURE	MUNSELL COLOR
7"	topsoil	10yR 3/2
7-48"	sand	10yR 4/4
48-51"	sand	10yR 3/6

## 2 (PROPOSED) SOILS DATA

DEPTH (INCHES)	TEXTURE	MUNSELL COLOR
6"	topsoil	10yR 3/2
6-48"	sand	10yR 4/4
48+	mottles past 48"	

## 1 (ALTERNATE) SOILS DATA

DEPTH (INCHES)	TEXTURE	MUNSELL COLOR
6"	topsoil	10yR 3/2
6-48"	sand	10yR 4/4
48+	mottles past 48"	

## 2 (ALTERNATE) SOILS DATA

DEPTH (INCHES)	TEXTURE	MUNSELL COLOR
6"	topsoil	10yR 3/2
6-49"	sand	10yR 4/4
49+	mottles past 49"	

ADDITIONAL SOIL BORINGS MAY BE REQUIRED

# TRENCH AND BED WORKSHEET

## 1. AVERAGE DESIGN FLOW

A. Estimated 300 gpd (see figure A-1)  
or measured x 1.5 (safety factor) = gpd

B. Septic tank capacity 1000 gal (see figure C-1) 1500 Sather

*Compartment tank  
(500' Lift)*

## 2. SOILS (Site evaluation data)

- C. Depth to restricting layer = 4 ft
- D. Max depth of system Item 2C - 3 ft = 4 ft - 3 ft = 1 ft
- E. Texture Sand Percolation rate \_\_\_\_\_ MPI
- F. Soil Sizing Factor (SSF) 1.27 sqft/gpd (see figure D-15)
- G. % Land Slope 0 %

## 3. TRENCH or BED BOTTOM AREA

H. For trenches with 6 inches of rock below the pipe:

$$A \times F = \text{gpd} \times \text{sqft/gpd} = \text{sqft}$$

I. For trenches with 12 inches of rock below the pipe:

$$A \times F \times 0.8 = \text{gpd} \times \text{sqft/gpd} \times 0.8 = \text{sqft}$$

J. For trenches with 18 inches of rock below the pipe:

$$A \times F \times 0.66 = \text{gpd} \times \text{sqft/gpd} \times 0.66 = \text{sqft}$$

K. For trenches with 24 inches of rock below the pipe:

$$A \times F \times 0.6 = \text{gpd} \times \text{sqft/gpd} \times 0.6 = \text{sqft}$$

L. For gravity beds with 6 or 12 inches of rock below the pipe;

$$1.5 \times A \times F = 1.5 \times \text{gpd} \times \text{sqft/gpd} = \text{sqft}$$

For pressure beds with 6 or 12 inches of rock below the pipe;

$$A \times F = \text{gpd} \times \text{sqft/gpd} = \text{sqft}$$

10 X 38

## 4. DISTRIBUTION (Check all that apply)

- Bed (< 6% slope)  Drop boxes (any slope)
- Rock
- Trenches  Distribution box (< 3%)
- Chamber
- Pressure  Gravity
- Gravelless

## 5. SYSTEM WIDTH, LENGTH and VOLUME

M. Select trench width = ft

N. If using rock, divide bottom area by width:  $(H, I, J, K \text{ or } L) \div M = \text{sqft} \div \text{ft} = \text{lineal feet}$

Rock depth below distribution pipe plus 0.5 foot times bottom area:

