

FIELD EVALUATION SHEET

PRELIMINARY EVALUATION DATE 9-29-18 FIELD EVALUATION DATE 6/1/2020
PROPERTY OWNER: CLINT & KIM - DURBOIS PHONE 612-232-8594
ADDRESS: 36386 170th ST CITY, STATE, ZIP: MCGRAW 56350
LEGAL DESCRIPTION: _____
PIN# 28-0-044600 SEC 28 T 44 R 24 TWP NAME Sealeg
FIRE# X LAKE/RIVER X LAKE CLASS X / OHWL X FT.

DESCRIPTION OF SOIL TREATMENT AREAS

	AREA #1	AREA #2	REFERENCE BM ELEV. <u>180</u> FT.
DISTURBED AREAS	YES <u>NO</u> X	YES <u>NO</u>	REFERENCE BM DESCRIPTION
COMPACTED AREAS	YES <u>NO</u> X	YES <u>NO</u>	<u>TOP OF STAIR FOR HOME</u>
FLOODING	YES <u>NO</u> X	YES <u>NO</u>	_____
RUN ON POTENTIAL	YES <u>NO</u> X	YES <u>NO</u>	_____
SLOPE %	<u>8%</u>	YES <u>NO</u>	_____
DIRECTION OF SLOPE	<u>SE</u>	YES <u>NO</u>	_____
LANDSCAPE POSITION	<u>Sideslope</u>	YES <u>NO</u>	_____
VEGETATION TYPES	<u>Wooded Maple, Pine, Poplar</u>	YES <u>NO</u>	_____

DEPTH TO STANDING WATER OR MOTTLED SOIL: BORING# 1 15", 1A 15", 2 2A

BOTTOM ELEVATION - FIRST TRENCH OR BOTTOM OF ROCK BED: #1 12 FT., #2 FT.

SOIL SIZING FACTOR: SITE #1 1.27, SITE #2

CONSTRUCTION RELATED ISSUES: None

LIC# 2006 SITE EVALUATOR SIGNATURE: Dave Engdahl

SITE EVALUATOR NAME: Dave Engdahl TELEPHONE# 592-3606

LUG REVIEW _____ DATE _____

Comments: _____

SOIL BORING LOGS ON REVERSE SIDE

Subsurface Sewage Treatment System Management Plan

Property Owner: CLINT + Kim DUBOIS Phone: 612-232-8554 Date: 6/10/20
 Mailing Address: 26385 170th ST City: MCGRATH Zip: 56350
 Site Address: 26385 170th ST City: _____ Zip: _____

This management plan will identify the operation and maintenance activities necessary to ensure long-term performance of your septic system. Some of these activities must be performed by you, the homeowner. Other tasks must be performed by a licensed septic service provider or maintenance provider.

System Designer: Recommends SSTS check every months.
 Local Government: Recommends SSTS check every months.
 State Requirement: Requires SSTS check every 36 months.
(State requirements are based on MN Rules Chapter 7080.2450, Subp. 2 & 3)

My System needs to be checked every 36 months.



Homeowner Management Tasks:

- ✓ Leaks – Check (look, listen) for leaks in toilets and dripping faucets. Repair leaks promptly.
- ✓ Surfacing sewage – Regularly check for wet or spongy soil around your soil treatment area.
- ✓ Effluent filter – *Inspect and clean twice a year or more.*
- ✓ Alarms – Alarm signals when there is a problem. Contact a service or maintenance provider any time an alarm signals.
- Event counter or water meter – Record your water use.
 - recommend meter readings be conducted (circle one): DAILY WEEKLY MONTHLY (N/A)

Licensed septic service provider or maintenance provider (Check all that apply):

- Check to make sure tank is not leaking
- Check and clean the in-tank effluent filter (if exists)
- Check the sludge/scum layer levels in all septic tanks
- Recommend if tank should be pumped
- Check inlet and outlet baffles
- Check the drainfield effluent levels in the rock layer
- Check the pump and alarm system functions
- Check wiring for corrosion and function
- Check dissolved oxygen and effluent temperature in tank
- Provide homeowner with list of results and any action to be taken
- Flush and clean laterals if cleanouts exist

"I understand it is my responsibility to properly operate and maintain the sewage treatment system on this property, utilizing the Management Plan. If requirements in the Management Plan are not met, I will promptly notify the permitting authority and take necessary corrective actions. If I have a new system, I agree to adequately protect the reserve area for future use as a soil treatment system."

