

FIELD EVALUATION SHEET

PRELIMINARY EVALUATION DATE

PROPERTY OWNER: CLAUDIA

ADDRESS: 19629 327TH AVE JACOB

LEGAL DESCRIPTION:

PIN#

FIRE#

LAKE/RIVER

MILLE LACS

, FIELD EVALUATION DATE

PHONE 612-599-5131

CITY, STATE, ZIP: ISLE MN 56342

SEC T 44 R 25

TWP NAME LAKE CLASS

OHWL

FT.

DESCRIPTION OF SOIL TREATMENT AREAS

AREA #1

DISTURBED AREAS

YES NO

COMPACTED AREAS

YES NO

FLOODING

YES NO

RUN ON POTENTIAL

YES NO

SLOPE %

YES NO

DIRECTION OF SLOPE

—

LANDSCAPE POSITION

—

VEGETATION TYPES

—

LAWN AREA

AREA #2

YES NO

YES NO

YES NO

YES NO

REFERENCE BM ELEV.

FT

REFERENCE BM DESCRIPTION

100' BENCHMARK
IS GROUND LEVEL
BY HOUSE NEXT TO
EXISTING CLEANOUT

DEPTH TO STANDING WATER OR MOTTLED SOIL: BORING# 1 12", 1A 12', 2 , 2A

BOTTOM ELEVATION--FIRST TRENCH OR BOTTOM OF ROCK BED: #1 FT., #2 FT.

SOIL SIZING FACTOR: SITE #1 1.27 , SITE #2

CONSTRUCTION RELATED ISSUES: NEW 1650 TANK. 10'X38' ROCKBED. LOT
HAS BEEN FILLED YEARS AGO

LIC# 127

SITE EVALUATOR SIGNATURE:

Larry Lymquist

SITE EVALUATOR NAME: LARRY LYMQUIST TELEPHONE# 318 820 8886

LUG REVIEW

DATE

Comments:

SOIL BORING LOGS ON REVERSE SIDE

Average Design Flow (A) = 450 gpd (see figure A-1) Flows up to 1200 gpd

A. Average Design FLOW

Estimated 450 gpd (see figure A-1)
or measured _____ $\times 1.5$ (safety factor) = _____ gpd

B. SEPTIC TANK Capacity

1000 gallons (see figure C-1)

C. SOILS (refer to site evaluation)

1. Depth to restricting layer = 1 feet
2. Depth of percolation tests = feet
3. Texture SANDY LOAM
Percolation rate 6-15 mpi
4. Soil loading rate 790 gpd/sqft (see figure D-33)
5. Percent land slope 0 %

A-1: Estimated Sewage Flows in Gallons per Day

number of bedrooms	Class I	Class II	Class III	Class IV
2	300	225	180	60%
3	450	300	218	of the values in the
4	600	375	256	Class I, II, or III
5	750	450	294	columns.
6	900	525	332	
7	1050	600	370	
8	1200	675	408	

C-1: Septic Tank Capacities (in gallons)

Number of Bedrooms	Minimum Liquid Capacity	Liquid capacity with garbage disposal	Liquid capacity with disposal & lift inside
2 or less	750	1125	1500
3 or 4	1000	1500	2000
5 or 6	1500	2250	3000
7, 8 or 9	2000	3000	4000

D. ROCK LAYER DIMENSIONS

1. Multiply average design flow (A) by 0.83 to obtain required rock layer area.
450 gpd $\times 0.83 \text{ sqft/gpd} = 380 \text{ sqft}$
2. Determine rock layer width = 0.83 sqft/gpd \times linear Loading Rate (LLR)
0.83 sqft/gpd $\times 10 \text{ gpd/sqft} = 10 \text{ ft}$
3. Length of rock layer = area \div width =
380 sqft (D1) $\div 10 \text{ ft (D2)} = 38 \text{ ft}$

E. ROCK VOLUME

1. Multiply rock area (D1) by rock depth of 1 ft to get cubic feet of rock
380 sqft $\times 1 \text{ ft} = 380 \text{ cuft}$
2. Divide cuft by 27 cuft/cuyd to get cubic yards
380 cuft $\div 27 \text{ cuyd/cuft} = 14 \text{ cuyd}$
3. Multiply cubic yards by 1.4 to get weight of rock in tons
14 cuyd $\times 1.4 \text{ ton/cuyd} = 19.6 \text{ tons}$