Rock depth in feet + 0.5 feet x Area (H,I,J,K, or L)

$$(\text{ft} + 0.5 \text{ ft}) \times \text{sqft} = \text{cuft}$$

Volume in cubic yards = cuft ÷ 27

$$\text{cuft} \div 27 = \text{cuyds}$$

Weight of rock in tons = cubic yds x 1.4

$$\text{cuyds} \times 1.4 = \text{tons}$$

O. If using 10" Gravelless Pipe, Flow (A) x Gravelless SSF(see figure D-9)

$$\text{gpd} \times \text{lineal feet/gpd} = \text{lineal feet}$$

P. If using Chambers, H,I,J, or K(based on height of chamber slats) ÷ width of chamber in feet(M)

$$\text{sqft} \div \text{ft} = \text{lineal ft}$$

6. LAWN AREA 10 X 38 Pressure Bed = 380 Sq ft

Q. Select trench spacing, center to center = feet

R. Multiply trench spacing by lineal feet R x Q = sqft of lawn area

$$\text{ft} \times \text{ft} = \text{sqft}$$

7. Include a drawing with scale (one inch = ft). Show pertinent boundaries, right of way, easements, location of house, garage, driveway, all other improvements, existing or proposed soil treatment system, well and dimensions of all elevations, setbacks and separation distances.

A-1: Estimated Sewage Flows in Gallons per Day

number of bedrooms	Class I	Class II	Class III	Class IV
2	300	225	180	60%
3	450	300	218	of the values
4	600	375	256	in the Class I,
5	750	450	294	II, or III
6	900	525	332	columns.
7	1050	600	370	
8	1200	675	408	

C-1: Septic Tank Capacities (in gallons)

Number of Bedrooms	Minimum Liquid Capacity	Liquid capacity with garbage disposal	Liquid capacity with disposal & lift inside
2 or less	750	1125	1500
3 or 4	1000	1500	2000
5 or 6	1500	2250	3000
7, 8 or 9	2000	3000	4000

D-15: Soil Characteristics and Soil Sizing Factors (SSF) (> 3' separation)

Percolation Rate (minutes/inch (in/h))	Soil Texture	Soil Sizing Factor (square feet/gallon per day (sqft/gpd))
faster than 0.1* 0.1 to 5** 0.1 to 5*** 6 to 15 16 to 30 31 to 45	Coarse sand Medium sand Loamy sand Fine sand Sandy loam Loam Silt loam Silt	0.83 1.67 1.27 1.67 2.00 2.20
46 to 60	Clay loam Sandy clay Silty clay Clay Sandy clay Silty clay	4.20
over 61 to 120***		
slower than 120****		

\*Use systems for rapidly permeable soils:  
pressure distribution or serial distribution with no trench >25% of the total system.

\*\*Soil having 50% or more fine sand plus very fine sand.

\*\*\*A mound must be used.

\*\*\*\*An other or performance system must be used

D-9: Soil Characteristics and Soil sizing factors (SSF) for Gravelless Pipe

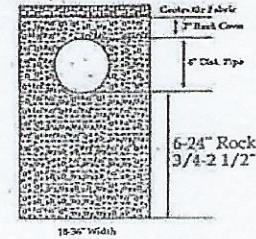
percolation rate (minutes/inch)	soil texture	lineal feet/gallon/day
faster than 0.1 * 0.1 to 5 0.1 to 5*** 6 to 15 16 to 30 31 to 45	Coarse Sand Medium Sand Loamy Sand Fine Sand Sandy Loam Loam Silt Loam Silt	0.28 0.6 0.42 0.56 0.67 0.74
46 to 60	Clay Loam (CL) Sandy CL Silty CL Clay Sandy Clay Silty Clay	---
slower than 60***		

\*Soil too coarse for sewage treatment.

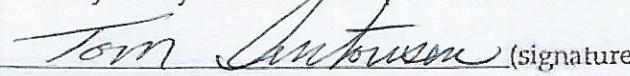
Use systems for rapidly permeable soils.

\*\*Soil having 50% or more fine sand plus very fine sand.

\*\*\*Soil with too high a percentage of clay for installation of a standard inground system.



I hereby certify that I have completed this work in accordance with applicable ordinances, rules and laws.