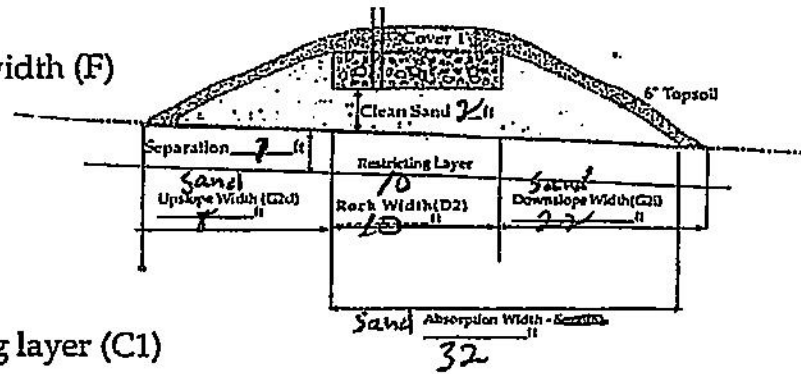
Property Owner Signature:  Date: 6/10/2020
 Designer Signature:  Date: 6/10/2020

See Reverse Side for Management Log

(landslope greater than 1%)

1. Downslope absorption width = absorption width (F) minus rock layer width (D2)

$20 \text{ ft} - 10 \text{ ft} = 10 \text{ ft}$



2. Calculate mound size
UPSLOPE

a. Depth of clean sand fill at upslope edge of rock layer = 3 ft minus the distance to restricting layer (C1)

$3 \text{ ft} - 1 \text{ ft} = 2 \text{ ft}$

D-34: SLOPE MULTIPLIER TABLE

Land Slope, in %	UPSLOPE multipliers for various slope ratios						DOWNSLOPE multipliers for various slope ratios				
	3:1	4:1	5:1	6:1	7:1	8:1	3:1	4:1	5:1	6:1	7:1
0	3.0	4.0	5.0	6.0	7.0	8.0	3.0	4.0	5.0	6.0	7.0
1	2.91	3.85	4.76	5.66	6.54	7.41	3.09	4.17	5.26	6.38	7.53
2	2.83	3.70	4.54	5.36	6.14	6.90	3.19	4.35	5.56	6.82	8.14
3	2.75	3.57	4.35	5.08	5.79	6.45	3.30	4.54	5.88	7.32	8.86
4	2.68	3.45	4.17	4.84	5.46	6.06	3.41	4.76	6.25	7.89	9.72
5	2.61	3.33	4.00	4.62	5.19	5.71	3.53	5.00	6.67	8.57	10.77
6	2.54	3.23	3.85	4.41	4.93	5.41	3.66	5.26	7.14	9.38	12.07
7	2.48	3.12	3.70	4.23	4.70	5.13	3.80	5.56	7.69	10.34	13.73
8	2.42	3.03	3.57	4.05	4.49	4.88	3.95	5.88	8.33	11.54	15.91
9	2.36	2.94	3.45	3.90	4.30	4.65	4.11	6.25	9.09	13.04	18.92
10	2.31	2.86	3.33	3.75	4.12	4.44	4.29	6.67	10.00	15.00	23.33
11	2.26	2.78	3.23	3.61	3.95	4.26	4.48	7.14	11.11	17.65	30.43
12	2.21	2.70	3.12	3.49	3.80	4.08	4.69	7.69	12.50	21.43	43.75

b. Mound height at the upslope edge of rock layer = depth of clean sand for separation (G2a) at upslope edge plus depth of rock layer (1 ft) plus depth of cover (1 ft)

$2 \text{ ft} + 1 \text{ ft} + 1 \text{ ft} = 4 \text{ ft}$

c. Upslope berm multiplier based on land slope 3.03 (see figure D-34)

d. Upslope width = berm multiplier (G2c) x upslope mound height (G2b):

$4 \text{ ft} \times 3.03 \text{ ft} = 12.1 \text{ ft}$

DOWNSLOPE

e. Drop in elevation = rock layer width (D2) x percent landslope (C5) ÷ 100

$10 \text{ ft} \times 8 \% \div 100 = .8 \text{ ft}$

f. Downslope mound height = depth of clean sand for slope difference (G2e) at downslope rock edge plus the mound height at the upslope edge of rock layer (G2b)

$4 \text{ ft} + .8 \text{ ft} = 4.8 \text{ ft}$

g. Downslope berm multiplier based on percent land slc. 5.88 (see figure D-34)

h. Downslope width = downslope multiplier (G2g) times downslope mound height (G2f)

