

# FIELD EVALUATION SHEET

PRELIMINARY EVALUATION DATE Oct. 23, 2019, FIELD EVALUATION DATE Oct. 23, 2019  
PROPERTY OWNER: Gene Kelling PHONE \_\_\_\_\_  
ADDRESS: 44638-State Hwy. 210 CITY, STATE, ZIP: Aitkin, Mn. 56431  
LEGAL DESCRIPTION: W-330ft. of W-660ft. of NE-SW  
PIN# 01-0-070102 SEC 29 T 47 R 27 TWP NAME Aitkin  
FIRE# \_\_\_\_\_ LAKE/RIVER \_\_\_\_\_ LAKE CLASS \_\_\_\_\_ OHWL \_\_\_\_\_ FT.

## DESCRIPTION OF SOIL TREATMENT AREAS

	AREA #1	AREA #2	REFERENCE BM ELEV. <u>100</u> FT.
DISTURBED AREAS	YES _____ NO <u>X</u>	YES _____ NO _____	REFERENCE BM DESCRIPTION _____
COMPACTED AREAS	YES _____ NO <u>X</u>	YES _____ NO _____	<u>Next to house by box</u>
FLOODING	YES _____ NO <u>X</u>	YES _____ NO _____	<u>with hose in it</u>
RUN ON POTENTIAL	YES _____ NO <u>X</u>	YES _____ NO _____	_____
SLOPE %	_____	_____	_____
DIRECTION OF SLOPE	<u>S-N</u>	_____	_____
LANDSCAPE POSITION	<u>E-W</u>	_____	_____
VEGETATION TYPES	<u>Wooded</u>	_____	_____

DEPTH TO STANDING WATER OR MOTTLED SOIL: BORING# 1 14", 1A 15", 2 \_\_\_\_\_, 2A \_\_\_\_\_

BOTTOM ELEVATION—FIRST TRENCH OR BOTTOM OF ROCK BED: #1 102.5 FT., #2 \_\_\_\_\_ FT.

SOIL SIZING FACTOR: SITE #1 1.67, SITE #2 \_\_\_\_\_

CONSTRUCTION RELATED ISSUES: Several large trees must be cut. The closer to the top of the slope by the field the flatter the slope, but other trees may have to be cut

LIC# 22132 SITE EVALUATOR SIGNATURE: Tom O'Neil

SITE EVALUATOR NAME: Tom O'Neil TELEPHONE# 218-927-6070

LUG REVIEW \_\_\_\_\_ DATE \_\_\_\_\_

Comments: \_\_\_\_\_  
\_\_\_\_\_  
\_\_\_\_\_

SOIL BORING LOGS ON REVERSE SIDE

APPROVED  
✓ ONSITE INSPECTION  
NO ONSITE INSPECTION  
RS

SOILS DATA

#1

DEPTH (INCHES)	TEXTURE	MUNSELL COLOR
0-6	Sandy loam	10yr 3/2
6-12	fine Sand	10yr 4/4
12-30	fine Sand	10yr 4/6
mottles at 14"		

SOILS DATA

#2

DEPTH (INCHES)	TEXTURE	MUNSELL COLOR
0-6	Sandy loam	10yr 3/2
6-16	fine Sand	10yr 4/4
16-40	fine Sand	10yr 4/6
mottles at 15"		

SOILS DATA

DEPTH (INCHES)	TEXTURE	MUNSELL COLOR
<p>APPROVED</p> <p><input type="checkbox"/> ONSITE INSPECTION</p> <p><input checked="" type="checkbox"/> NO ONSITE INSPECTION</p> <p>SIGN <u>RS</u> DATE <u>11/19</u></p>		

SOILS DATA

DEPTH (INCHES)	TEXTURE	MUNSELL COLOR
<p>APPROVED</p> <p><input type="checkbox"/> ONSITE INSPECTION</p> <p><input checked="" type="checkbox"/> NO ONSITE INSPECTION</p> <p>SIGN _____ DATE _____</p>		

# MOUND DESIGN WORK SHEET (For Flows up to 1200 gpd)

## A. Average Design FLOW

Estimated 450 gpd (see figure A-1)  
 or measured \_\_\_\_\_ x 1.5 (safety factor) = \_\_\_\_\_ gpd

**A-1: Estimated Sewage Flows in Gallons per Day**

number of bedrooms	Class I	Class II	Class III	Class IV
2	300	225	180	60%
3	450	300	218	of the
4	600	375	256	values
5	750	450	294	in the
6	900	525	332	Class I,
7	1050	600	370	II, or III
8	1200	675	408	columns.