Mound LLR	
< 120 MPI	< 12
$\geq 120 \text{ MPI}$	≤ 6

F. SEWAGE ABSORPTION WIDTH

Absorption width equals absorption ratio (See Figure D-33)
times rock layer width (D2)

$$1.5 \times 10 \text{ ft} = 15 \text{ ft}$$

D-33: Absorption Width Sizing Table

Percolation Rate in Minutes per Inch (MPI)	Soil Texture	Loading Rate Gallons per day per square foot	Absorption Ratio
Faster than 5	Coarse Sand Medium Sand Loamy Sand Fine Sand Sandy Loam Loam Silt Loam Silt	1.20	1.00
6 to 15	Sandy Clay Loam Silty Clay Loam Clay Loam Silty Clay Sandy Clay Clay	0.22 0.60 0.30 0.45 0.24	1.81 2.00 2.40 2.07 3.00
16 to 30			
31 to 45			
46 to 60			
61 to 120			
Slower than 120*			

*Systems designed for these soils must be rated for performance

G. Mound Slope Width and Length
(landslope less than or equal to 1%)

1. Absorption width (F) 15 ft

2. Calculate mound size

a. Determine depth of clean sand fill

at upslope edge of rock layer = 3 ft

minus the distance to restricting layer (C1)

$$3 \text{ ft} - 1 \text{ ft} = 2 \text{ ft}$$

b. Mound height at the upslope edge of rock layer = depth of clean sand for separation (G2a)

at upslope edge plus depth of rock layer (1 ft) plus depth of cover (1 ft)

$$2 \text{ ft} + 1 \text{ ft} + 1 \text{ ft} = 4 \text{ ft}$$

c. Berm width = upslope mound height (G2b) times 4 (4 is recommended, but could be 3-12)

$$4 \times 4 = 16 \text{ ft}$$

d. The total landscape width is the sum of berm (G2c) width plus rock layer width (D2) plus berm width (G2c): 16 ft + 10 ft + 16 ft = 42 ft

e. Additional width necessary for absorption = absorption width (F) minus the landscape width (G2d)

$$\text{_____ ft} - \text{_____ ft} = \text{_____ ft}, \text{ if number is negative } (<0) \text{ skip to g}$$

f. Final berm width = additional width (G2e) plus the berm width (G2c)

$$\text{_____ ft} + \text{_____ ft} = \text{_____ ft}$$

g. Total mound width is the sum of berm width (G2f or G2c) plus rock layer width (D2) plus berm width (G2f or G2c): 16 ft + 10 ft + 16 ft = 42 ft

h. Total mound length is the sum of berm (G2f or G2c) plus rock layer length (D3) plus berm (G2f or G2c): 16 ft + 38 ft + 16 ft = 70 ft

i. Setbacks from the rockbed are calculated as follows: the absorption width (F) minus the rock bed width (D2) divided by 2: (15 ft - 10 ft) ÷ 2 = 2.5 ft

Final Dimensions:

42 x 70

SANDBASE: 28' x 56'