 (signature) 1054 (license #) 7-19-20 (date)

# PRESSURE DISTRIBUTION SYSTEM

1. Select number of perforated laterals 3

2. Select perforation spacing = 2.5 ft

3. Since perforations should not be placed closer than 1 foot to the edge of the rock layer (see diagram), subtract 2 feet from the rock layer length.

$$\frac{38}{\text{Rock layer length}} - 2 \text{ ft} = \underline{36} \text{ ft}$$

4. Determine the number of spaces between perforations. Divide the length (3) by perforation spacing (2) and round down to nearest whole number.

$$\text{Perforation spacing} = \frac{\cancel{36}}{\cancel{2.5}} \text{ ft} \div 2.5 \text{ ft} = \underline{14} \text{ spaces}$$

5. Number of perforations is equal to one plus the number of perforation spaces(4). Check figure E-4 to assure the number of perforations per lateral guarantees <10% discharge variation.

$$\underline{14} \text{ spaces} + 1 = \underline{15} \text{ perforations/lateral}$$

6. A. Total number of perforations = perforations per lateral (5) times number of laterals (1)

$$\underline{15} \text{ perfs/lat} \times \underline{3} \text{ lat} = \underline{45} \text{ perforations}$$

B. Calculate the square footage per perforation.

Should be 6-10 sqft/perf. Does not apply to at-grades.

Rock bed area = rock width (ft) x rock length (ft)

$$10 \cancel{\text{ft}} \times \underline{38} \text{ ft} = \underline{380} \text{ sqft}$$

Square foot per perforation = Rock bed area ÷ number of perfs (6)

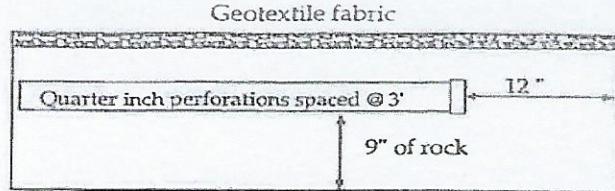
$$\underline{380} \text{ sqft} \div \underline{45} \text{ perfs} = \underline{8.4} \text{ sqft/perf}$$

7. Determine required flow rate by multiplying the total number of perforations (6A) by flow per perforation (see figure E-6)

$$\underline{45} \text{ perfs} \times \underline{.74} \text{ gpm/perf} = \underline{33} \text{ gpm}$$

8. If laterals are connected to header pipe as shown on upper example, to select minimum required lateral diameter; enter figure E-4 with perforation spacing (2) and number of perforations per lateral (5) Select minimum diameter for perforated lateral = 1 1/2 inches.

9. If perforated lateral system is attached to manifold pipe near the center, lower diagram, perforated lateral length (3) and number of perforations per lateral (5) will be approximately one half of that in step 8. Using these values, select minimum diameter for perforated lateral = \_\_\_\_\_ inches.



Perf Sizing 3/16" - 1/4"  
Perf Spacing 1.5'-5'

E-4: Maximum allowable number of 1/4-inch perforations per lateral to guarantee <10% discharge variation				
perforation spacing (feet)	1 inch	1.25 inch	1.5 inch	2.0 inch
2.5	8	14	18	28
3.0	8	13	17	26
3.3	7	12	16	25
4.0	7	11	15	23
5.0	6	10	14	22

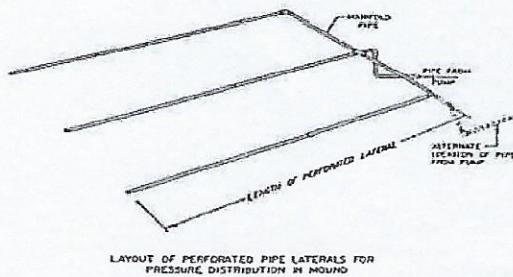
E-6: Perforation Discharge in gpm

head (feet)	perforation diameter (inches)			
	1/8	3/16	7/32	1/4
1.0 <sup>a</sup>	0.18	0.42	0.56	0.74
2.0 <sup>b</sup>	0.26	0.59	0.80	1.04
5.0	0.41	0.94	1.26	1.65