## B. SEPTIC TANK Capacity

1,000 gallons (see figure C-1)  
 use a 1650 Combo Tank

## C. SOILS (refer to site evaluation)

- Depth to restricting layer = 1 feet
- Depth of percolation tests = \_\_\_\_\_ feet
- Texture Sandy loam topsoil  
 Percolation rate 6-15 mpi
- Soil loading rate .79 gpd/sqft (see figure D-33)
- Percent land slope 8 %

**C-1: Septic Tank Capacities (in gallons)**

Number of Bedrooms	Minimum Liquid Capacity	Liquid capacity with garbage disposal	Liquid capacity with disposal & lift inside
2 or less	750	1125	1500
3 or 4	1000	1500	2000
5 or 6	1500	2250	3000
7, 8 or 9	2000	3000	4000

## D. ROCK LAYER DIMENSIONS

- Multiply average design flow (A) by 0.83 to obtain required rock layer area.  
450 gpd x 0.83 sqft/gpd = 375 sqft
- Determine rock layer width = 0.83 sqft/gpd x linear Loading Rate (LLR)  
 0.83 sqft/gpd x 12 gpd/sqft = 10 ft
- Length of rock layer = area ÷ width =  
375 sqft (D1) ÷ 10 ft (D2) = 38 ft

### Mound LLR

< 120 MPI ≤ 12  
 ≥ 120 MPI ≤ 6

## E. ROCK VOLUME

- Multiply rock area (D1) by rock depth of 1 ft to get cubic feet of rock  
380 sqft x 1 ft = 380 cuft
- Divide cuft by 27 cuft/cuyd to get cubic yards  
380 cuft ÷ 27 cuyd/cuft = 14 cuyd
- Multiply cubic yards by 1.4 to get weight of rock in tons  
14 cuyd x 1.4 ton/cuyd = 20 tons

NO SITE INSPECTION  
 DATE 11/1/19

## F. SEWAGE ABSORPTION WIDTH

Absorption width equals absorption ratio (See Figure D-33) times rock layer width (D2)

1.5 x 10 ft = 15 ft

**D-33: Absorption Width Sizing Table**

Percolation Rate in Minutes per Inch (MPI)	Soil Texture	Loading Rate Gallons per day per square foot	Absorption Ratio
Faster than 5	Coarse Sand Medium Sand Loamy Sand Fine Sand	1.20	1.00
6 to 15	Sandy Loam	0.79	1.50
16 to 30	Loam	0.60	2.00
31 to 45	Silt Loam	0.50	2.40
46 to 60	Silt Sandy Clay Loam Silty Clay Loam Clay Loam	0.45	2.67
61 to 120	Silty Clay Sandy Clay Clay	0.24	5.00
Slower than 120*			

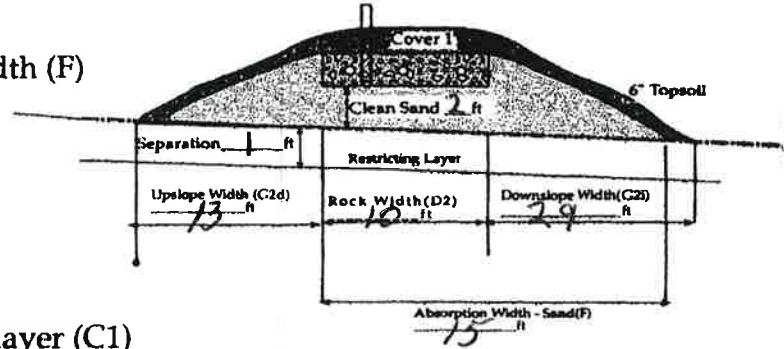
\*System designed for these soils must be other or performance