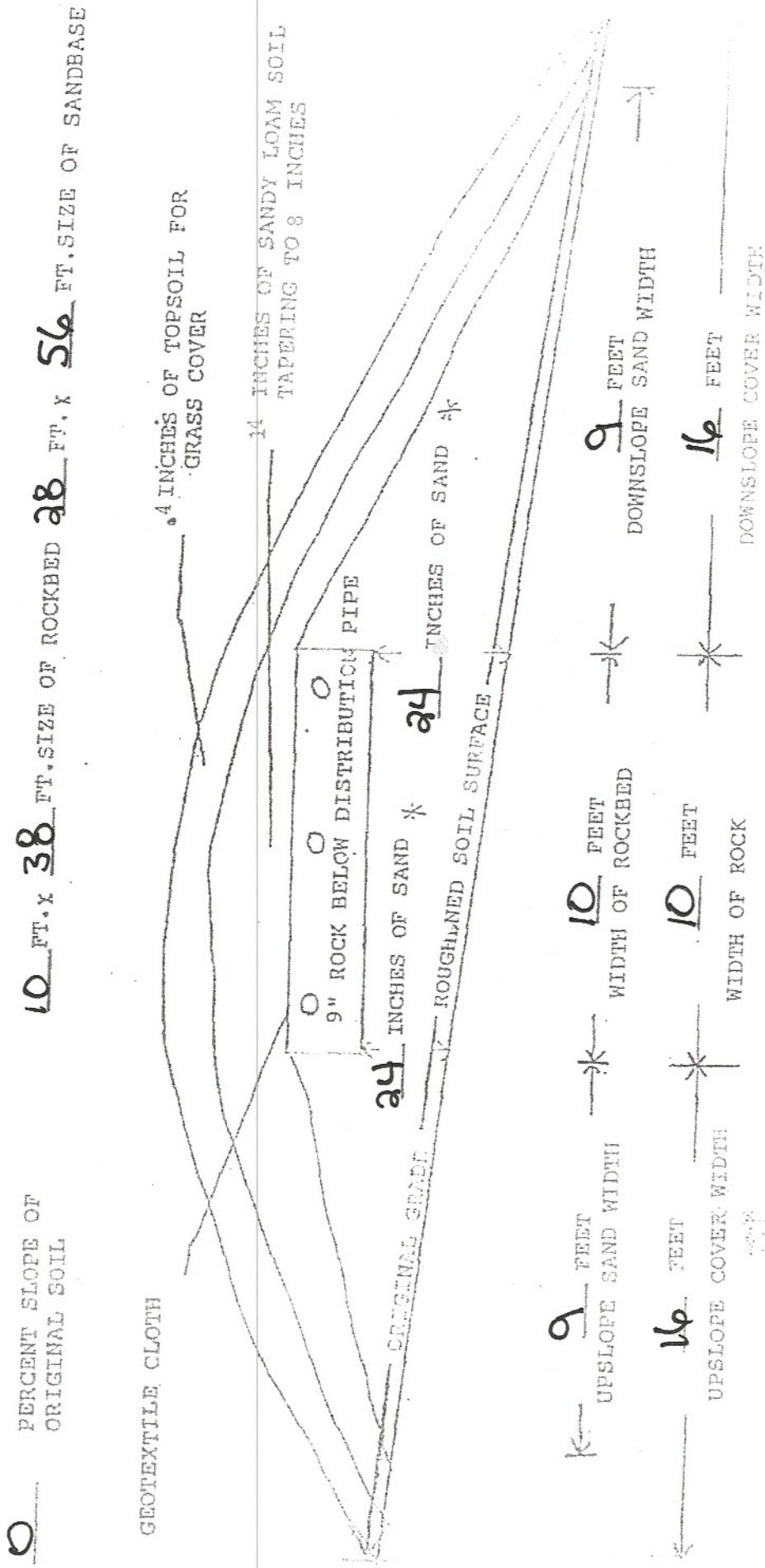
I hereby certify that I have completed this work in accordance with applicable ordinances, rules and laws.

Larry Glynnquist

(signature) 127

(license #) 7-18-19 (date)

MOULD CROSS-SECTION



2. Select perforation spacing = 3 ft

3. Since perforations should not be placed closer than 1 foot to the edge of the rock layer (see diagram), subtract 2 feet from the rock layer length.

$$\begin{array}{r} \text{Rock layer length} \\ \hline 38 \\ - 2 \text{ ft} \\ \hline 36 \text{ ft} \end{array}$$

4. Determine the number of spaces between perforations. Divide the length (3) by perforation spacing (2) and round down to nearest whole number.

$$\text{Perforation spacing} = \underline{36} \text{ ft} \div \underline{3} \text{ ft} = \underline{12} \text{ spaces}$$

5. Number of perforations is equal to one plus the number of perforation spaces(4). Check figure E-4 to assure the number of perforations per lateral guarantees <10% discharge variation.

$$\underline{12} \text{ spaces} + 1 = \underline{13} \text{ perforations/lateral}$$

6. A. Total number of perforations = perforations per lateral (5) times number of laterals (1)

$$\underline{13} \text{ perfs/lat} \times \underline{3} \text{ lat} = \underline{39} \text{ perforations}$$

B. Calculate the square footage per perforation.

Should be 6-10 sqft/perf. Does not apply to at-grades.

