FIELD EVALUATION SHEET

PRELIMINARY EVALUATION DATE 6-7-2020 FIELD EVALUATION DATE 6-7-20
PROPERTY OWNER: Tim Patterson PHONE 952-826-9480
ADDRESS: 29652 335th Lane CITY,STATE,ZIP: Aitkin, Mn 56431
LEGAL DESCRIPTION:
PIN# 09-0-002310 SEC___ T___ R___ TWP NAME
FIRE#___ LAKE/RIVER Damn Lake LAKE CLASS RD OHWL___FT.

DESCRIPTION OF SOIL TREATMENT AREAS

	AREA #1	AREA #2
DISTURBED AREAS	YES___NO X	YES___NO___
COMPACTED AREAS	YES___NO X	YES___NO___
FLOODING	YES___NO X	YES___NO___
RUN ON POTENTIAL	YES___NO X	YES___NO___
SLOPE %	Flat 0%	
DIRECTION OF SLOPE	—	
LANDSCAPE POSITION	Backyard	
VEGETATION TYPES	grass - existing mound	

REFERENCE BM ELEV.___FT.
REFERENCE BM DESCRIPTION___

DEPTH TO STANDING WATER OR MOTTLED SOIL: BORING# 1 6", 1A 6", 2___,2A___

BOTTOM ELEVATION--FIRST TRENCH OR BOTTOM OF ROCK BED: #1 105 FT., #2 105 FT.

SOIL SIZING FACTOR: SITE # 1 1.27, SITE #2 —

CONSTRUCTION RELATED ISSUES: Mound Failed Compliance - needs a "other" system

LIC# 1054 SITE EVALUATOR SIGNATURE: Tom Antonsen
SITE EVALUATOR NAME: Tom Antonsen TELEPHONE# 218-851-7757
LUG REVIEW___ DATE 6-14-2020

Comments:

SOIL BORING LOGS ON REVERSE SIDE

Form des 2/20/98

SOILS CHARTS FOR BOTH PROPOSED AND ALTERNATE SITES

1 (PROPOSED) SOILS DATA

DEPTH (INCHES)	TEXTURE	MUNSELL COLOR
5" 10YR 3/2 topsoil 5-12" Sandy loam 10YR 5/3 Standing water @ 9"		

2 (PROPOSED) SOILS DATA

DEPTH (INCHES)	TEXTURE	MUNSELL COLOR
6" Topsoil 10YR 3/2 6-12" Sandy loam 10YR 5/3 Standing water at 8-9"		

1 (ALTERNATE) SOILS DATA

DEPTH (INCHES)	TEXTURE	MUNSELL COLOR

2 (ALTERNATE) SOILS DATA

DEPTH (INCHES)	TEXTURE	MUNSELL COLOR

ADDITIONAL SOIL BORINGS MAY BE REQUIRED

MOUND DESIGN WORK SHEET (For Flows up to 1200 gpd)

A. Average Design FLOW

Estimated 450 gpd *(see figure A-1)*
or measured _____ x 1.5 (safety *factor*) =_____ gpd

A-1: Estimated Sewage Flows in Gallons per Day				
number of bedrooms	Class I	Class II	Class III	Class IV
2	300	225	180	60%
3	450	300	218	of the
4	600	375	256	values
5	750	450	294	in the
6	900	525	332	Class I,
7	1050	600	370	II, or III
8	1200	675	408	columns.

B. SEPTIC TANK Capacity

Existing 1350 Compartment tank
gallons *(see figure C-1)*

C-1: Septic Tank Capacities (in gallons)			
Number of Bedrooms	Minimum Liquid Capacity	Liquid capacity with garbage disposal	Liquid capacity with disposal & lift inside
2 or less	750	1125	1500
3 or 4	1000	1500	2000
5 or 6	1500	2250	3000
7, 8 or 9	2000	3000	4000

C. SOILS *(refer to site evaluation)*

1. Depth to restricting layer = 6" feet
2. Depth of percolation tests = — feet
3. Texture Sandy loam
   Percolation rate 1.27 mpi
4. Soil loading rate 1.50 gpd/sqft *(see figure D-33)*
5. Percent land slope 0 %

D. ROCK LAYER DIMENSIONS

1. Multiply average design flow (A) by 0.83 to obtain required rock layer area.
   450 gpd x 0.83 sqft/gpd = 373.5 sqft
2. Determine rock layer width = 0.83 sqft/gpd x linear Loading Rate (LLR)
   0.83 sqft/gpd x _____ gpd/sqft = 10 ft
3. Length of rock layer = area ÷ width =
   380 sqft (D1) ÷ 10 ft (D2) = 38 ft