**G. MOUND SLOPE WIDTH & LENGTH**  
(landslope greater than 1%)

Landslope > 1% slope

1. Downslope absorption width = absorption width (F) minus rock layer width (D2)

15 ft - 10 ft = 5 ft



2. Calculate mound size  
UPSLOPE

a. Depth of clean sand fill at upslope edge of rock layer = 3 ft minus the distance to restricting layer (C1)

3 ft - 1 ft = 2 ft

b. Mound height at the upslope edge of rock layer = depth of clean sand for separation (G2a) at upslope edge plus depth of rock layer (1 ft) plus depth of cover (1 ft)

2 ft + 1ft + 1ft = 4 ft

c. Upslope berm multiplier based on land slope

3.03 (see figure D-34)

d. Upslope width = berm multiplier (G2c) times upslope mound height (G2b):

3.03 x 4 ft = 13 ft

DOWNSLOPE

e. Drop in elevation = rock layer width (D2) times percent landslope (C5) + 100

12 ft x 8 % + 100 = 1 ft

f. Downslope mound height = depth of clean sand for separation (G2a) at downslope rock edge plus the mound height at the upslope edge of rock layer (G2b)

1 ft + 4 ft = 5 ft

g. Downslope berm multiplier based on percent land slope

5.88 (see figure D-34)

h. Downslope width = downslope multiplier (G2g) times downslope mound height (G2f)

5.88 x 5 ft = 29 ft

Select the greater of G1 and G2h as the downslope width: 29 ft  
Total mound width is the sum of upslope width (G2d) width plus rock layer width (D2) plus downslope width (G2i)

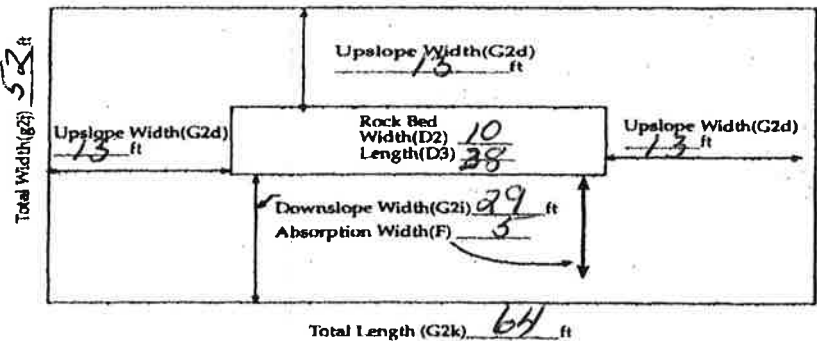
13 ft + 10 ft + 29 ft = 52 ft

Total mound length is the sum of upslope width (G2d) plus rock layer length (D3) plus upslope width (G2d)

13 ft + 38 ft + 13 ft = 64 feet

D-34: SLOPE MULTIPLIER TABLE

Land Slope, in %	UPSLOPE multipliers for various slope ratios						DOWNSLOPE multipliers for various slope ratios				
	3:1	4:1	5:1	6:1	7:1	8:1	3:1	4:1	5:1	6:1	7:1
0	3.0	4.0	5.0	6.0	7.0	8.0	3.0	4.0	5.0	6.0	7.0
1	2.91	3.85	4.76	5.66	6.54	7.41	3.09	4.17	5.26	6.38	7.53
2	2.83	3.70	4.54	5.36	6.14	6.90	3.19	4.35	5.56	6.82	8.14
3	2.75	3.57	4.35	5.08	5.79	6.45	3.30	4.54	5.88	7.32	8.86
4	2.68	3.45	4.17	4.84	5.46	6.06	3.41	4.76	6.25	7.89	9.72
5	2.61	3.33	4.00	4.62	5.19	5.71	3.53	5.00	6.67	8.57	10.77
6	2.54	3.23	3.85	4.41	4.93	5.41	3.66	5.26	7.14	9.38	12.07
7	2.48	3.12	3.70	4.23	4.70	5.13	3.80	5.56	7.69	10.34	13.73
8	2.42	3.03	3.57	4.05	4.49	4.88	3.95	5.88	8.33	11.54	15.91
9	2.36	2.94	3.45	3.90	4.30	4.65	4.11	6.25	9.09	13.04	18.92
10	2.31	2.86	3.33	3.75	4.12	4.44	4.29	6.67	10.00	15.00	23.33
11	2.26	2.78	3.23	3.61	3.95	4.26	4.48	7.14	11.11	17.65	30.43
12	2.21	2.70	3.12	3.49	3.80	4.08	4.69	7.69	12.50	21.43	43.75