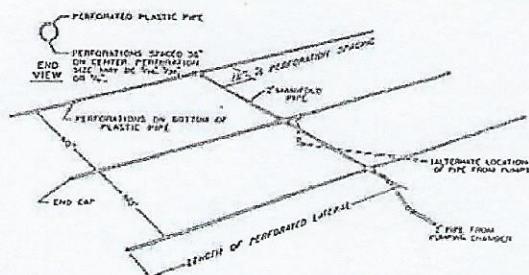
<sup>a</sup> Use 1.0 foot for single-family homes.

<sup>b</sup> Use 2.0 feet for anything else.

MANIFOLD LOCATED AT END OF PRESSURE DISTRIBUTION SYSTEM



LAYOUT OF PERFORATED PIPE LATERALS FOR PRESSURE DISTRIBUTION IN MOUND



I hereby certify that I have completed this work in accordance with applicable ordinances, rules and laws.

Tom Antonson

(signature)

1054

(license #)

7-19-20

(date)

# PUMP SELECTION PROCEDURE

## 1. Determine pump capacity:

### A. Gravity distribution

1. Minimum required discharge is 10 gpm
2. Maximum suggested discharge is 45 gpm. For other establishments at least 10% greater than the water supply rate, but no faster than the rate at which effluent will flow out of the distribution device.

### B. Pressure distribution

*See pressure distribution work sheet*

From A or B Selected pump capacity: 33 gpm

## 2. Determine pump head requirements:

### A. Elevation difference between pump and point of discharge?

9 feet

### B. Special head requirement? (See Figure at right - Special Head Requirements)

5 feet

### C. Calculate Friction loss

1. Select pipe diameter 2 in

2. Enter Figure E-9 with gpm (1A or B) and pipe diameter (C1).

Read friction loss in feet per 100 feet from Figure E-9

$$\text{Friction Loss} = \underline{2.06} \text{ ft/100ft of pipe}$$

3. Determine total pipe length from pump discharge to soil treatment discharge point. Estimate by adding 25 percent to pipe length for fitting loss. Total pipe length times 1.25 = equivalent pipe length

$$\underline{85} \text{ feet} \times 1.25 = \underline{106.25} \text{ feet}$$

4. Calculate total friction loss by multiplying friction loss (C2)

in ft/100 ft by the equivalent pipe length (C3) and divide by 100.  

$$= \underline{2.06} \text{ ft/100ft} \times \underline{106.25} \div 100 = \underline{2} \text{ ft}$$

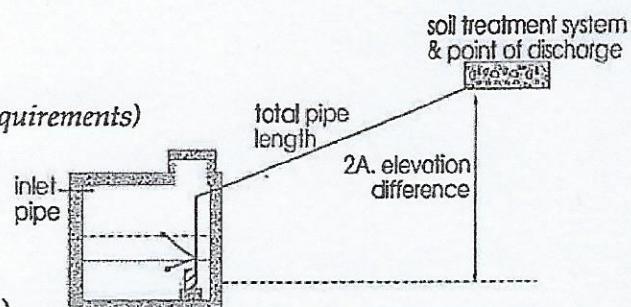
D. Total head required is the sum of elevation difference (A), special head requirements (B), and total friction loss (C4)

$$\underline{9} \text{ ft} + \underline{5} \text{ ft} + \underline{2} \text{ ft} =$$

Total head: 16 feet

### 3. Pump selection

A pump must be selected to deliver at least 33 gpm  
 (1A or B) with at least 16 feet of total head (2D)



Special Head Requirements	
Gravity Distribution	0 ft
Pressure Distribution	5 ft

flow rate gpm	E-9: Friction Loss in Plastic Pipe Per 100 feet		
	nominal pipe diameter 1.5"	2"	3"
20	2.47	0.73	0.11
25	3.73	1.11	0.16
30	5.23	1.55	0.23
35	6.96	2.06	0.30
40	8.91	2.64	0.39
45	11.07	3.28	0.48
50	13.46	3.99	0.58
55	16.00	4.76	0.70
60	18.70	5.60	0.82
65	21.54	6.48	0.95
70	24.52	7.44	1.09

I hereby certify that I have completed this work in accordance with applicable ordinances, rules and laws.