Mound LLR	
< 120 MPI	≤12
≥ 120 MPI	≤6

E. ROCK VOLUME

10 x 38 Bed

1. Multiply rock area (D1) by rock depth of 1 ft to get cubic feet of rock
   380 sqft x 1 ft = 380 cuft
2. Divide cuft by 27 cuft/cuyd to get cubic yards
   380 cuft ÷ 27 cuyd/cuft = 14.07 cuyd
3. Multiply cubic yards by 1.4 to get weight of rock in tons
   14.07 cuyd x 1.4 ton/cuyd = 19.7 tons

F. SEWAGE ABSORPTION WIDTH

Absorption width equals absorption ratio (See Figure D-33) times rock layer width (D2)

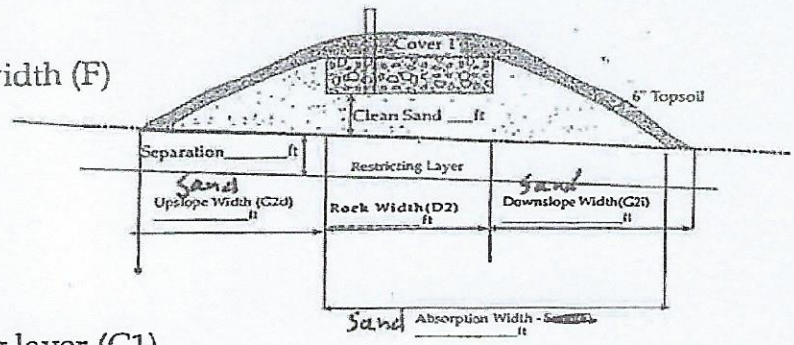
1.50 x 10 ft = 15 ft

D-33: Absorption Width Sizing Table			
Percolation Rate in Minutes per Inch (MPI)	Soil Texture	Loading Rate Gallons per day per square foot	Absorption Ratio
Faster than 5	Coarse Sand Medium Sand Loamy Sand Fine Sand	1.20	1.00
6 to 15	Sandy Loam	0.79	1.50
16 to 30	Loam	0.60	2.00
31 to 45	Silt Loam Silt	0.50	2.40
46 to 60	Sandy Clay Loam Silty Clay Loam Clay Loam	0.45	2.67
61 to 120	Silty Clay Sandy Clay Clay	0.24	5.00
Slower than 120*			

*System designed for these soils must be other or performance

G. MOUND SLOPE WIDTH & LENGTH (landslope greater than 1%)

Landslope > 1% slope



1. Downslope absorption width = absorption width (F) minus rock layer width (D2)

15 ft - 10 ft = 5 ft

2. Calculate mound size

UPSLOPE

a. Depth of clean sand fill at upslope edge of rock layer = 3 ft minus the distance to restricting layer (C1)

3 ft - 0 ft = 3 ft

b. Mound height at the upslope edge of rock layer = depth of clean sand for separation (G2a) at upslope edge plus depth of rock layer (1 ft) plus depth of cover (1 ft)

3 ft + 1ft + 1ft = 5 ft

c. Upslope berm multiplier based on land slope

3.0 *(see figure D-34)*

d. Upslope width = berm multiplier (G2c) x upslope mound height (G2b):

3 x 5 ft = 15 ft

DOWNSLOPE

e. Drop in elevation = rock layer width (D2) x percent landslope (C5) ÷ 100

0 ft x 0 % ÷ 100 = 0 ft

f. Downslope mound height = depth of clean sand for slope difference (G2e) at downslope rock edge plus the mound height at the upslope edge of rock layer (G2b)

0 ft + 5 ft = 5 ft

g. Downslope berm multiplier based on percent land slc

3 *(see figure D-34)*

h. Downslope width = downslope multiplier (G2g) times downslope mound height (G2f)

3 x 5 ft = 15 ft

i. Select the greater of G1 and G2h as the downslope width: 15 ft

j. Total mound width is the sum of upslope width (G2d) width plus rock layer width (D2) plus downslope width (G2i)