$4.8 \text{ ft} \times 5.88 \text{ ft} = 28.2 \text{ ft}$

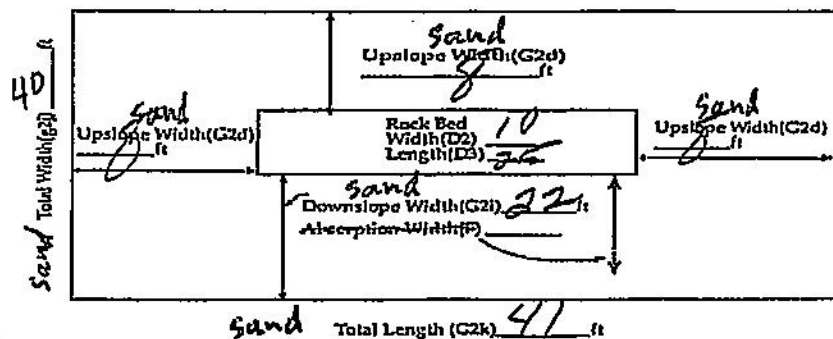
i. Select the greater of G1 and G2h as the downslope width: 28.2 ft

j. Total mound width is the sum of upslope width (G2d) width plus rock layer width (D2) plus downslope width (G2i)

$12.1 \text{ ft} + 10 \text{ ft} + 28.2 \text{ ft} = 50.3 \text{ ft}$

k. Total mound length is the sum of upslope width (G2d) plus rock layer length (D3) plus upslope width (G2d)

$12 \text{ ft} + 25 \text{ ft} + 16 \text{ ft} = 57 \text{ feet}$



Final Dimensions:
50.3 x 57

I hereby certify that I have completed this work in accordance with applicable ordinances, rules and laws.
Bruce Egan (signature) 62006 (license #) 6/10/20 (date)

PRESSURE DISTRIBUTION SYSTEM

1. Select number of perforated laterals, 3
2. Select perforation spacing = 2.5 ft
3. Since perforations should not be placed closer than 1 foot to the edge of the rock layer (see diagram), subtract 2 feet from the rock layer length.

$$\frac{25}{\text{Rock layer length}} - 2 \text{ ft} = 23 \text{ ft}$$

4. Determine the number of spaces between perforations. Divide the length (3) by perforation spacing (2) and round down to nearest whole number.

$$\text{Perforation spacing} = 23 \text{ ft} \div 2.5 \text{ ft} = 9 \text{ spaces}$$

5. Number of perforations is equal to one plus the number of perforation spaces (4). Check figure E-4 to assure the number of perforations per lateral guarantees <10% discharge variation.

$$9 \text{ spaces} + 1 = 10 \text{ perforations/lateral}$$

6. A. Total number of perforations = perforations per lateral (5) times number of laterals (1)

$$10 \text{ perms/lat} \times 3 \text{ lat} = 30 \text{ perforations}$$

- B. Calculate the square footage per perforation. Should be 6-10 sqft/perf. Does not apply to at-grades.

Rock bed area = rock width (ft) x rock length (ft)

$$25 \text{ ft} \times 10 \text{ ft} = 250 \text{ sqft}$$

Square foot per perforation = Rock bed area ÷ number of perms (6)

$$250 \text{ sqft} \div 30 \text{ perms} = 8.3 \text{ sqft/perf}$$

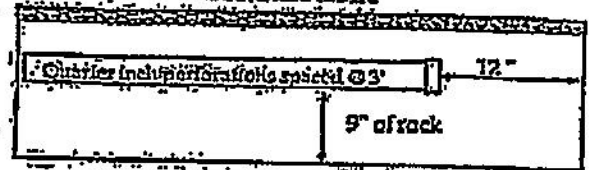
7. Determine required flow rate by multiplying the total number of perforations (6A) by flow per perforation (see figure E-6)

$$30 \text{ perms} \times .74 \text{ gpm/perfs} = 22.2 \text{ gpm}$$

8. If laterals are connected to header pipe as shown on upper example, to select minimum required lateral diameter; enter figure E-4 with perforation spacing (2) and number of perforations per lateral (5) Select minimum diameter for perforated lateral = 1 1/2 inches.

9. If perforated lateral system is attached to manifold pipe near the center, lower diagram, perforated lateral length (3) and number of perforations per lateral (5) will be approximately one half of that in step 8. Using these values, select minimum diameter for perforated lateral = 1 1/2 inches.

