

### FIELD EVALUATION SHEET

PRELIMINARY EVALUATION DATE 6-10-16 FIELD EVALUATION DATE 6-10-16  
 PROPERTY OWNER: Clark Penson PHONE \_\_\_\_\_  
 ADDRESS: 23950 U.S. Hwy 1169 CITY, STATE, ZIP: Wilkes 36431  
 LEGAL DESCRIPTION: 1 ac lot 5.11 Acres  
 PIN# 07-0-002400 SEC 2 T 76 R 27 TWP NAME Farm Island  
 FIRE# \_\_\_\_\_ LAKE/RIVER Ripple River LAKE CLASS \_\_\_\_\_ OHWL \_\_\_\_\_ FT \_\_\_\_\_

#### DESCRIPTION OF SOIL TREATMENT AREAS

	AREA #1	AREA #2
DISTURBED AREAS	YES _____ NO <u>X</u>	YES _____ NO _____
COMPACTED AREAS	YES _____ NO <u>X</u>	YES _____ NO _____
FLOODING	YES _____ NO <u>X</u>	YES _____ NO _____
RUN ON POTENTIAL	YES _____ NO <u>X</u>	YES _____ NO _____
SLOPE %	<u>1%</u>	YES _____ NO _____
DIRECTION OF SLOPE	<u>Easterly</u>	YES _____ NO _____
LANDSCAPE POSITION	<u>Gentle</u>	YES _____ NO _____
VEGETATION TYPES	<u>grass yard Oak trees</u>	

REFERENCE BM ELEV. 100" F  
 REFERENCE BM DESCRIPTION  
Toot Shed in yard at Base

DEPTH TO STANDING WATER OR MOTTLED SOIL: BORING# 1 16" 1A \_\_\_\_\_, 2 15" 2A \_\_\_\_\_

BOTTOM ELEVATION—FIRST TRENCH OR BOTTOM OF ROCK BED: #1 124" FT., #2 \_\_\_\_\_ FT.

SOIL SIZING FACTOR: SITE #1 1.67, SITE #2 \_\_\_\_\_

CONSTRUCTION RELATED ISSUES: 1820 tank to 10x50 Rock Bed on 2' End Base

LIC# 2088

SITE EVALUATOR SIGNATURE: Bob Burt

SITE EVALUATOR NAME: \_\_\_\_\_

TELEPHONE# \_\_\_\_\_

LUG REVIEW DA

DATE 6/24/16

Comments: Replacement System Ripple is Forged here carrier,  
100ft setback

SOIL BORING LOGS ON REVERSE SIDE

07-0-002400

### SOILS CHARTS FOR BOTH PROPOSED AND ALTERNATE SITES

1 (PROPOSED) SOILS DATA

DEPTH (INCHES)	TEXTURE	MUNSELL COLOR
5"	top soil	10YR 3/3
1	Loam clay	7.5YR 4/4
16		
	Lo	7.5YR 4/2

2 (PROPOSED) SOILS DATA

DEPTH (INCHES)	TEXTURE	MUNSELL COLOR
4"	Top soil	10YR 3/3
1	Loam	7.5YR 4/4
15"		
	Lo	7.5YR 4/2

1 (ALTERNATE) SOILS DATA

DEPTH (INCHES)	TEXTURE	MUNSELL COLOR

2 (ALTERNATE) SOILS DATA

DEPTH (INCHES)	TEXTURE	MUNSELL COLOR

ADDITIONAL SOIL BORINGS MAY BE REQUIRED

# MOUND DESIGN WORK SHEET (For Flows up to 1200 gpd)

67-0-002460

## A. Average Design FLOW

Estimated 600 gpd (see figure A-1)  
or measured \_\_\_\_\_ x 1.5 (safety factor) = \_\_\_\_\_ gpd

number of bedrooms	Class I	Class II	Class III	Class IV
2	300	225	180	60% of the values in the Class I, II, or III columns
3	450	300	218	
4	600	375	256	
5	750	450	294	
6	900	525	332	
7	1050	600	370	
8	1200	675	408	