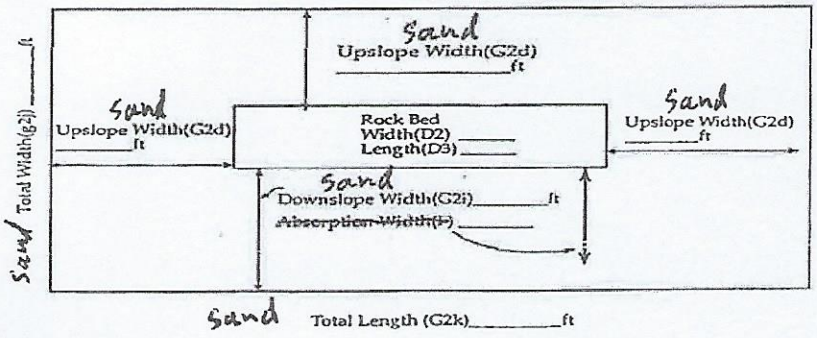
15 ft + 10 ft + 15 ft = 40 ft

k. Total mound length is the sum of upslope width (G2d) plus rock layer length (D3) plus upslope width (G2d)

15 ft + 38 ft + 15 ft = 68 feet

D-34: SLOPE MULTIPLIER TABLE

Land Slope, in %	UPSLOPE multipliers for various slope ratios						DOWNSLOPE multipliers for various slope ratios				
	3:1	4:1	5:1	6:1	7:1	8:1	3:1	4:1	5:1	6:1	7:1
0	3.0	4.0	5.0	6.0	7.0	8.0	3.0	4.0	5.0	6.0	7.0
1	2.91	3.85	4.76	5.66	6.54	7.41	3.09	4.17	5.26	6.38	7.53
2	2.83	3.70	4.54	5.36	6.14	6.90	3.19	4.35	5.56	6.82	8.14
3	2.75	3.57	4.35	5.08	5.79	6.45	3.30	4.54	5.88	7.32	8.86
4	2.68	3.45	4.17	4.84	5.46	6.06	3.41	4.76	6.25	7.89	9.72
5	2.61	3.33	4.00	4.62	5.19	5.71	3.53	5.00	6.67	8.57	10.77
6	2.54	3.23	3.85	4.41	4.93	5.41	3.66	5.26	7.14	9.38	12.07
7	2.48	3.12	3.70	4.23	4.70	5.13	3.80	5.56	7.69	10.34	13.73
8	2.42	3.03	3.57	4.05	4.49	4.88	3.95	5.88	8.33	11.54	15.91
9	2.36	2.94	3.45	3.90	4.30	4.65	4.11	6.25	9.09	13.04	18.92
10	2.31	2.86	3.33	3.75	4.12	4.44	4.29	6.67	10.00	15.00	23.33
11	2.26	2.78	3.23	3.61	3.95	4.26	4.48	7.14	11.11	17.65	30.43
12	2.21	2.70	3.12	3.49	3.80	4.08	4.69	7.69	12.50	21.43	43.75