Tom Ambrose (signature) 1054 (license #) 7-19-20 (date)

# DOSING CHAMBER SIZING

**1. Determine area**

A. Rectangle area =  $L \times W$

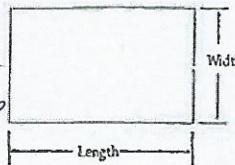
$$\underline{\quad} \times \underline{\quad} = \underline{\quad} \text{ square feet}$$

B. Circle area =  $\pi (3.14) \times \text{radius in feet} \times \text{radius in feet}$

$$3.14 \times \underline{\quad} \text{ ft} \times \underline{\quad} \text{ ft} = \underline{\quad} \text{ sqft}$$

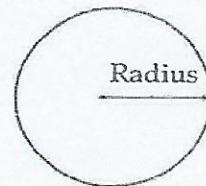
C. Get area from manufacturer  $\underline{\quad}$  sqft

USING 1500 Compartment SATER  
TANK 500 cu ft - 42" deep



~~12~~ 9 gpi

12



**2. Calculate gallons per inch**

There are 7.5 gallons per cubic foot of volume, therefore multiply the area (1A, B or C) times the conversion factor and divide by 12 inches per foot to calculate gallon per inch.

$$\text{Area} \times 7.5 \div 12 = \underline{\quad} \text{ sqft} \times 7.5 \div 12 \text{ in/ft} = \underline{\quad} \text{ gallon per inch}$$

12

**3. Calculate total tank volume**

A. Depth from bottom of inlet pipe to tank bottom 42 in

B. Total tank volume = depth from bottom of inlet pipe to tank bottom (3A)  $\times$  gal/in (2)  
 $= \underline{42} \text{ in} \times \underline{12} \text{ gal/in} = \underline{504} \text{ gal}$

**4. Calculate gallons to cover pump (with 2-3 inches of water covering pump)**

(Pump and block height (inch) + 2 inch)  $\times$  gallon/inch

$$(\underline{12} \text{ in} + 2 \text{ in}) \times \underline{12} \text{ gal/in} = \underline{168} \text{ gallon}$$

**5. Calculate total pumpout volume**

A. Select pump size for 4-5 doses per day. Gallon per dose = gpd (see figure A-1)  
 $/ \text{ doses per day} = \underline{80} \text{ gpd} \div \underline{4} \text{ doses/day} = \underline{20} \text{ gallons}$

B. Calculate drainback

1. Determine total pipe length, 85 feet

2. Determine liquid volume of pipe, .17 gal per ft (see figure E-20)

3. Drainback quantity = 85 ft (5B1)  $\times$  .17 gal per ft (5B2) = 14.5 gal

C. Total pump out volume = dose volume (5A) + drainback (5B3)

$$\underline{75} \text{ gal} + \underline{14.5} \text{ gal} = \underline{89.5} \text{ Total gallon}$$

**6. Float separation distance (using total pumpout volume)**

Total pumpout volume (5C)  $\div$  gal/inch (2)

$$\underline{89.5} \text{ gal} \div \underline{12} \text{ gal/in} = \underline{7.5} \text{ inch}$$

**7. Calculate volume for alarm (typically 2 to 3 inches)**

Alarm depth (inch)  $\times$  gallon/inch (2) = 3 in  $\times$  12 gal/in = 36 gal