Rock bed area = rock width (ft) X rock length (ft)

$$10 \text{ ft} \times \underline{38} \text{ ft} = \underline{380} \text{ sqft}$$

Square foot per perforation = Rock bed area ÷ number of perfs (6)

$$\underline{380} \text{ sqft} \div \underline{39} \text{ perfs} = \underline{9.7} \text{ sqft/perf}$$

7. Determine required flow rate by multiplying the total number of perforations (6A) by flow per perforation (see figure E-6)

$$\underline{39} \text{ perfs} \times \underline{.74} \text{ gpm/perf} = \underline{29} \text{ gpm}$$

8. If laterals are connected to header pipe as shown on upper example, to select minimum required lateral diameter; enter figure E-4 with perforation spacing (2) and number of perforations per lateral (5) Select minimum diameter for perforated lateral = 1 1/4 inches.

9. If perforated lateral system is attached to manifold pipe near the center, lower diagram, perforated lateral length (3) and number of perforations per lateral (5) will be approximately one half of that in step 8. Using these values, select minimum diameter for perforated lateral = _____ inches.

I hereby certify that I have completed this work in accordance with applicable ordinances, rules and laws

Larry Alyngiel

(Signature)

127

(license #)

7-18-19 (date)

Perf Sizing 3/16" - 1/4"
Perf Spacing 1.5" - 5"

E-4: Maximum allowable number of 1/4-inch perforations per lateral to guarantee <10% discharge variation

perforation spacing (feet)	1 inch	1.25 inch	1.5 inch	2.0 inch
2.5	8	14	18	28
3.0	8	13	17	26
3.3	7	12	16	25
4.0	7	11	15	23
5.0	6	10	14	22

E-6: Perforation Discharge in gpm

head (feet)	perforation diameter (inches)			
	1/8	3/16	7/32	1/4
1.0 ^a	0.18	0.42	0.56	0.74
2.0 ^b	0.26	0.59	0.80	1.02
5.0	0.41	0.94	1.26	1.65

^a Use 1.0 foot for single-family homes.

^b Use 2.0 feet for anything else.

MANIFOLD LOCATED AT END OF PRESSURE DISTRIBUTION SYSTEM

LAYOUT OF PERFORATED PIPE LATERALS FOR



1. Determine pump capacity:

A. Gravity distribution

1. Minimum required discharge is 10 gpm
2. Maximum suggested discharge is 45 gpm. For other establishments at least 10% greater than the water supply rate, but no faster than the rate at which effluent will flow out of the distribution device.

B. Pressure distribution

See pressure distribution work sheet

From A or B Selected pump capacity: 29 gpm

2. Determine pump head requirements:

A. Elevation difference between pump and point of discharge?

8 feet

B. Special head requirement? (See Figure at right - Special Head Requirements)

5 feet

C. Calculate Friction loss

1. Select pipe diameter 2 in

2. Enter Figure E-9 with gpm (1A or B) and pipe diameter (C1).

Read friction loss in feet per 100 feet from Figure E-9

Friction Loss = 1.55 ft/100ft of pipe

3. Determine total pipe length from pump discharge to soil treatment discharge point. Estimate by adding 25 percent to pipe length for fitting loss. Total pipe length times 1.25 = equivalent pipe length

30 feet x 1.25 = 37.5 feet

4. Calculate total friction loss by multiplying friction loss (C2) in ft/100 ft by the equivalent pipe length (C3) and divide by 100.
 $= 1.55 \text{ ft/100ft} \times 37.5 \div 100 = .58 \text{ ft}$

D. Total head required is the sum of elevation difference (A), special head requirements (B), and total friction loss (C4)

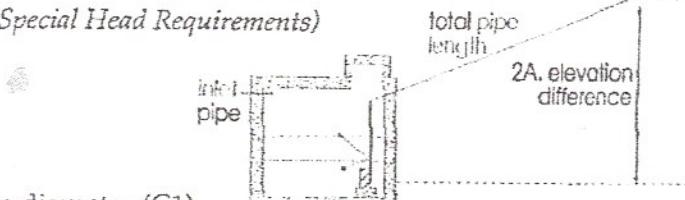
8 ft + 5 ft + .58 ft =

Total head: 13.58 feet

3. Pump selection

A pump must be selected to deliver at least 29 gpm (1A or B) with at least 14 feet of total head (2D)

soil treatment system
& point of discharge
[estimated]