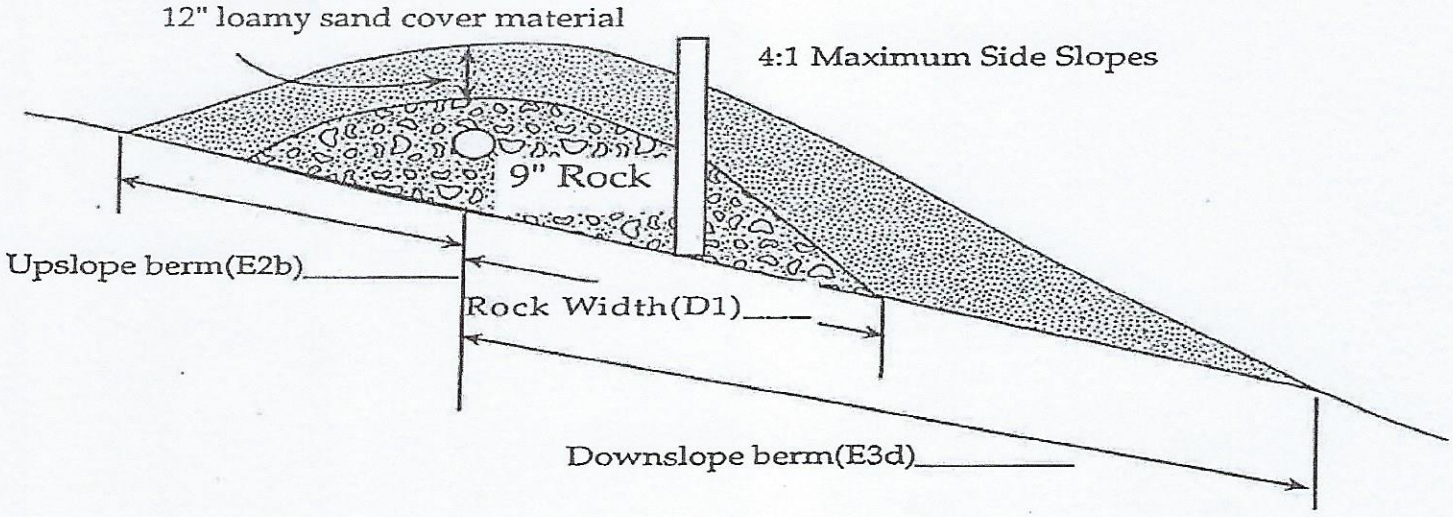


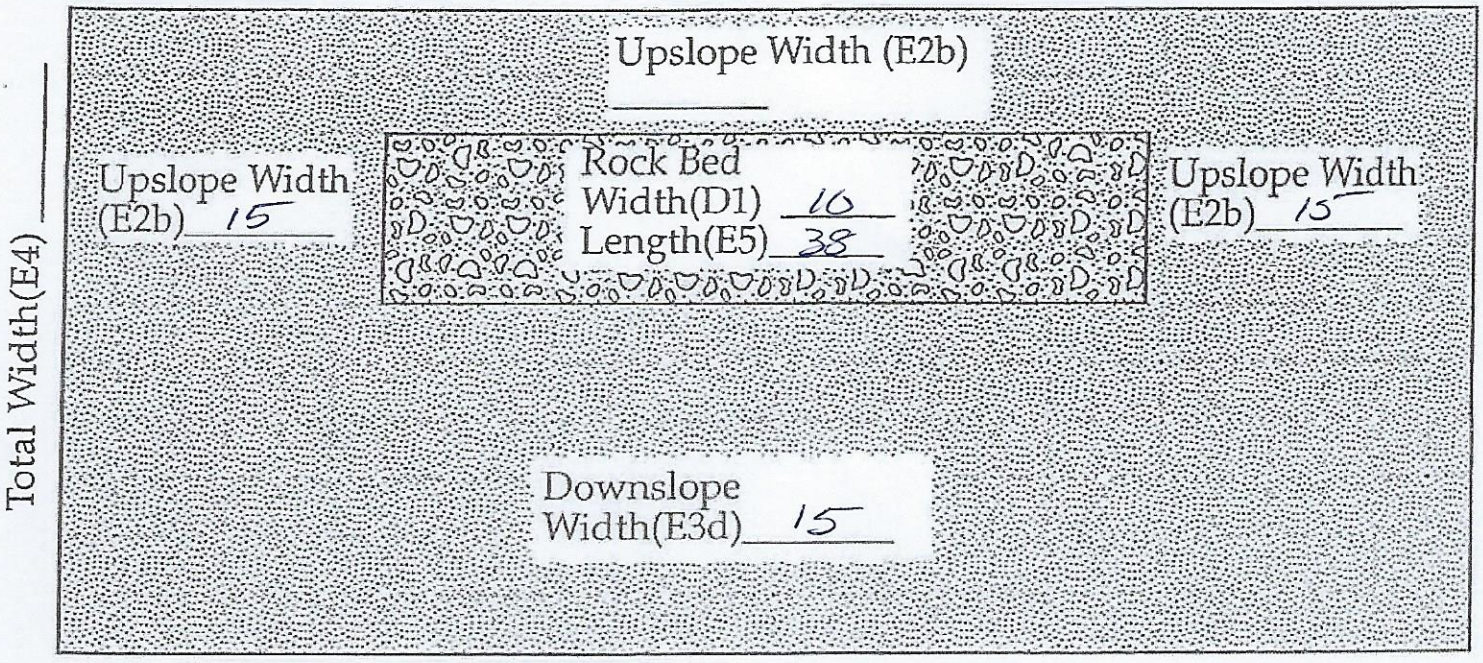
Final Dimensions:

40 x 68

I hereby certify that I have completed this work in accordance with applicable ordinances, rules and laws.