## B. SEPTIC TANK Capacity

1826 <sup>60-60</sup> gallons (see figure C-1)

Number of Bedrooms	Minimum Liquid Capacity	Liquid capacity with garbage disposal	Liquid capacity with disposal & lift inside
2 or less	750	1125	1500
3 or 4	1000	1500	2000
5 or 6	1500	2250	3000
7, 8 or 9	2000	3000	4000

## C. SOILS (refer to site evaluation)

- Depth to restricting layer = 15" feet
- Depth of percolation tests = \_\_\_\_\_ feet
- Texture Loam  
Percolation rate \_\_\_\_\_ mpi
- Soil loading rate 1.67 gpd/sqft (see figure D-33)
- Percent land slope 1 %

## D. ROCK LAYER DIMENSIONS

- Multiply average design flow (A) by 0.83 to obtain required rock layer area.  
600 gpd x 0.83 sqft/gpd = 500 sqft
- Determine rock layer width = 0.83 sqft/gpd x linear Loading Rate (LLR)  
0.83 sqft/gpd x \_\_\_\_\_ gpd/sqft = 10 ft
- Length of rock layer = area ÷ width =  
500 sqft (D1) ÷ 10 ft (D2) = 50 ft

< 120 MPI	≤ 12
≥ 120 MPI	≤ 6

## E. ROCK VOLUME

- Multiply rock area (D1) by rock depth of 1 ft to get cubic feet of rock  
500 sqft x 1 ft = 500 cuft
- Divide cuft by 27 cuft/cuyd to get cubic yards  
500 cuft ÷ 27 cuyd/cuft = 19 cuyd
- Multiply cubic yards by 1.4 to get weight of rock in tons  
19 cuyd x 1.4 ton/cuyd = 27 tons

## F. SEWAGE ABSORPTION WIDTH

Absorption width equals absorption ratio (See Figure D-33) times rock layer width (D2)

10 x 2.00 ft = 20 ft

Percolation Rate in Minutes per Inch (MPI)	Soil Texture	Loading Rate Gallons per day per square foot	Absorption Ratio
Faster than 5	Coarse Sand Medium Sand Loamy Sand Fine Sand	1.20	1.00
6 to 15	Sandy Loam	0.75	1.50
16 to 30	Loam	0.60	2.00
31 to 45	Silt Loam Silt	0.50	2.40
46 to 60	Sandy Clay Loam Silty Clay Loam Clay Loam	0.45	2.67
61 to 120	Silty Clay Sandy Clay Clay	0.34	3.00
Slower than 120*			

\*Systems designed for these soils must be under air pressure.

07-0-002400

← 1% land slope

G. Mound Slope Width and Length (landslope less than or equal to 1%)

1. Absorption width (F) 20 ft.

2. Calculate mound size

a. Determine depth of clean sand fill at upslope edge of rock layer = 3 ft minus the distance to restricting layer (C1)

3 ft - 1 ft = 2 ft

b. Mound height at the upslope edge of rock layer = depth of clean sand for separation (G2a) at upslope edge plus depth of rock layer (1 ft) plus depth of cover (1 ft)

2 ft + 1ft + 1ft = 4 ft

c. Berm width = upslope mound height (G2b) times 4 (4 is recommended, but could be 3-12)

4 x 4 = 16 ft

d. The total landscape width is the sum of berm (G2c) width plus rock layer width (D2) plus berm width (G2c): 16 ft + 10 ft + 16 ft = 42 ft

e. Additional width necessary for absorption = absorption width (F) minus the landscape width (G2c)