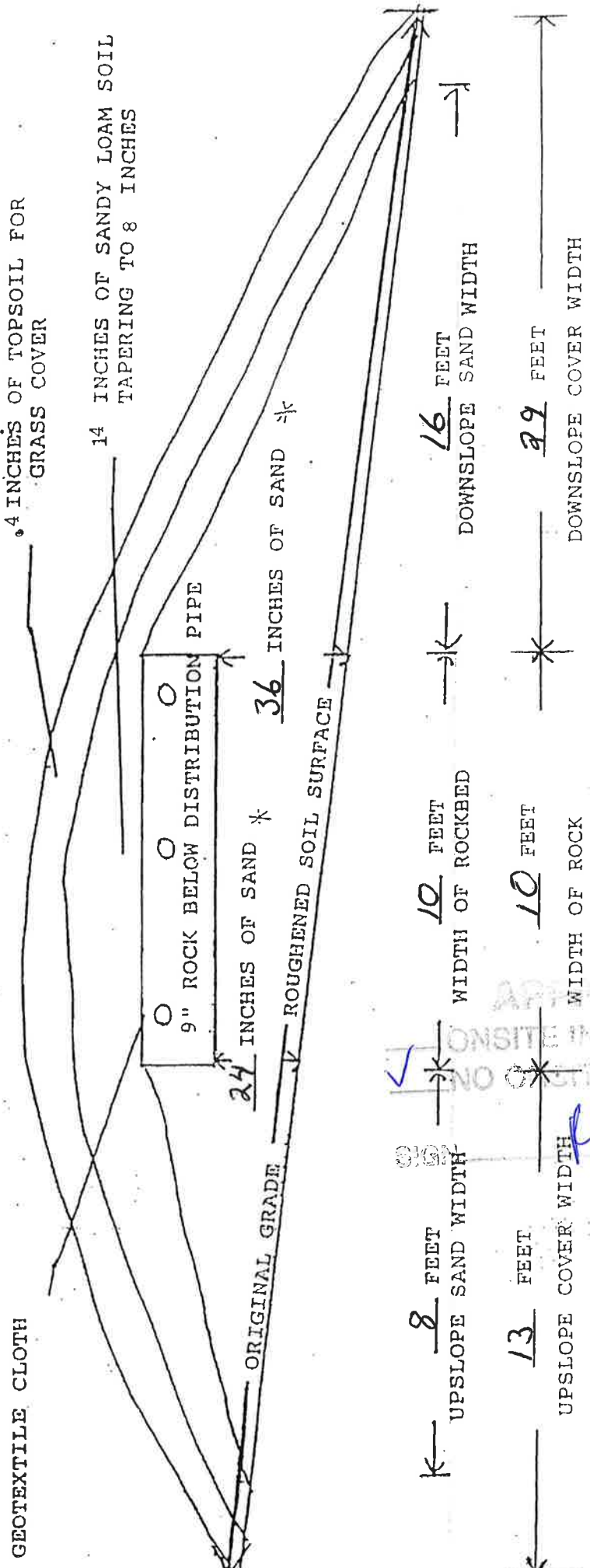
**Final Dimensions:**  
52 x 64

I hereby certify that I have completed this work in accordance with applicable ordinances, rules and laws.