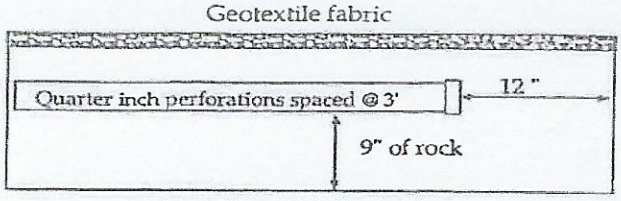
Tom Centofanti (signature) 1054 (license #) 6-14-20 (date)





Mound is 40 x 68 - 3.0 slope

PRESSURE DISTRIBUTION SYSTEM



Perf Sizing 3/16" - 1/4"
Perf Spacing 1.5'- 5'

1. Select number of perforated laterals 3

2. Select perforation spacing = 2.5 ft

3. Since perforations should not be placed closer than 1 foot to the edge of the rock layer (see diagram), subtract 2 feet from the rock layer length.

   38 (Rock layer length) - 2 ft = 36 ft

4. Determine the number of spaces between perforations. Divide the length (3) by perforation spacing (2) and round down to nearest whole number.

   Perforation spacing = 38 ft ÷ 2.5 ft = 14 spaces

5. Number of perforations is equal to one plus the number of perforation spaces(4). *Check figure E-4 to assure the number of perforations per lateral guarantees <10% discharge variation.*

   14 spaces + 1 = 15 perforations/lateral

6. A. Total number of perforations = perforations per lateral (5) times number of laterals (1)

   15 perfs/lat x 3 lat = 45 perforations

   B. Calculate the square footage per perforation.
   Should be 6-10 sqft/perf. *Does not apply to at-grades.*
   Rock bed area = rock width (ft) x rock length (ft)
   10 ft x 38 ft = 380 sqft
   Square foot per perforation = Rock bed area ÷ number of perfs (6)
   380 sqft ÷ 45 perfs = 8.4 sqft/perf

7. Determine required flow rate by multiplying the total number of perforations (6A) by flow per perforation *(see figure E-6)*

   45 perfs x .74 gpm/perfs = 33.3 gpm

8. If laterals are connected to header pipe as shown on upper example, to select minimum required lateral diameter; enter figure E-4 with perforation spacing (2) and number of perforations per lateral (5) Select minimum diameter for perforated lateral = 1.5 inches.

9. If perforated lateral system is attached to manifold pipe near the center, lower diagram, perforated lateral length (3) and number of perforations per lateral (5) will be approximately one half of that in step 8. Using these values, select minimum diameter for perforated lateral = _____ inches.

E-4: Maximum allowable number of 1/4-inch perforations per lateral to guarantee <10% discharge variation

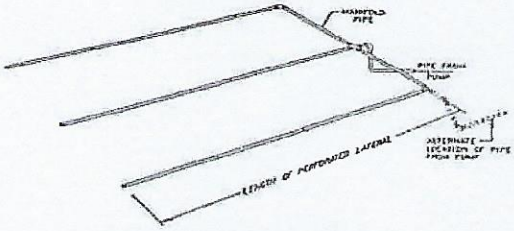
perforation spacing (feet)	1 inch	1.25 inch	1.5 inch	2.0 inch
2.5	8	14	18	28
3.0	8	13	17	26
3.3	7	12	16	25
4.0	7	11	15	23
5.0	6	10	14	22

E-6: Perforation Discharge in gpm

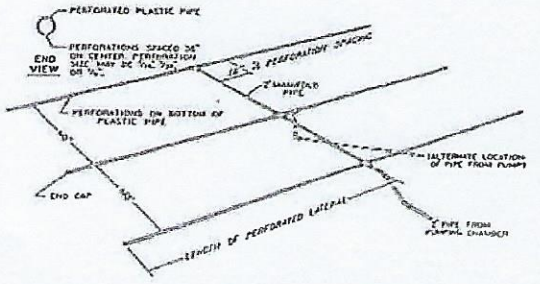
head (feet)	perforation diameter (inches)			
	1/8	3/16	7/32	1/4
1.0[a]	0.18	0.42	0.56	0.74
2.0[b]	0.26	0.59	0.80	1.04
5.0	0.41	0.94	1.26	1.65

[a] Use 1.0 foot for single-family homes.
[b] Use 2.0 feet for anything else.

MANIFOLD LOCATED AT END OF PRESSURE DISTRIBUTION SYSTEM



LAYOUT OF PERFORATED PIPE LATERALS FOR PRESSURE DISTRIBUTION IN MOUND



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Tom Untersee (signature) 1054 (license #) 6-14-20 (date)

PUMP SELECTION PROCEDURE

1. Determine pump capacity:

A. Gravity distribution

1. Minimum required discharge is 10 gpm
2. Maximum suggested discharge is 45 gpm. For other establishments at least 10% greater than the water supply rate, but no faster than the rate at which effluent will flow out of the distribution device.