20 ft - 16 ft = 4 ft, if number is negative (<0) skip to g

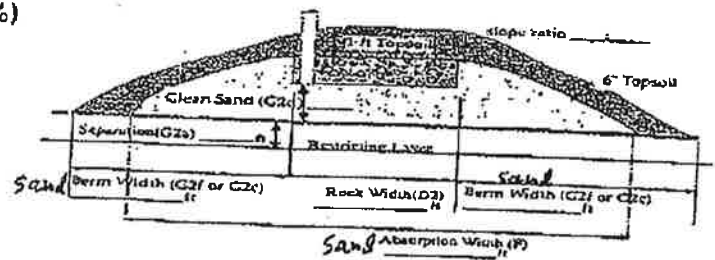
f. Final berm width = additional width (G2e) plus the berm width (G2c)

4 ft + 16 ft = 20 ft

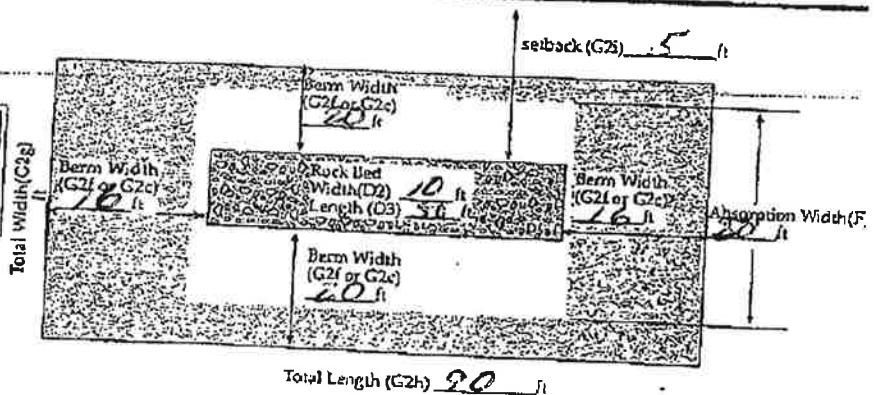
g. Total mound width is the sum of berm width (G2f or G2c) plus rock layer width (D2) plus berm width (G2f or G2c): 16 ft + 10 ft + 20 ft = 46 ft

h. Total mound length is the sum of berm (G2f or G2c) plus rock layer length (D3) plus berm (G2f or G2c): 20 ft + 50 ft + 20 ft = 90 ft

i. Setbacks from the rockbed are calculated as follows: the absorption width (F) minus the rock bed width (D2) divided by 2: (20 ft - 10 ft) ÷ 2 = 5 ft



Final Dimensions:  
46' x 90'



I hereby certify that I have completed this work in accordance with applicable ordinances, rules and laws.  
Bob Burt (signature) 2088 (license #) 6-10-16 (date)

07-0-002400

### PRESSURE DISTRIBUTION SYSTEM

1. Select number of perforated laterals 3
2. Select perforation spacing = 4 ft
3. Since perforations should not be placed closer than 1 foot to the edge of the rock layer (see diagram), subtract 2 feet from the rock layer length.

$$\frac{50}{\text{Rock layer length}} - 2 \text{ ft} = \underline{48} \text{ ft}$$

4. Determine the number of spaces between perforations. Divide the length (3) by perforation spacing (2) and round down to nearest whole number.

$$\text{Perforation spacing} = \underline{48} \text{ ft} \div \underline{4} \text{ ft} = \underline{12} \text{ spaces}$$

5. Number of perforations is equal to one plus the number of perforation spaces (4). Check figure E-4 to assure the number of perforations per lateral guarantees <10% discharge variation.

$$\underline{12} \text{ spaces} + 1 = \underline{13} \text{ perforations/lateral}$$

6. A. Total number of perforations = perforations per lateral (5) times number of laterals (1)

$$\underline{13} \text{ perfs/lat} \times \underline{3} \text{ lat} = \underline{39} \text{ perforations}$$