Tom O'Neil (signature) 22132 (license #) Oct 23, 2019 (date)

BOUND CROSS-SECTION

8 PERCENT SLOPE OF ORIGINAL SOIL      10 FT. X 38 FT. SIZE OF ROCKBED      34 FT. X 54 FT. SIZE OF SANDBASE



4 INCHES OF TOPSOIL FOR GRASS COVER

14 INCHES OF SANDY LOAM SOIL TAPERING TO 8 INCHES

9" ROCK BELOW DISTRIBUTION PIPE

36 INCHES OF SAND

ROUGHENED SOIL SURFACE

ORIGINAL GRADE

8 FEET UPSLOPE SAND WIDTH

10 FEET WIDTH OF ROCKBED

16 FEET DOWNSLOPE SAND WIDTH

13 FEET UPSLOPE COVER WIDTH

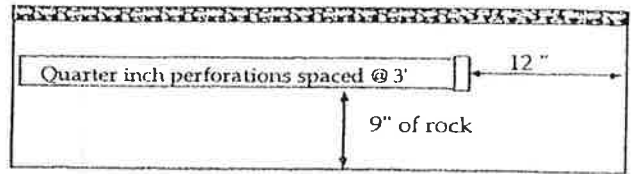
10 FEET WIDTH OF ROCK

29 FEET DOWNSLOPE COVER WIDTH

APPROVED  
 ON-SITE INSPECTION  
 NO ON-SITE INSPECTION  
 SIGN  DATE 11/1/19

# PRESSURE DISTRIBUTION SYSTEM

Geotextile fabric



Perf Sizing 3/16" - 1/4"  
Perf Spacing 1.5' - 5'

E-4: Maximum allowable number of 1/4-inch perforations per lateral to guarantee <10% discharge variation

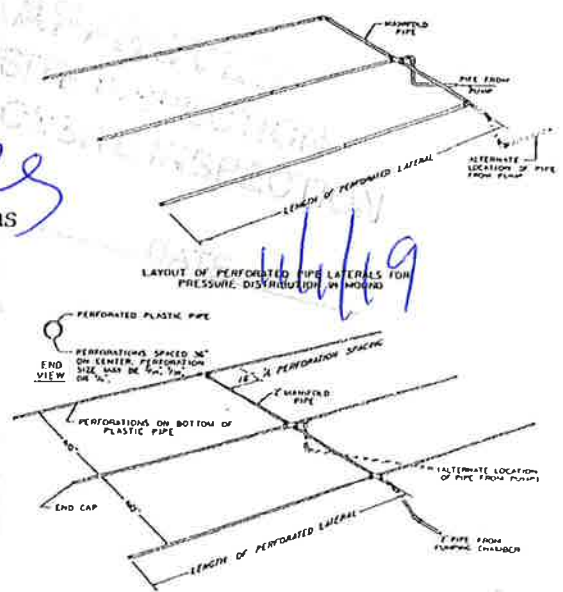
perforation spacing (feet)	1 inch	1.25 inch	1.5 inch	2.0 inch
2.5	8	14	18	28
3.0	8	13	17	26
3.3	7	12	16	25
4.0	7	11	15	23
5.0	6	10	14	22

E-6: Perforation Discharge in gpm

head (feet)	perforation diameter (inches)			
	1/8	3/16	7/32	1/4
1.0 <sup>a</sup>	0.18	0.42	0.56	0.74
2.0 <sup>b</sup>	0.26	0.59	0.80	1.04
5.0	0.41	0.94	1.26	1.65

<sup>a</sup> Use 1.0 foot for single-family homes.  
<sup>b</sup> Use 2.0 feet for anything else.