Geotextile fabric



Perf Sizing 3/16" - 1/4"
Perf Spacing 1.5' - 5'

E-4: Maximum allowable number of 1/4-inch perforations per lateral to guarantee <10% discharge variation.

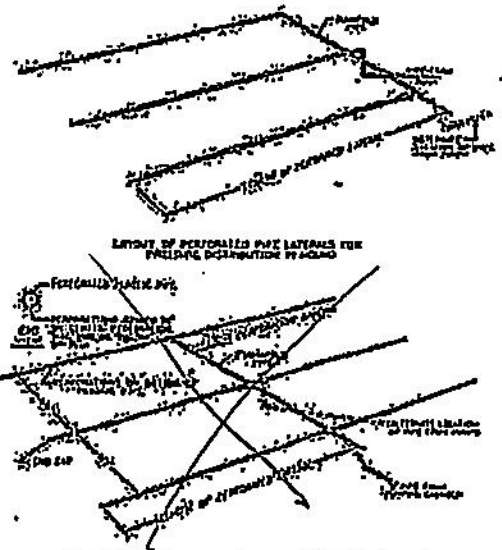
perforation spacing (feet)	1 inch	1.25 inch	1.5 inch	2.0 inch
2.5	8	14	18	28
3.0	8	13	17	26
3.3	7	12	16	25
4.0	7	11	15	23
5.0	6	10	14	22

E-6: Perforation Discharge in gpm

head (feet)	perforation diameter (inches)			
	1/8	3/16	7/32	1/4
1.0 ^a	0.18	0.42	0.56	0.74
2.0 ^b	0.26	0.59	0.80	1.04
5.0	0.41	0.94	1.26	1.66

^a Use 1.0 foot for single-family homes.
^b Use 2.0 foot for anything else.

MANIFOLD LOCATED AT END OF PRESSURE DISTRIBUTION SYSTEM



I hereby certify that I have completed this work in accordance with applicable ordinances, rules and laws.

Diana P. Ladd (Signature) 27006 (License #) 6/10/20 (date)

MOUND DESIGN WORK SHEET (For Flows up to 1200 gpd)

A. Average Design FLOW

Estimated 300 gpd (see figure A-1)
 or measured _____ x 1.5 (safety factor) = _____ gpd

A-1: Estimated Sewage Flows in Gallons per Day

number of bedrooms	Class I	Class II	Class III	Class IV
2	300	225	180	60% of the values in the Class I, II, or III columns.
3	450	300	218	
4	600	375	256	
5	750	450	294	
6	900	525	332	
7	1050	600	370	
8	1200	675	408	

B. SEPTIC TANK Capacity

1,000 gallons (see figure C-1)

C. SOILS (refer to site evaluation)

- Depth to restricting layer = 1.1 feet
- Depth of percolation tests = N/A feet
- Texture SAND LOAM / CLAY
 Percolation rate 16-30 mpi
- Soil loading rate = 60 gpd/sqft (see figure D-33)
- Percent land slope = 8 %

C-1: Septic Tank Capacities (in gallons)

Number of Bedrooms	Minimum Liquid Capacity	Liquid capacity with garbage disposal	Liquid capacity with disposal & infiltration
2 or less	750	1125	1500
3 or 4	1000	1500	2000
5 or 6	1500	2250	3000
7, 8 or 9	2000	3000	4000

D. ROCK LAYER DIMENSIONS

- Multiply average design flow (A) by 0.83 to obtain required rock layer area.
300 gpd x 0.83 sqft/gpd = 250 sqft

- Determine rock layer width = 0.83 sqft/gpd x linear Loading Rate (LLR)
 0.83 sqft/gpd x 12 gpd/sqft = 10 ft.

- Length of rock layer = area ÷ width =
250 sqft (D1) ÷ 10 ft (D2) = 25 ft

Mound LLR

< 120 MPI ≤ 12
 ≥ 120 MPI ≤ 6

E. ROCK VOLUME

- Multiply rock area (D1) by rock depth of 1 ft to get cubic feet of rock
250 sqft x 1 ft = 250 cuft

- Divide cuft by 27 cuft/cuyd to get cubic yards
250 cuft ÷ 27 cuft/cuyd = 9.3 cuyd

- Multiply cubic yards by 1.4 to get weight of rock in tons
9.3 cuyd x 1.4 ton/cuyd = 13 tons

F. SEWAGE ABSORPTION WIDTH

Absorption width equals absorption ratio (See Figure D-33) times rock layer width (D2)