- B. Calculate the square footage per perforation. Should be 6-10 sqft/perf. Does not apply to at-grades.

$$\text{Rock bed area} = \text{rock width (ft)} \times \text{rock length (ft)}$$

$$\underline{10} \text{ ft} \times \underline{50} \text{ ft} = \underline{500} \text{ sqft}$$

$$\text{Square foot per perforation} = \text{Rock bed area} \div \text{number of perfs (6)}$$

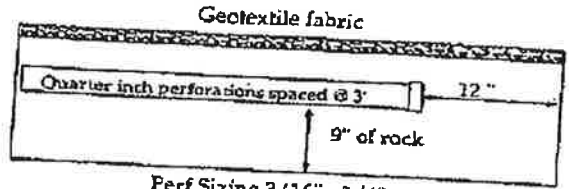
$$\underline{500} \text{ sqft} \div \underline{39} \text{ perfs} = \underline{13} \text{ sqft/perf}$$

7. Determine required flow rate by multiplying the total number of perforations (6A) by flow per perforation (see figure E-6)

$$\underline{39} \text{ perfs} \times \underline{.74} \text{ gpm/perfs} = \underline{29} \text{ gpm}$$

8. If laterals are connected to header pipe as shown on upper example, to select minimum required lateral diameter; enter figure E-4 with perforation spacing (2) and number of perforations per lateral (5) Select minimum diameter for perforated lateral = 1 1/2 inches.

9. If perforated lateral system is attached to manifold pipe near the center, lower diagram, perforated lateral length (3) and number of perforations per lateral (5) will be approximately one half of that in step 8. Using these values, select minimum diameter for perforated lateral = \_\_\_\_\_ inches.



Perf Sizing 3/16" - 1/4"  
Perf Spacing 1.5' - 5'

E-4: Maximum allowable number of 1/4-inch perforations per lateral to guarantee <10% discharge variation

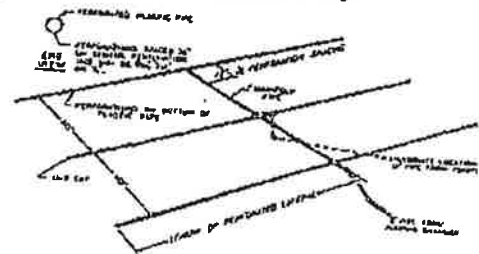
perforation spacing (feet)	1 inch	1.25 inch	1.5 inch	2.0 inch
2.5	8	14	18	28
3.0	8	13	17	26
3.3	7	12	16	25
4.0	7	11	15	23
5.0	6	10	14	22

E-6: Perforation Discharge in gpm

head (feet)	perforation diameter (inches)			
	1/8	3/16	7/32	1/4
1.0 <sup>a</sup>	0.18	0.42	0.56	0.74
2.0 <sup>b</sup>	0.26	0.59	0.80	1.04
5.0	0.41	0.94	1.26	1.65

<sup>a</sup> Use 1.0 foot for single-family homes.  
<sup>b</sup> Use 2.0 feet for anything else.

MANIFOLD LOCATED AT END OF PRESSURE DISTRIBUTION SYSTEM



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07-0-002400

# PUMP SELECTION PROCEDURE

## 1. Determine pump capacity:

### A. Gravity distribution

1. Minimum required discharge is 10 gpm
2. Maximum suggested discharge is 45 gpm. For other establishments at least 10% greater than the water supply rate, but no faster than the rate at which effluent will flow out of the distribution device.