MANIFOLD LOCATED AT END OF PRESSURE DISTRIBUTION SYSTEM



1. Select number of perforated laterals 3

2. Select perforation spacing = 3 ft

3. Since perforations should not be placed closer than 1 foot to the edge of the rock layer (see diagram), subtract 2 feet from the rock layer length.

$$\frac{38}{\text{Rock layer length}} - 2 \text{ ft} = 36 \text{ ft}$$

4. Determine the number of spaces between perforations. Divide the length (3) by perforation spacing (2) and round down to nearest whole number.

$$\text{Perforation spacing} = 36 \text{ ft} \div 3 \text{ ft} = 12 \text{ spaces}$$

5. Number of perforations is equal to one plus the number of perforation spaces(4). Check figure E-4 to assure the number of perforations per lateral guarantees <10% discharge variation.

$$12 \text{ spaces} + 1 = 13 \text{ perforations/lateral}$$

6. A. Total number of perforations = perforations per lateral (5) times number of laterals (1)

$$13 \text{ perfs/lat} \times 3 \text{ lat} = 39 \text{ perforations}$$

B. Calculate the square footage per perforation.

Should be 6-10 sqft/perf. Does not apply to at-grades.

Rock bed area = rock width (ft) x rock length (ft)

$$10 \text{ ft} \times 38 \text{ ft} = 380 \text{ sqft}$$

Square foot per perforation = Rock bed area ÷ number of perfs (6)

$$380 \text{ sqft} \div 39 \text{ perfs} = 9.07 \text{ sqft/perf}$$

7. Determine required flow rate by multiplying the total number of perforations (6A) by flow per perforation (see figure E-6)

$$39 \text{ perfs} \times 0.74 \text{ gpm/perfs} = 29 \text{ gpm}$$

8. If laterals are connected to header pipe as shown on upper example, to select minimum required lateral diameter; enter figure E-4 with perforation spacing (2) and number of perforations per lateral (5) Select minimum diameter for perforated lateral = 1.25 inches.

9. If perforated lateral system is attached to manifold pipe near the center, lower diagram, perforated lateral length (3) and number of perforations per lateral (5) will be approximately one half of that in step 8. Using these values, select minimum diameter for perforated lateral = 1 inches.

I hereby certify that I have completed this work in accordance with applicable ordinances, rules and laws.

Tom O'Neil (signature)

(signature)

L2132 (license #)

(license #)

Oct. 23, 2019 (date)

(date)

## PUMP SELECTION PROCEDURE

### A. Determine pump capacity

#### Gravity distribution

1. Minimum is 10 GPM
2. Maximum is 45 GPM

#### Pressure Distribution

3. a. Select number of perforated laterals 3
- b. Select perforation spacing = 3 ft.
- c. Subtract 2 ft from rock layer length:  
 $\underline{38} - 2 = \underline{36}$  feet. (Length of laterals)

### ROCK LAYER LENGTH

- d. Determine the number of spaces between perfs:

$$\frac{\underline{36}}{\text{(length of lateral)}} \div \frac{\underline{3}}{\text{(perf. spacing)}} = \underline{12} \text{ spaces}$$

- e.  $\underline{12}$  spaces + 1 =  $\underline{13}$  perforations per lateral

- f. Multiply perforations per lateral by number of laterals to get total number of perforations:

$$\frac{\underline{13}}{\text{(perfs/lateral)}} \times \frac{\underline{3}}{\text{(laterals)}} = \underline{39} \text{ (perforations)}$$

- g.  $\underline{39} \times \underline{0.74} = \underline{29}$  GPM  
 (Perforations) x (gpm/perfs)

SELECTED PUMP CAPACITY 29 GPM

### B. Determine head requirements:

1. Elevation difference between pump & point of discharge:

$$\underline{18} \text{ feet}$$

2. If pumping to a pressure distribution system, add 5 feet; for gravity add zero: 5 feet

3. Friction Loss

- a. Enter friction loss table with GPM and pipe diameter. Read friction loss in feet per 100 ft in table.

$$\text{F.L.} = \underline{1.55} \text{ ft/100 of pipe}$$

- b. Determine total pipe length from pump to discharge point. Add 25% to pipe length for fitting loss.

$$\underline{67} \text{ length} \times 1.25 = \underline{84} \text{ feet.}$$

- c. Calculate total friction loss by multiplying friction loss in 100 ft. of pipe by equivalent pipe length (B):

$$\text{Total friction loss} = \underline{1.55} \times \underline{84} / 100 = \underline{2} \text{ feet}$$

4. Total head required is the sum of the elevation difference, special head requirements and total friction loss:

$$\frac{\underline{18}}{(1)} + \frac{\underline{5}}{(2)} + \frac{\underline{2}}{(3c)} \text{ TOTAL HEAD } \underline{25}$$

SELECT A PUMP TO DELIVER AT LEAST 29 GPM WITH AT LEAST 25 FEET OF TOTAL HEAD.