Special Head Requirements		
Gravity Distribution	0 ft	
Pressure Distribution	5 ft	

flow rate gpm	E-9: Friction Loss In Plastic Pipe Per 100 feet		
	nominal pipe diameter 1.5"	2"	3"
20	2.47	0.73	0.11
25	3.73	1.11	0.16
30	5.23	1.55	0.23
35	6.96	2.06	0.30
40	8.91	2.64	0.39
45	11.07	3.28	0.48
50	13.46	3.99	0.58
55		4.76	0.70
60		5.60	0.82
65		6.48	0.95
70		7.44	1.09

I hereby certify that I have completed this work in accordance with applicable ordinances, rules and laws.

Darryl Dlyngard

(signature)

127

(license #)

1-18-19

(date)

SOILS CHARTS FOR BOTH PROPOSED AND ALTERNATE SITES

1 (PROPOSED) SOILS DATA

DEPTH (INCHES)	TEXTURE	MUNSEY COLOR
0-7	SANDY	10 YR 3/2
7-10	SANDY LOAM	10 YR 4/1
10-14	SANDY LOAM	10 YR 4/4 10 YR 3/2

MIXED SOILS

2 (PROPOSED) SOILS DATA

DEPTH (INCHES)	TEXTURE	MUNSEY COLOR
0-7	LOAM	10 YR 3/2
7-12	SANDY LOAM	10 YR 4/2

MIXED SOILS

1 (ALTERNATE) SOILS DATA

DEPTH (INCHES)	TEXTURE	MUNSEY COLOR
0-14		

2 (ALTERNATE) SOILS DATA

DEPTH (INCHES)	TEXTURE	MUNSEY COLOR
0-14		

ADDITIONAL SOIL BORINGS MAY BE REQUIRED

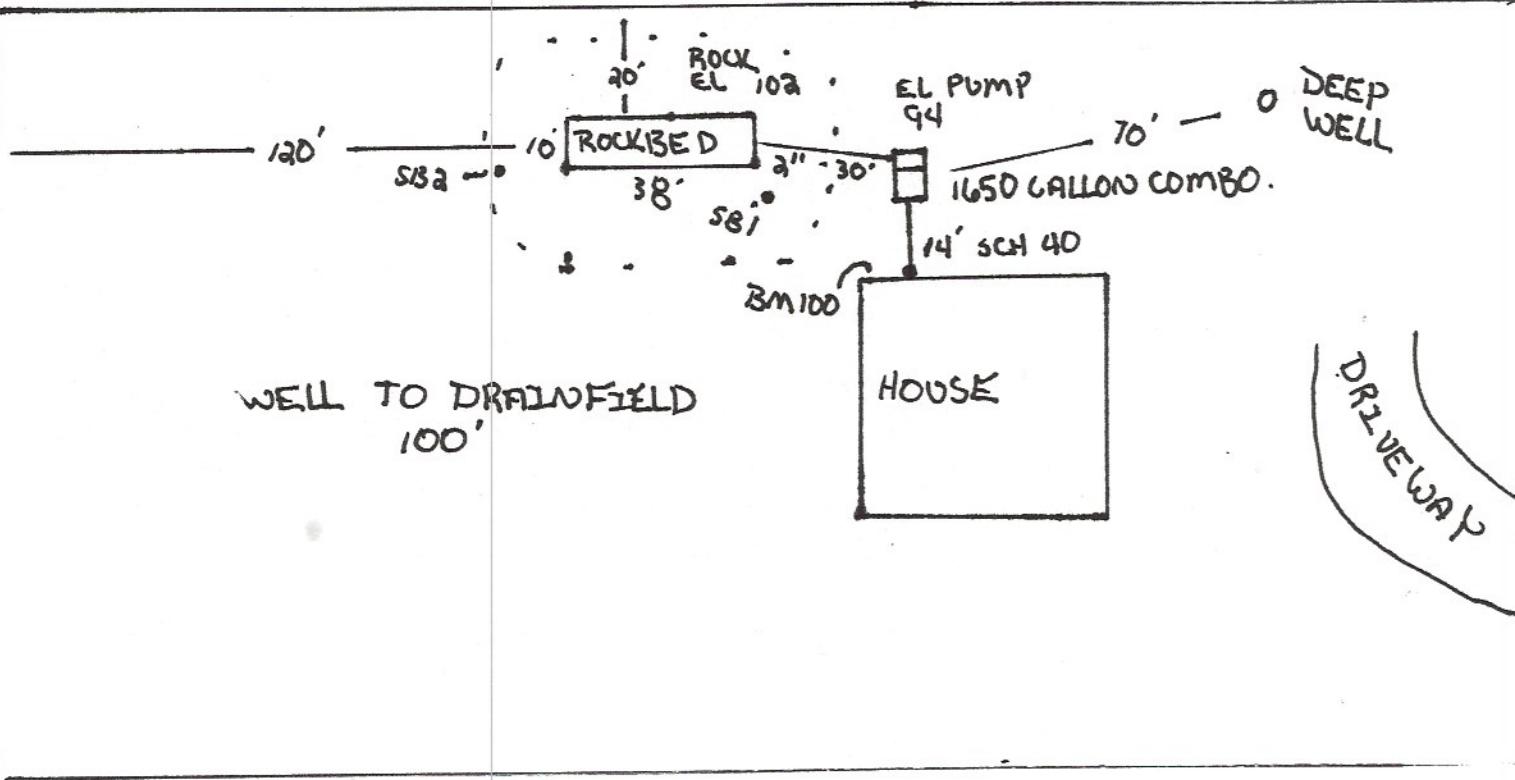
1" = 40'