**8. Calculate total gallon = gallons over pump (4) + gallons pumpout (5C) + gallons alarm (7)**

$$\underline{168} \text{ gal} + \underline{89.5} \text{ gal} + \underline{36} \text{ gal} = \underline{293.5} \text{ gallons}$$

**9. Total Tank Depth = total gallon (8)  $\div$  gallon/inch (2)**

$$\underline{293.5} \text{ gal} \div \underline{12} \text{ gal/in} = \underline{24.5} \text{ in}$$

**Recommended:**

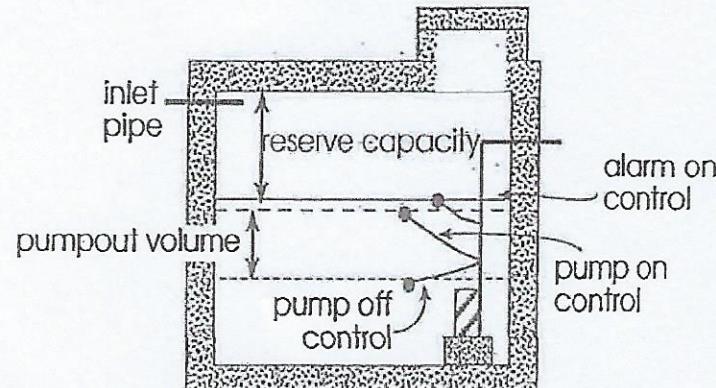
Calculate reserve capacity (75% the daily flow)

$$\text{Daily flow} \times .75 = \underline{300} \times .75 = \underline{225} \text{ gallons}$$

**Legal Tank:**  
500 gallons or  
100% the Daily flow  
or  
**Alternating Pumps**

A-1: Estimated Sewage Flows in Gallons per Day				
number of bedrooms	Class I	Class II	Class III	Class IV
2	300	225	180	60%
3	450	300	218	of the values
4	600	375	256	in the
5	750	450	294	columns.
6	900	525	332	Class I,
7	1050	600	370	II, or III
8	1200	675	408	

E-20: Volume of Liquid in Pipe	
Pipe Diameter inches	Gallons per foot
1	0.045
1.25	0.078
1.5	0.11
2	0.17
2.5	0.25
3	0.38
4	0.66



I hereby certify that I have completed this work in accordance with applicable ordinances, rules and laws.

Tom Antunes (signature)