B. Pressure distribution

See pressure distribution work sheet

From A or B Selected pump capacity: 33.3 gpm

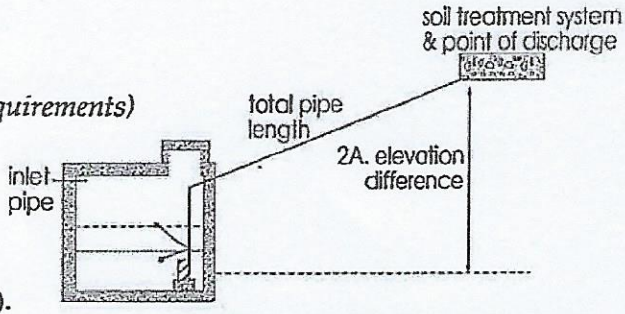
2. Determine pump head requirements:

A. Elevation difference between pump and point of discharge?

9 feet

B. Special head requirement? *(See Figure at right - Special Head Requirements)*

5 feet



Special Head Requirements	
Gravity Distribution	0 ft
Pressure Distribution	5 ft

C. Calculate Friction loss

1. Select pipe diameter 2" in
2. Enter *Figure E-9* with gpm (1A or B) and pipe diameter (C1).

   Read friction loss in feet per 100 feet from Figure E-9

   Friction Loss = 2.06 ft/100ft of pipe
3. Determine total pipe length from pump discharge to soil treatment discharge point. Estimate by adding 25 percent to pipe length for fitting loss. Total pipe length times 1.25 = equivalent pipe length

   50 feet x 1.25 = 62.5 feet
4. Calculate total friction loss by multiplying friction loss (C2) in ft/100 ft by the equivalent pipe length (C3) and divide by 100.

   = 2.06 ft/100ft x 62.5 ÷100 = 1.3 ft

E-9: Friction Loss in Plastic Pipe Per 100 feet			
	nominal pipe diameter		
flow rate gpm	1.5"	2"	3"
20	2.47	0.73	0.11
25	3.73	1.11	0.16
30	5.23	1.55	0.23
35	6.96	2.06	0.30
40	8.91	2.64	0.39
45	11.07	3.28	0.48
50	13.46	3.99	0.58
55		4.76	0.70
60		5.60	0.82
65		6.48	0.95
70		7.44	1.09

D. Total head required is the sum of elevation difference (A), special head requirements (B), and total friction loss (C4)

9 ft + 5 ft+ 1.3 ft =

Total head: 15.3 feet

3. Pump selection

A pump must be selected to deliver at least 33.3 gpm (1A or B) with at least 15.3 feet of total head (2D)

152 ZOEHLER

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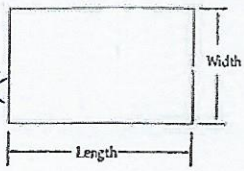
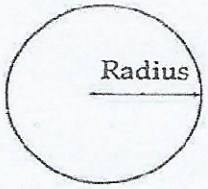
Tom Christensen (signature) 1054 (license #) 6-14-20 (date)

DOSING CHAMBER SIZING

1. Determine area
   A. Rectangle area = L x W
   _____ x _____ = _____ square feet
   B. Circle area = π (3.14) x radius in feet x radius in feet
   3.14 x _____ ft x _____ ft = _____ sqft
   C. Get area from manufacturer _____ sqft

USE Sathen 500gallon Lift Tank





4½" to Inlet
14.13 gp inch

2. Calculate gallons per inch
There are 7.5 gallons per cubic foot of volume, therefore multiply the area (1A, B or C) times the conversion factor and divide by 12 inches per foot to calculate gallon per inch.
Area x 7.5 ÷ 12 = _____ sqft x 7.5 ÷ 12 in/ft = _____ gallon per inch

3. Calculate total tank volume
   A. Depth from bottom of inlet pipe to tank bottom _____ in
   B. Total tank volume = depth from bottom of inlet pipe to tank bottom (3A) x gal/in (2)
   = _____ in x _____ gal/in = _____ gal

Legal Tank:
500 gallons or
100% the Daily flow
or
Alternating Pumps

4. Calculate gallons to cover pump (with 2-3 inches of water covering pump)
(Pump and block height (inch) + 2 inch) x gallon/inch
(_____ in +2 in) x _____ gal/in = _____ gallon