### B. Pressure distribution

See pressure distribution work sheet

From A or B Selected pump capacity: 29 gpm

## 2. Determine pump head requirements:

A. Elevation difference between pump and point of discharge?

8 feet

B. Special head requirement? (See Figure at right - Special Head Requirements)

5 feet

### C. Calculate Friction loss

1. Select pipe diameter 2 in

2. Enter Figure E-9 with gpm (1A or B) and pipe diameter (C1).

Read friction loss in feet per 100 feet from Figure E-9

Friction Loss = 1.55 ft/100ft of pipe

3. Determine total pipe length from pump discharge to soil treatment discharge point. Estimate by adding 25 percent to pipe length for fitting loss. Total pipe length times 1.25 = equivalent pipe length

70 feet x 1.25 = 88 feet

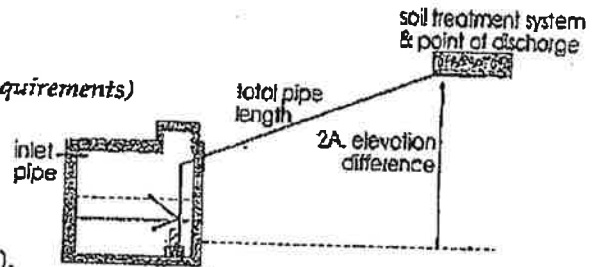
4. Calculate total friction loss by multiplying friction loss (C2) in ft/100 ft by the equivalent pipe length (C3) and divide by 100.

= 1.55 ft/100ft x 88 ÷ 100 = 1.4 ft

D. Total head required is the sum of elevation difference (A), special head requirements (B), and total friction loss (C4)

13 ft + 1.4 ft + 14.4 ft =

Total head: 14.4 feet



Special Head Requirements	
Gravity Distribution	0 ft
Pressure Distribution	5 ft

flow rate gpm	E-9: Friction Loss in Plastic Pipe Per 100 feet		
	nominal pipe diameter		
	1.5"	2"	3"
20	2.47	0.73	0.11
25	3.73	1.11	0.16
30	5.23	1.55	0.23
35	6.96	2.06	0.30
40	8.91	2.64	0.39
45	11.07	3.28	0.48
50	13.46	3.99	0.58
55		4.76	0.70
60		5.60	0.82
65		6.48	0.95
70		7.44	1.09

## 3. Pump selection

A pump must be selected to deliver at least 29 gpm (1A or B) with at least 14.4 feet of total head (2D)

I hereby certify that I have completed this work in accordance with applicable ordinances, rules and laws.

Bob Bull (signature)

(signature)

2088 (license #)

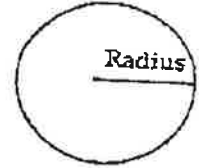
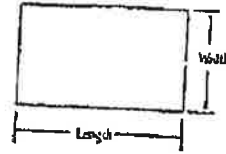
(license #)

6-10-16 (date)

(date)

# 07-0-002400 DOSING CHAMBER SIZING

- Determine area
  - Rectangle area =  $L \times W$   
\_\_\_\_\_ x \_\_\_\_\_ = \_\_\_\_\_ square feet
  - Circle area =  $\pi (3.14) \times \text{radius in feet} \times \text{radius in feet}$   
 $3.14 \times \text{ft} \times \text{ft} = \text{sqft}$
  - Get area from manufacturer 23.5 sqft



- Calculate gallons per inch  
There are 7.5 gallons per cubic foot of volume, therefore multiply the area (1A, B or C) times the conversion factor and divide by 12 inches per foot to calculate gallon per inch.  
 $\text{Area} \times 7.5 \div 12 = \text{sqft} \times 7.5 \div 12 \text{ in/ft} = \underline{15.9}$  gallon per inch

- Calculate total tank volume
  - Depth from bottom of inlet pipe to tank bottom 42 in
  - Total tank volume = depth from bottom of inlet pipe to tank bottom (3A) x gal/in (2)  
 $= 42 \text{ in} \times 15.9 \text{ gal/in} = \underline{666}$  gal

**Legal Tank:**  
500 gallons or  
100% the Daily flow  
or  
Alternating Pumps

- Calculate gallons to cover pump (with 2-3 inches of water covering pump)  
(Pump and block height (inch) + 2 inch) x gallon/inch  
 $(\underline{12} \text{ in} + 2 \text{ in}) \times 15.9 \text{ gal/in} = \underline{223}$  gallon