10 x 2.0 ft = 20 ft

D-33: Absorption Width Sizing Table

Percolation Rate in Minutes per Inch (MPI)	Soil Texture	Loading Rate Gallons per cubic per square foot	Absorption Ratio
Faster than 5	Coarse Sand Medium Sand Loamy Sand Fine Gravel	1.20	1.00
6 to 15	Sandy Loam	0.77	1.50
16 to 30	Loam	0.60	2.00
31 to 45	Silt Loam Silt	0.50	2.40
46 to 60	Sandy Clay Loam Silty Clay Loam Clay Loam	0.45	2.67
61 to 120	Silty Clay Sandy Clay Clay	0.24	5.00
Slower than 120*			

*Saturated soil for these soils will not be a factor on performance

PUMP SELECTION PROCEDURE

1. Determine pump capacity:

A. Gravity distribution

1. Minimum required discharge is 10 gpm
2. Maximum suggested discharge is 45 gpm. For other establishments at least 10% greater than the water supply rate, but no faster than the rate at which effluent will flow out of the distribution device.

B. Pressure distribution

See pressure distribution work sheet

From A or B Selected pump capacity: 22.2 gpm

2. Determine pump head requirements:

A. Elevation difference between pump and point of discharge?

5.9 feet

B. Special head requirement? (See Figure at right - Special Head Requirements)

5.0 feet

C. Calculate Friction loss

1. Select pipe diameter 2 in
2. Enter Figure E-9 with gpm (1A or B) and pipe diameter (C1).

Read friction loss in feet per 100 feet from Figure E-9

Friction Loss = 1.11 ft/100ft of pipe

3. Determine total pipe length from pump discharge to soil treatment discharge point. Estimate by adding 25 percent to pipe length for fitting loss. Total pipe length times 1.25 = equivalent pipe length

25 feet x 1.25 = 32 feet

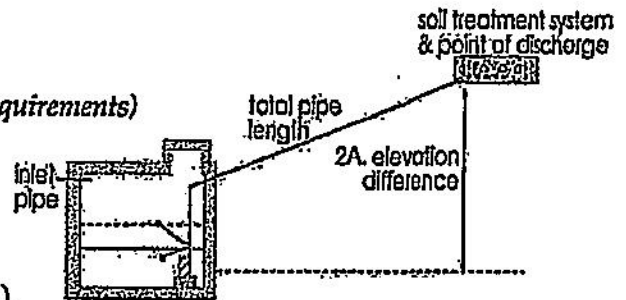
4. Calculate total friction loss by multiplying friction loss (C2) in ft/100 ft by the equivalent pipe length (C3) and divide by 100.

= 1.11 ft/100ft x 32 +100 = .35 ft

D. Total head required is the sum of elevation difference (A), special head requirements (B), and total friction loss (C4)

5.9 ft + 5 ft + .4 ft =

Total head: 11.3 feet



Special Head Requirements	
Gravity Distribution	0 ft
Pressure Distribution	5 ft

flow rate gpm	E-9: Friction Loss in Plastic Pipe Per 100 feet		
	nominal pipe diameter		
	1.5"	2"	3"
20	2.47	0.73	0.11
25	3.73	1.11	0.16
30	5.23	1.55	0.23
35	6.96	2.06	0.30
40	8.91	2.64	0.39
45	11.07	3.28	0.48
50	13.46	3.99	0.58
55		4.76	0.70
60		5.60	0.82
65		6.48	0.95
70		7.44	1.09

3. Pump selection

A pump must be selected to deliver at least 22.2 gpm (1A or B) with at least 11.3 feet of total head (2D)

I hereby certify that I have completed this work in accordance with applicable ordinances, rules and laws.

Bruce E. Dell

(signature)

62006 (license #)

5/10/20 (date)

DOSING CHAMBER SIZING

1. Determine area

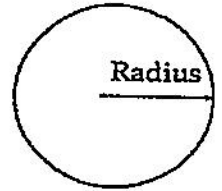
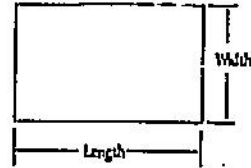
A. Rectangle area = L x W

 x = square feet

B. Circle area = $\pi (3.14) \times$ radius in feet \times radius in feet

3.14 x ft x ft = sqft

C. Get area from manufacturer sqft



2. Calculate gallons per inch

There are 7.5 gallons per cubic foot of volume, therefore multiply the area (1A, B or C) times the conversion factor and divide by 12 inches per foot to calculate gallon per inch.