5. Calculate total pumpout volume
   A. Select pump size for 4-5 does per day. Gallon per dose = gpd (see *figure A-1)*
   / doses per day = 450 gpd ÷ 4 doses/day = 112.5 gallons
   B. Calculate drainback
      1. Determine total pipe length, 50 feet
      2. Determine liquid volume of pipe, .17 gal per ft (*see figure E-20)*
      3. Drainback quantity = 50 ft (5B1) x .17 gal per ft (5B2) = 8.5 gal
   C. Total pump out volume = dose volume (5A) + drainback (5B3)
   112.5 gal + 8.5 gal = 121 Total gallon

A-1: Estimated Sewage Flows in Gallons per Day				
number of bedrooms	Class I	Class II	Class III	Class IV
2	300	225	180	60%
3	450	300	218	of the
4	600	375	256	values
5	750	450	294	in the
6	900	525	332	Class I,
7	1050	600	370	II, or III
8	1200	675	408	columns.

E-20: Volume of Liquid in Pipe	
Pipe Diameter *inches*	Gallons per foot
1	0.045
1.25	0.078
1.5	0.11
2	0.17
2.5	0.25
3	0.38
4	0.66

6. Float separation distance (using total pumpout volume)
Total pumpout volume (5C) ÷ gal/inch (2)
121 gal ÷ 14.13 gal/in = 8.5 inch

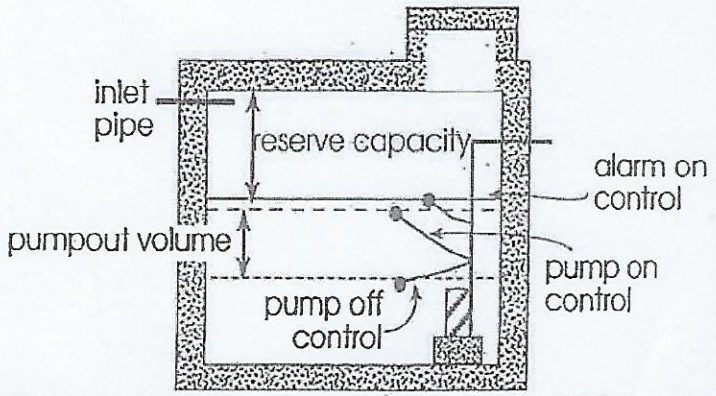
7. Calculate volume for alarm (typically 2 to 3 inches)
Alarm depth (inch) x gallon/inch (2) = _____ in x _____ gal/in) = _____ gal

Set Alarm Float 3" higher than Pump Float

8. Calculate total gallon = gallons over pump (4) + gallons pumpout (5C) + gallons alarm (7)
_____ gal + _____ gal + _____ gal = _____ gallons

9. Total Tank Depth = total gallon (8) ÷ gallon/inch (2)
_____ gal ÷ _____ gal/in = _____ in

Recommended:
Calculate reserve capacity (75% the daily flow)
Daily flow x .75 = 450 x .75 = 337.5 gallons