- Calculate total pumpout volume
  - Select pump size for 4-5 doses per day. Gallon per dose = gpd (see figure A-1) / doses per day = 600 gpd ÷ 5 doses/day = 120 gallons

- Calculate drainback
  - Determine total pipe length, 70 feet
  - Determine liquid volume of pipe, 112 gal per ft (see figure E-20)
  - Drainback quantity = 70 ft (5B1) x 1.6 gal per ft (5B2) = 112 gal
- Total pump out volume = dose volume (5A) + drainback (5B3)  
 $\underline{120} \text{ gal} + \underline{112} \text{ gal} = \underline{232}$  Total gallon

A-1: Estimated Sewage Flows in Gallons per Day

Number of bedrooms	Class I	Class II	Class III	Class IV
2	300	225	180	60%
3	450	300	216	of the
4	600	375	252	values
5	750	450	294	in the
6	900	525	332	Class I,
7	1050	600	370	L or II
8	1200	675	408	columns.

- Float separation distance (using total pumpout volume)  
Total pumpout volume (5C) ÷ gal/inch (2)  
 $\underline{232} \text{ gal} \div 15.9 \text{ gal/in} = \underline{14}$  inch

E-20: Volume of Liquid in Pipe

Pipe Diameter inches	Gallons per foot
1	0.045
1.25	0.078
1.5	0.11
2	0.17
2.5	0.25
3	0.38
4	0.66

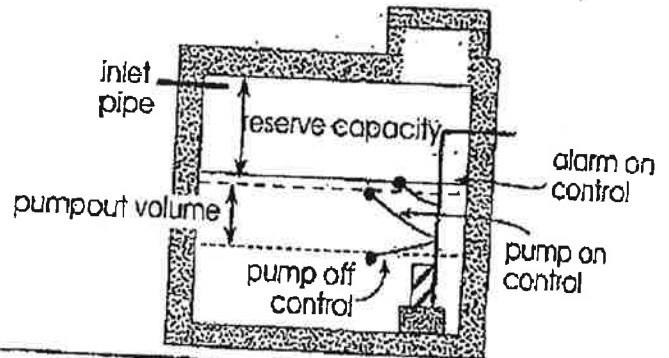
- Calculate volume for alarm (typically 2 to 3 inches)  
Alarm depth (inch) x gallon/inch (2) = 3 in x 15.9 gal/in = 48 gal

- Calculate total gallon = gallons over pump (4) + gallons pumpout (5C) + gallons alarm (7)  
 $\underline{120} \text{ gal} + \underline{223} \text{ gal} + \underline{48} \text{ gal} = \underline{403}$  gallons

- Total Tank Depth = total gallon (8) ÷ gallon/inch (2)  
 $\underline{403} \text{ gal} \div 15.9 \text{ gal/in} = \underline{25}$  in

**Recommended:**

Calculate reserve capacity (75% the daily flow)  
Daily flow x .75 = 600 x .75 = 450 gallons