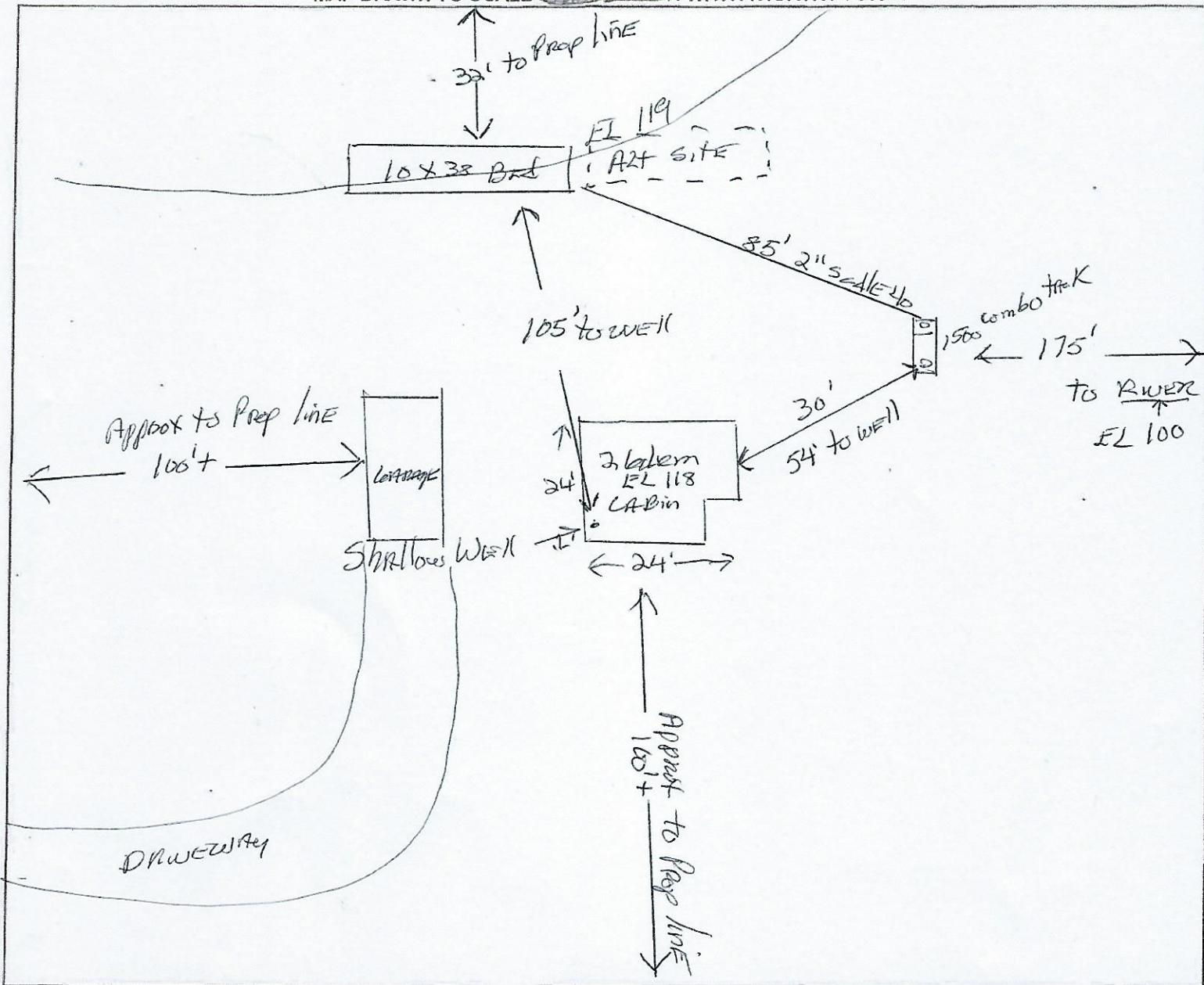
1054 (license #) 7-19-20 (date)

CLIENT: Scott Paulsen

SKETCH SHEET

DATE: 7-19-20

MAP DRAWN TO SCALE WITH A NORTH ARROW



CHECK OFF LIST--HAVE ALL OF THE FOLLOWING BEEN DRAWN ON THE MAP??

SHOW EXISTING OR PROPOSED

- WATER WELLS WITHIN 100 FT OF TREATMENT AREAS
  - PRESSURE WATER LINES WITHIN 10 FT OF TREATMENT AREAS
  - STRUCTURES
  - ALL SOIL TREATMENT AREAS
  - HORIZONTAL AND VERTICAL REFERENCE
  - POINT OF SOIL BORINGS
  - LOT EASEMENTS
  - DISTURBED/COMPACTED AREAS
  - SITE PROTECTION-LATHE AND RIBBON EVERY 15 FT
  - ACCESS ROUTE FOR TANK MAINTENANCE
- REQUIRED SETBACKS
- STRUCTURES
  - OHWL

COMMENTS:

DESIGNER SIGNATURE

LICENSE# 1054

*Tom Lutzen*

INDICATE ELEVATIONS

BENCHMARK RIVER = 100

ELEVATION OF SEWER LINE @ HOUSE 116

ELEVATION @ TANK INLET 115

ELEVATION @ BOTTOM OF ROCK LAYER 118

ELEVATION @ BOTTOM OF BORING OR RESTRICTIVE LAYER 115

ELEVATION OF PUMP 110

ELEVATION OF DISTRIBUTION DEVICE 118.0

DATE 7-19-20