19629 327TH AVE



LAKE
LAKES
MILLIE

NORTH LOT LINE



Maintenance Log

Activity	Date Accomplished
<i>Check frequently:</i>	
Leaks: check for plumbing leaks	
Soil treatment area check for surfacing	
Lint filter: check, clean if needed	
Effluent screen: if owner-maintained	
Water usage rate (monitor frequency)	
<i>Check annually:</i>	
Caps: inspect, replace if needed	
Sludge & Scum/Pump	
Inlet & Outlet baffles	
Drainfield effluent leaks	
Pump, alarm, wiring	
Flush & clean laterals if cleanouts exists	
Other:	
Other:	

Notes:

Mitigation/corrective action plan:

Call a licensed septic professional with problems.

Subsurface Sewage Treatment System Management Plan

Property Owner: _____ Phone: _____ Date: _____
Mailing Address: _____ City: _____ Zip: _____
Site Address: _____ City: _____ Zip: _____

This management plan will identify the operation and maintenance activities necessary to ensure long-term performance of your septic system. Some of these activities must be performed by you, the homeowner. Other tasks must be performed by a licensed septic service provider.

System Designer: check every 36 months.
Local Government: check every 36 months.
State Requirement: check every 36 months.

(State requirements are based on MN Rules Chapter 7080.2450, Subp 2 & 3)

**My System needs to be checked
every 36 months.**

Homeowner Management Tasks

- Leaks – Check (look, listen) for leaks in toilets and dripping faucets. Repair leaks promptly.
 - Surfacing sewage – Regularly check for wet or spongy soil around your soil treatment area.
 - Effluent filter – Inspect and clean twice a year or more.
 - Alarms – Alarm signals when there is a problem. Contact a service provider any time an alarm signals.
 - Event counter or water meter – Record your water use.
- recommend meter reading: he conducted (circle one: DAILY WEEKLY MONTHLY)

Professional Management Tasks

- Check to make sure tank is not leaking
- Check and clean the in-tank effluent filter
- Check the sludge/scum layers levels in all septic tanks
- Recommend if tank should be pumped
- Check inlet and outlet baffle
- Check the drainfield effluent levels in the rock layer
- Check the pump and alarm system functions
- Check wiring for corrosion and function
- Check dissolved oxygen and effluent temperature in tank
- Provide homeowner with list of results and any action to be taken
- Flush and clean laterals if clogs exist

"I understand it is my responsibility to properly operate and maintain the sewage treatment system on this property, utilizing the Management Plan. If requirements in the Management Plan are not met, I will promptly notify the permitting authority and take necessary corrective actions. If I have a new system, I agree to adequately protect the reserve area for future use as a soil treatment system."

Property Owner Signature: _____ Date: _____

Designer Signature: Jerry Flynn Date: _____

See Reverse Side for Management Log