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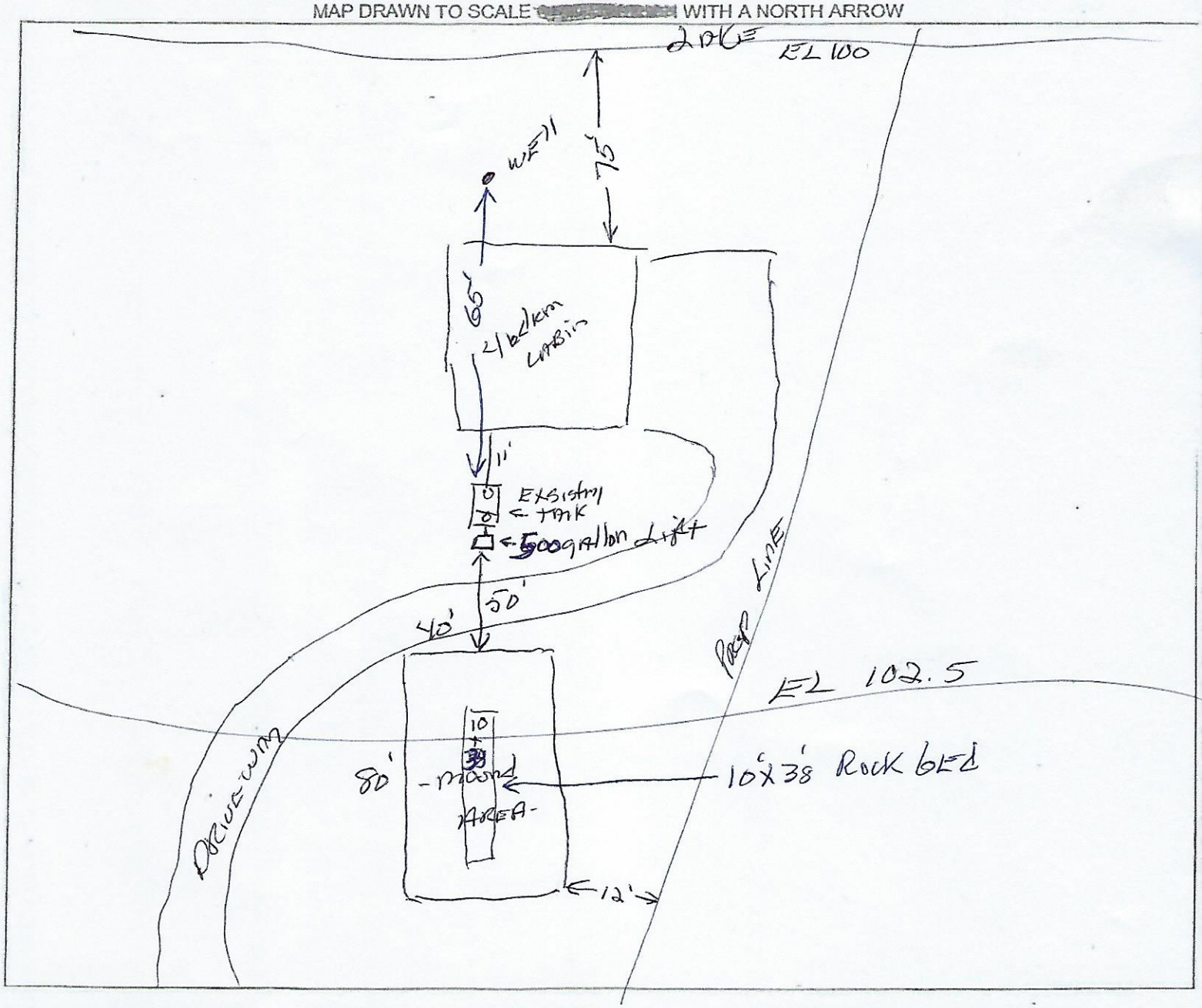
Tom Sentowski (signature) 1054 (license #) 6-14-20 (date)

SKETCH SHEET

CLIENT: Tom Patterson DATE: 6-14-20

MAP DRAWN TO SCALE [redacted] WITH A NORTH ARROW



CHECK OFF LIST--HAVE ALL OF THE FOLLOWING BEEN DRAWN ON THE MAP??

SHOW EXISTING OR PROPOSED

- ☐ WATER WELLS WITHIN 100 FT OF TREATMENT AREAS
- ☐ PRESSURE WATER LINES WITHIN 10 FT OF TREATMENT AREAS
- ☐ STRUCTURES ☐ LOT IMPROVEMENTS
- ☐ ALL SOIL TREATMENT AREAS ☐ ALL ISTS COMPONENTS
- ☐ HORIZONTAL AND VERTICALREFERENCE
- ☐ POINT OF SOIL BORINGS ☐ DIRECTION OF SLOPE
- ☐ LOT EASEMENTS ☐ ALL LOT DIMENSIONS
- ☐ DISTURBED/ COMPACTED AREAS
- ☐ SITE PROTECTION--LATHE AND RIBBON EVERY 15 FT
- ☐ ACCESS ROUTE FOR TANK MAINTENANCE

REQUIRED SETBACKS

- ☐ STRUCTURES ☐ PROPERTY LINES
- ☐ OHWL

COMMENTS:

INDICATE ELEVATIONS

BENCHMARK	Lake = 100
ELEVATION OF SEWER LINE @ HOUSE	101
ELEVATION @ TANK INLET	100
ELEVATION @ BOTTOM OF ROCK LAYER	105
ELEVATION @ BOTTOM OF BORING OR RESTRICTIVE LAYER	101.5
ELEVATION OF PUMP	95
ELEVATION OF DISTRIBUTION DEVICE	105

DESIGNER SIGNATURE Tom Anderson DATE 6-14-20

LICENSE# 1054