Area x 7.5 ÷ 12 = sqft x 7.5 ÷ 12 in/ft = 12.1 gallon per inch
MANUFACTURE est.

3. Calculate total tank volume

A. Depth from bottom of inlet pipe to tank bottom 43 in

B. Total tank volume = depth from bottom of inlet pipe to tank bottom (3A) x gal/in (2)
 = 43 in x 12.1 gal/in = 520 gal

Legal Tank:
 500 gallons or
 100% the Daily flow
 or
 Alternating Pumps

4. Calculate gallons to cover pump (with 2-3 inches of water covering pump)

(Pump and block height (inch) + 2 inch) x gallon/inch
(12 in + 2 in) x 12.1 gal/in = gallon

5. Calculate total pumpout volume

A. Select pump size for 4-5 does per day. Gallon per dose = gpd (see figure A-1)
 / doses per day = 300 gpd ÷ 5 doses/day = 60 gallons

B. Calculate drainback

- Determine total pipe length, 30 feet
- Determine liquid volume of pipe, .17 gal per ft (see figure E-20)
- Drainback quantity = 30 ft (5B1) x .17 gal per ft (5B2) = 5.1 gal

C. Total pump out volume = dose volume (5A) + drainback (5B3)
60 gal + 5 gal = 65 Total gallon

6. Float separation distance (using total pumpout volume)

Total pumpout volume (5C) ÷ gal/inch (2)
65 gal ÷ 12.1 gal/in = 5.3 inch

7. Calculate volume for alarm (typically 2 to 3 inches)

Alarm depth (inch) x gallon/inch (2) = 2 in x 12 gal/in = 24 gal

8. Calculate total gallon = gallons over pump (4) + gallons pumpout (5C) + gallons alarm (7)

168 gal + 65 gal + 24 gal = 257 gallons

9. Total Tank Depth = total gallon (8) ÷ gallon/inch (2)

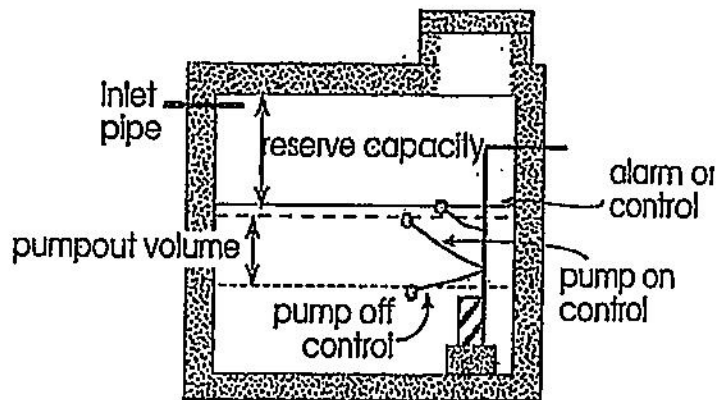
257 gal ÷ 12 gal/in = 21.4 in

A-1: Estimated Sewage Flows in Gallons per Day

number of bedrooms	Class I	Class II	Class III	Class IV
2	300	225	180	60%
3	450	300	218	of the
4	600	375	256	values
5	750	450	294	in the
6	900	525	332	Class I,
7	1050	600	370	II, or III
8	1200	675	408	columns.

E-20: Volume of Liquid in Pipe

Pipe Diameter inches	Gallons per foot
1	0.045
1.25	0.078
1.5	0.11
2	0.17
2.5	0.25
3	0.38
4	0.66



Recommended:

Calculate reserve capacity (75% the daily flow)
 Daily flow x .75 = 300 x .75 = 225 gallons

I hereby certify that I have completed this work in accordance with applicable ordinances, rules and laws.

Brian Egan (signature) 22006 (license #) 6/10/20 (date)

Kim & Clint Dubois

PIT 1

0-10 T.S 7.5yR 3/2
Sand loam
Lots roots
Friable
Blocky

10-15 Sand loam 7.5yR 4/3
Blocky
Friable
Roots

15-22 Sand loam
FIRM 7.5yR 5/3
Blocky 4/4
Reddy 4/4
Few roots

PIT 2

0-7 T.S
Sand loam
Roots
Friable

7-14 Sand loam 7.5yR 4/3
Friable
Blocky
Roots

15- Sand loam
FIRM 7.5yR 4/4
Blocky
Few roots 5/3 4/6

Gene Engel
2006 4/10/20