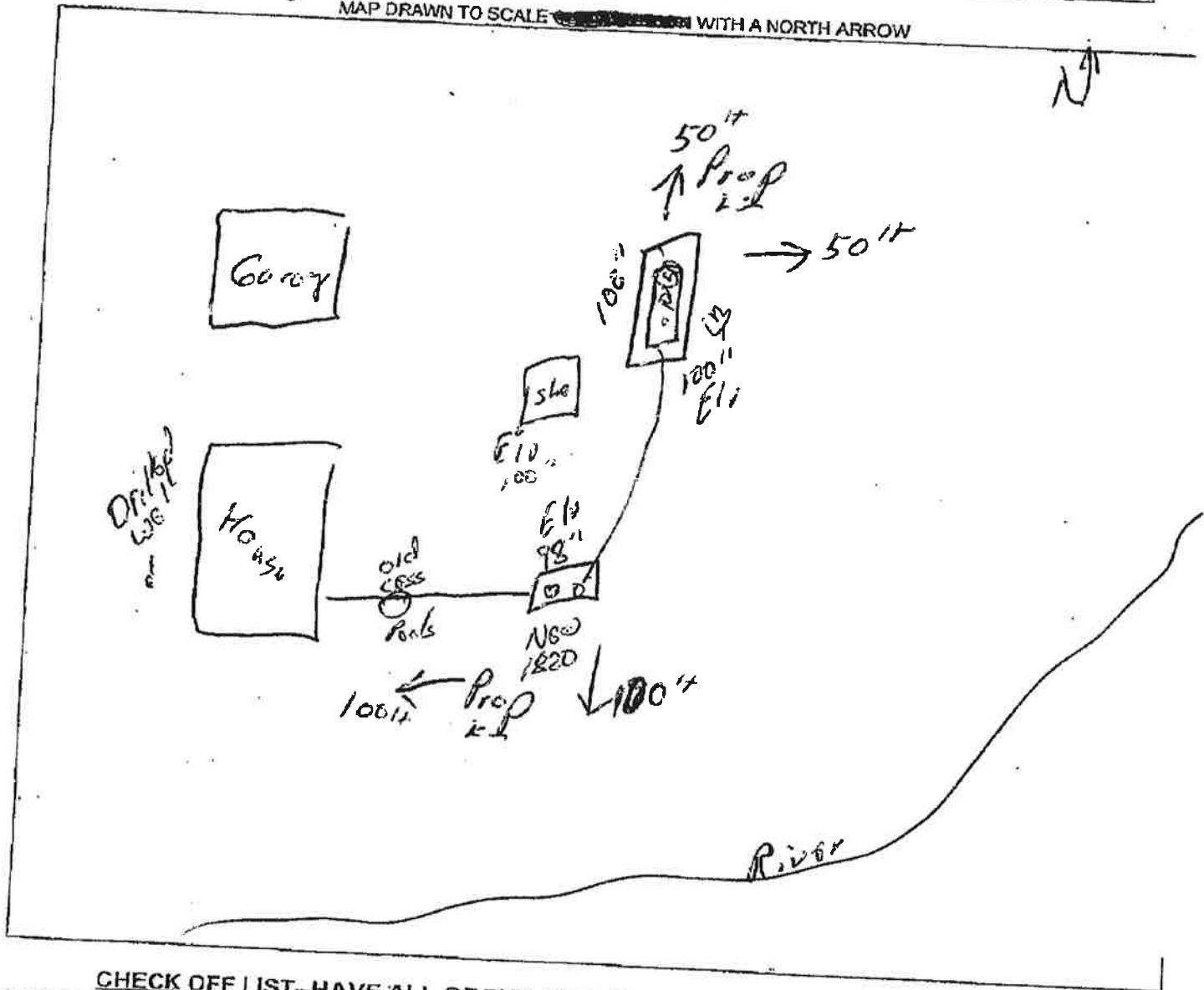
I hereby certify that I have completed this work in accordance with applicable ordinances, rules and laws.  
Bob Baird (signature) 2022 (license #) 6-10-16 (date)

CLIENT Clark Peysor

07-0-002400

DATE: 6-10-16

MAP DRAWN TO SCALE WITH A NORTH ARROW



**CHECK OFF LIST - HAVE ALL OF THE FOLLOWING BEEN DRAWN ON THE MAP??**

- SHOW EXISTING OR PROPOSED
- WATER WELLS WITHIN 100 FT OF TREATMENT AREAS
- PRESSURE WATER LINES WITHIN 10 FT OF TREATMENT AREAS
- STRUCTURES
- ALL SOIL TREATMENT AREAS
- HORIZONTAL AND VERTICAL REFERENCE
- POINT OF SOIL BORINGS
- LOT EASEMENTS
- DISTURBED/COMPACTED AREAS
- SITE PROTECTION - LATHE AND RIBBON EVERY 15 FT
- ACCESS ROUTE FOR TANK MAINTENANCE
- REQUIRED SETBACKS
- STRUCTURES
- OHWL
- PROPERTY LINES