If laterals are connected to a header pipe in a pressure system, select the minimum size lateral diameter; enter the table with perforation spacing and the number of perforations per lateral.

Select minimum size of lateral 1 1/4  
 For a center manifold system the values will be 1/2 of above.

## Perforation Discharges in GPM

Head (feet)	Perforation diameter (inches)	
1.0a	7/32	1/4
	0.56	<u>0.74</u>
1.5	0.69	0.90
2.0b	0.80	1.04

- a. Use 1.0 foot single homes
- b. Use 2.0 feet for anything else

## FRICTION LOSS IN PLASTIC PIPE

Flow Rate GPM	1.5"	2"	3"
20	2.47	0.73	0.11
25	3.73	1.11	0.16
30	5.23	<u>1.55</u>	0.23
35	6.96	2.06	0.30
40	8.91	2.64	0.39
45	11.07	3.28	0.48
50	13.46	3.99	0.58
55		4.76	0.70
60		5.60	0.82
65		6.48	0.95
70		7.44	1.09

Max. No. of 1/4" perfs per lateral. (10% var)

Perforation spacing (feet)	1/4" spacing		
	1	1	2"
2.5 feet	14	18	28
3.0 feet	13	17	26
3.3 feet	12	16	25
4.0 feet	11	15	23
5.0 feet	10	14	22

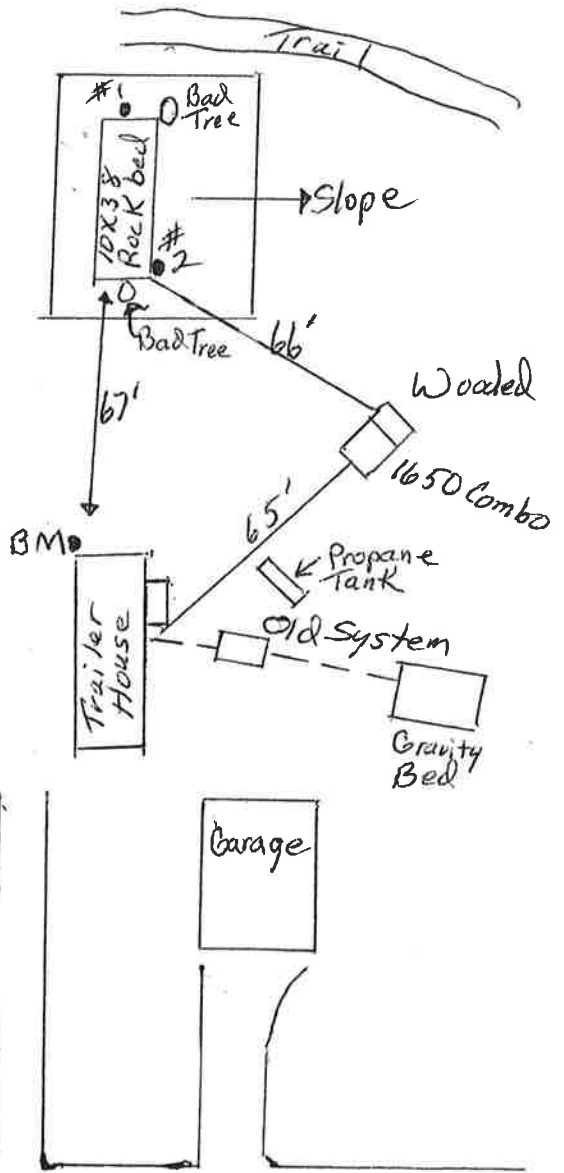
← north

State Hwy 270

ENCLOSURE  
NO TREE REMOVAL  
SIGN *RS*  
DATE 11/19

Field

Field



Frem's Cabinet Shop

Approximate Elevations:

Bench Mark	100
Tank Inlet	89
Pump Elevation	85
Manifold	103
Bottom of Rock	102.25



# Subsurface Sewage Treatment System Management Plan

Property Owner: Gene Kelling Phone: 218-927-3084 Date: Oct 23, 2019  
 Mailing Address: 44999-310<sup>th</sup> Street City: Aitkin, Mn. Zip: 56431  
 Site Address: 44638 State Hwy. 210 City: Aitkin, Mn. Zip: 56431

This management plan will identify the operation and maintenance activities necessary to ensure long-term performance of your septic system. Some of these activities must be performed by you, the homeowner. Other tasks must be performed by a licensed septic service provider.

System Designer: check every \_\_\_\_\_ months.  
 Local Government: check every \_\_\_\_\_ months.  
 State Requirement: check every 36 months.

**My System needs to be checked every 36 months.**

*(State requirements are based on MN Rules Chapter 7080.2450, Subp. 2 & 3)*

### Homeowner Management Tasks

- Leaks* – Check (look, listen) for leaks in toilets and dripping faucets. Repair leaks promptly.
- Surfacing sewage* – Regularly check for wet or spongy soil around your soil treatment area.
- Effluent filter* – *Inspect and clean twice a year or more.*
- Alarms* – Alarm signals when there is a problem. Contact a service provider any time an alarm signals.
- Event counter or water meter* – Record your water use.  
 -recommend meter readings be conducted (circle one: DAILY WEEKLY MONTHLY)

### Professional Management Tasks *Have Tank Pumper check these when tank is pumped*

- Check to make sure tank is not leaking
- Check and clean the in-tank effluent filter
- Check the sludge/scum layer levels in all septic tanks
- Recommend if tank should be pumped *Sooner*
- Check inlet and outlet baffles
- Check the drainfield effluent levels in the rock layer
- Check the pump and alarm system functions
- Check wiring for corrosion and function
- Check dissolved oxygen and effluent temperature in tank
- Provide homeowner with list of results and any action to be taken
- Flush and clean laterals if cleanouts exist

APPROVED  
 ONSITE INSPECTION  
 NO ONSITE INSPECTION  
 SIGN: PS DATE: 10/23/19

"I understand it is my responsibility to properly operate and maintain the sewage treatment system on this property, utilizing the Management Plan. If requirements in the Management Plan are not met, I will promptly notify the permitting authority and take necessary corrective actions. If I have a new system, I agree to adequately protect the reserve area for future use as a soil treatment system."

Property Owner Signature: \_\_\_\_\_ Date: \_\_\_\_\_

Designer Signature: Tom O'Neil Date: Oct 23, 2019

**See Reverse Side for Management Log**

Parcel Description



Press Enter to continue or enter new parcel/tax year: 01-0-070102 2020

**Parcel/Acct:** 01-0-070102 768 05  
**Asmt/Tax year:** 2019 / 2020  
**Type:** RE  
**Data set:** PRD  
**Production:** 2020  
**Primary owner:** 105592 KELLING, GENE  
**Escrow:**  
**FALCO:** 1 F.O.  
**Notes:**  
**Taxpayer:** 105592 KELLING, GENE  
**Hold tax stmt:**  
**Emergency #:**  
**Undeliverable Tax Address:** N  
**Lease type:**  
**Surveyed:**  
**Ref. parcel:** 00201000070102  
**Unit:**  
**Lake #/Name:**  
**Com district:** 1  
**UDI:** 100.00%  
**Physical adr:** 44638 STATE HWY 210  
**MH court nbr:**  
**Billing:** P  
**TIF district:**

User defined: AITKIN 56431

**TIF Knock down date:**  
**Unique Taxing Area:**  
**AMBU:** \*\*\*\*  
**Lot/Block:** /  
**Acres:** 10.00  
**Plat/Desc:**  
**Sec/Twp/Rge:** 29 47.0 27

**Legal:** W 330 FT OF W 660 FT OF NE-SW  
**Legal Description:**  
**Acres:** 10.00  
**Plat/Desc:**  
**Sec/Twp/Rge:** 29 47.0 27

(version 1)