**INDICATE ELEVATIONS**

- BENCHMARK 100"
- ELEVATION OF SEWER LINE @ HOUSE 76"
- ELEVATION @ TANK INLET 74"
- ELEVATION @ BOTTOM OF ROCK LAYER 124"
- ELEVATION @ BOTTOM OF BORING OR RESTRICTIVE LAYER 85"
- ELEVATION OF PUMP 26"
- ELEVATION OF DISTRIBUTION DEVICE 33"

DESIGNER SIGNATURE Bob Bault  
LICENSE# 2088

DATE 6-10-16



# Subsurface Sewage Treatment System Management Plan

07-0-002400

Property Owner: Clark Byssar Phone: \_\_\_\_\_ Date: 6-10-16  
 Mailing Address: \_\_\_\_\_ City: \_\_\_\_\_ Zip: \_\_\_\_\_  
 Site Address: 33250 US Hwy 169 City: A. Thomas Zip: 56431

This management plan will identify the operation and maintenance activities necessary to ensure long-term performance of your septic system. Some of these activities must be performed by you, the homeowner. Other tasks must be performed by a licensed septic service provider.

System Designer: check every \_\_\_\_\_ months.  
 Local Government: check every \_\_\_\_\_ months.  
 State Requirement: check every 36 months.

My System needs to be checked every 36 months.

### Homeowner Management Tasks

- Leaks - Check (look, listen) for leaks in toilets and dripping faucets. Repair leaks promptly.
- Surfacing sewage - Regularly check for wet or spongy soil around your soil treatment area.
- Effluent filter - Inspect and clean twice a year or more.
- Alarms - Alarm signals when there is a problem. Contact a service provider any time an alarm signals.
- Event counter or water meter - Record your water use.  
 -recommend meter readings be conducted (circle one): DAILY WEEKLY MONTHLY

### Professional Management Tasks

- Check to make sure tank is not leaking
- Check and clean the in-tank effluent filter
- Check the sludge/scum layer levels in all septic tanks
- Recommend if tank should be pumped
- Check inlet and outlet baffles
- Check the drainfield effluent levels in the rock layer
- Check the pump and alarm system functions
- Check wiring for corrosion and function
- Check dissolved oxygen and effluent temperature in tank
- Provide homeowner with list of results and any action to be taken
- Flush and clean laterals if cleanouts exist

"I understand it is my responsibility to properly operate and maintain the sewage treatment system on this property, utilizing the Management Plan. If requirements in the Management Plan are not met, I will promptly notify the permitting authority and take necessary corrective actions. If I have a new system, I agree to adequately protect the reserve area for future use as a soil treatment system."

Property Owner Signature: \_\_\_\_\_ Date: \_\_\_\_\_  
 Designer Signature: Bob Bartel Date: 6-10-16

See Reverse Side for Management Log

07-0-062460  
Maintenance Log

Activity	Date Accomplished											
<i>Check frequently:</i>												
Leaks: check for plumbing leaks												
Soil treatment area check for surfacing												
Lint filter: check, clean if needed												
Effluent screen: if owner-maintained												
Water usage rate (monitor frequency _____)												
<i>Check annually:</i>												
Caps: inspect, replace if needed												
Sludge & Scum/Pump												
Inlet & Outlet baffles												
Drainfield effluent leaks												
Pump, alarm, wiring												
Flush & clean laterals if cleanouts exists												
Other:												
Other:												

Notes:

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Mitigation/corrective action plan:

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*Bob Bartel*